

AD-A169 075



EVALUATION OF GEOTEXTILE REINFORCED
EMBANKMENTS ON THE FARMER'S LOOP ROAD IN
FAIRBANKS, ALASKA

A SPECIAL RESEARCH PROBLEM
PRESENTED TO
THE FACULTY OF THE SCHOOL OF CIVIL ENGINEERING

BY

GEORGE PAPAIOANOU

IN PARTIAL FULFILLMENT
OF THE REQUIREMENTS FOR THE DEGREE OF
MASTER OF SCIENCE

DTIC FILE COPY

This document has been approved
for release and distribution
by the DTIC staff.

JUNE, 1986

JUN 23 1986

GT

GEORGIA INSTITUTE OF TECHNOLOGY
A UNIT OF THE UNIVERSITY SYSTEM OF GEORGIA
SCHOOL OF CIVIL ENGINEERING
ATLANTA, GEORGIA 30332



**Best
Available
Copy**

DISCLAIMER NOTICE

THIS DOCUMENT IS BEST QUALITY PRACTICABLE. THE COPY FURNISHED TO DTIC CONTAINED A SIGNIFICANT NUMBER OF PAGES WHICH DO NOT REPRODUCE LEGIBLY.

①

EVALUATION OF GEOTEXTILE REINFORCED
EMBANKMENTS ON THE FARMER'S LOOP ROAD IN
FAIRBANKS, ALASKA

A SPECIAL RESEARCH PROBLEM
PRESENTED TO
THE FACULTY OF THE SCHOOL OF CIVIL ENGINEERING

BY

GEORGE PAPAIOANOU

IN PARTIAL FULFILLMENT
OF THE REQUIREMENTS FOR THE DEGREE OF
MASTER OF SCIENCE

NO 228-85-G-3247

GEORGIA INSTITUTE OF TECHNOLOGY

JUNE, 1986

UNIVERSITY OF
ALABAMA
LIBRARY
JUN 26 1986

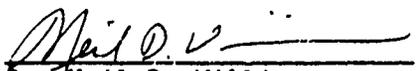
A

EVALUATION OF GEOTEXTILE REINFORCED
EMBANKMENTS ON THE FARMER'S LOOP ROAD IN
FAIRBANKS, ALASKA

BY

GEORGE PAPAIOANOU

APPROVED:



Dr. Neil D. Williams

Dr. Leland S. Riggs

A1 23 EW



ACKNOWLEDGEMENTS

I wish to thank Mr. Robert Roglin for the time he spent helping me understand the finite element analysis procedure and his efforts in writing the plotting program used in the analysis. I wish to thank Dr. Neil D. Williams for expressing his confidence in my ability by selecting me for this unique project. I also wish to thank Ms. Henrietta Bowman for her clerical support throughout the duration of this project.

I wish to express my deepest appreciation for my parents, family, and close friends for their faith, encouragement, and love which has given me the strength and endurance to complete my educational work.

TABLE OF CONTENTS

	Page
ACKNOWLEDGEMENTS.....	i
TABLE OF CONTENTS.....	ii
LIST OF TABLES.....	iv
LIST OF FIGURES.....	v
Chapter	
I. INTRODUCTION.....	1
1.1 Statement of Problem.....	1
1.2 Objective.....	5
1.3 Purpose.....	5
1.4 Research Approach.....	6
II. SLOPE STABILITY ANALYSIS.....	7
2.1 STABGM.....	7
2.1.1 Program Operation.....	8
2.2 Reinforcement of Embankments on Weak Soil.....	14
2.3 Initial Analysis.....	16
2.3.1 Assumptions.....	17
2.3.2 Results of Initial Analysis.....	20
2.3.3 Revisions.....	21
2.4 Results of STABGM Analyses.....	21
III. NUMERICAL ANALYSES.....	25
3.1 FEADAM84 Analyses.....	25
3.1.1 Non-Linear Incremental Finite Element Methodology.....	26
3.1.2 Analysis Procedure.....	38
3.1.3 Initial Analysis.....	40
3.1.4 Summary of Analysis.....	40
2 SSTIPN Analysis.....	46
3.2.1 Assumptions.....	50
3.2.2 Results of SSTIPN Analysis.....	55
3.2.3 Conclusions and Recommendations.....	88

Table of Contents (continued)

	Page
BIBLIOGRAPHY.....	91
APPENDICES	
A. STABGM Output for Initial Analyses.....	93
B. STABGM Output for Revised Analyses.....	105
C. Procurement of SSTIPN Program.....	131
D. BASIC Plotting Program.....	138
E. SSTIPN Output.....	142

LIST OF TABLES

Table		Page
2.1	Summary of Results.....	23
3.1	Summary of Hyperbolic Parameters.....	41
3.2	Summary of Soil Parametric Values.....	45

LIST OF FIGURES

Figure	Page
1.1 Typical Roadway Distress from Thaw Beneath Sideslopes.....	2
2.1 Coordinate System Used in STABGM.....	10
2.2 First Rotation Around the Given Center A.....	11
2.3 45-Degree Rotation Around the Center B.....	11
2.4 STABGM Initial Analysis.....	18
2.5 STABGM Revised Analysis.....	22
3.1 Hyperbolic Model for Stress-Strain Behavior.....	28
3.2 Comparison Between Stress Level and Stress State.....	29
3.3 Modelling Unloading - Reloading Moduli.....	34
3.4 Embankment Geometry Used in Numerical Analyses.....	43
3.5 Typical Cross Section of Farmers Loop Road.....	44
3.6 FEADAM84 Embankment Model.....	47
3.7 FEADAM84 Embankment Geometry.....	48
3.8 Deformed Embankment Geometry of FEADAM84 Analysis.....	49
3.9 SSTIPN Embankment Geometry.....	51
3.10 SSTIPN Embankment Model.....	53
3.11 Geotextile Strength Modulae.....	54
3.12 Deformed Embankment Geometry of SSTIPN Analysis.....	56
3.13 Deformed Embankment Geometry of SSTIPN Analysis.....	57
3.14 Deformed Embankment Geometry of SSTIPN Analysis.....	58
3.15 Deformed Embankment Geometry of SSTIPN Analysis.....	59
3.16 Deformed Embankment Geometry of SSTIPN Analysis.....	60

3.17	Deformed Embankment Geometry of SSTIPN Analysis.....	61
3.18	Deformed Embankment Geometry of SSTIPN Analysis.....	62
3.19	Deformed Embankment Geometry of SSTIPN Analysis.....	63
3.20	Deformed Embankment Geometry of SSTIPN Analysis.....	64
3.21	Deformed Embankment Geometry of SSTIPN Analysis.....	65
3.22	Deformed Embankment Geometry of SSTIPN Analysis.....	66
3.23	Deformed Embankment Geometry of SSTIPN Analysis.....	67
3.24	Deformed Embankment Geometry of SSTIPN Analysis.....	68
3.25	Deformed Embankment Geometry of SSTIPN Analysis.....	69
3.26	Deformed Embankment Geometry of SSTIPN Analysis.....	70
3.27	Deformed Embankment Geometry of SSTIPN Analysis.....	71
3.28	Deviatoric Stress Contour of SSTIPN Analysis.....	72
3.29	Deviatoric Stress Contour of SSTIPN Analysis.....	73
3.30	Deviatoric Stress Contour of SSTIPN Analysis.....	74
3.31	Deviatoric Stress Contour of SSTIPN Analysis.....	75
3.32	Deviatoric Stress Contour of SSTIPN Analysis.....	76
3.33	Deviatoric Stress Contour of SSTIPN Analysis.....	77
3.34	Deviatoric Stress Contour of SSTIPN Analysis.....	78
3.35	Deviatoric Stress Contour of SSTIPN Analysis.....	79
3.36	Deviatoric Stress Contour of SSTIPN Analysis.....	80
3.37	Deviatoric Stress Contour of SSTIPN Analysis.....	81
3.38	Deviatoric Stress Contour of SSTIPN Analysis.....	82
3.39	Deviatoric Stress Contour of SSTIPN Analysis.....	83
3.40	Deviatoric Stress Contour of SSTIPN Analysis.....	84
3.41	Deviatoric Stress Contour of SSTIPN Analysis.....	85
3.42	Deviatoric Stress Contour of SSTIPN Analysis.....	86
3.43	Deviatoric Stress Contour of SSTIPN Analysis.....	87

CHAPTER I

INTRODUCTION

Geotextiles have increasingly become an integral part of many civil engineering applications in recent years. Slope reinforcement, retaining walls, road construction, and drainage nets are a few among the many applications of these materials (3,12,16,19). One area which has been receiving more and more attention is the reinforcement of road embankments overlying soft subgrade soils (3,12,16,18,19). Much of the literature relating to the effect of geotextiles upon the performance of these embankments is conceptual in nature and have not been fully supported by experimental and theoretical studies (16). These analyses have primarily focused upon an embankment overlying one particular soft/weak subgrade soil. The analysis of an embankment overlying more than one subgrade soil has generally not been required, but has direct application to certain areas of the country.

1.1 Statement of the Problem

The effect of frozen ground on engineering considerations must allow deviations to conventional design practices. Normally one of two broad principles can be followed based on whether or not the frozen foundation soils are thaw-stable or thaw-unstable (14). When foundation soils are thaw-stable, conventional design and construction methods may be used (14). Where foundations soils are thaw-unstable, conventional design and construction methods must be modified to

attain sufficient functional and economic use of the facility. Previous studies by the Alaska Department of Highways of roadways constructed over permafrost have shown that progressively deeper thawing generally occurs beneath annually snow covered roadway side slopes (9). The insulating effect of snow-covered side slopes prevents the complete refreeze of the underlying soils during the winter which results in a progressive propagation of unfrozen ground or talik to occur annually (22). The degradation of this talik can be further accentuated by drifted or plowed snow accumulating on the roadway side slopes (14). The resulting decrease in shear strength and loss of bearing capacity results in consolidation and slope settlement in the thaw-unstable permafrost or talik (9,22). The thaw settlement which results due to these effects usually show up as lateral cracking on the wearing surface of the roadway (22). (See Figure 1.1)

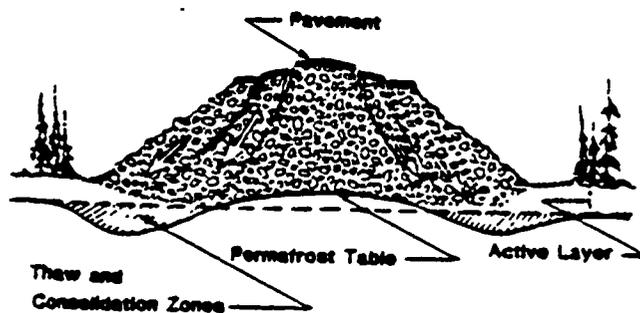


Figure 1.1 Typical roadway distress from thaw beneath slopes.

Methods such as the use of a peat underlay, placement of insulation layers within the embankment, toe berms, and air duct systems have been used to try and prevent and/or maintain the foundation soils in a frozen state (9,10,22). These methods have

aided in the short term maintenance of roadway integrity but have not solved the problems of long term thaw related settlements.

The use of geotextiles as a means of facilitating the construction of embankments on soft/weak foundations has gained in popularity (16). The mechanisms by which the fabric affects the behavior and performance of conventional aggregate-fabric-subgrade systems include the following:

- (1) Separation of the aggregate and subgrade soil
- (2) Provision of a filter medium to facilitate drainage
- (3) Confinement and reinforcement of the aggregate layer
- (4) Alteration of the failure mechanism in the subgrade soil (19)

The application of these principles in permafrost regions take on a slightly altered meaning. Separation of the aggregate and subgrade soil would primarily be affected over the talik zones. Although frozen soil may exhibit deformation and separation upon loading, the area of greatest concern would be over the thaw-unstable talik zone. The fabric overlying the thaw zone would prevent the aggregate and thawed subgrade soil from intermixing, which would tend to reduce the effective depth and load distributing capability of the aggregate fill (19).

The presence of a geotextile fabric may induce the formation of a natural filter in the subgrade soil. This process is initiated with the migration of some soil particles through the fabric as a result of fluid flow. Creation of the filter depends upon the soil grain size distribution, the porosity, opening size and opening size distribution of the geotextile, and the hydraulic gradient of the flow (19). The hydraulic gradient may be influenced by the in-situ stress state of the

fabric, where tensile and compressive stresses may alter the pore size distribution in a non-woven fabric (19). The filter induced by the fabric typically maintains the separation of the aggregate and subgrade soil while permitting the free flow of water (19). When the frozen soil under the toe of the embankment thaws, the resulting water is not able to drain off properly. The surrounding permafrost does not lend itself as an adequate drainage boundary so the water will tend to flow through the fabric as the outer edge of the embankment consolidates due to the loss of bearing capacity in the soil. This will result in differential settlement in the embankment causing longitudinal cracks in the pavement. This has been one of the primary failure mechanisms of roadway embankments constructed in these areas.

The primary function of the geotextile is provide tension reinforcement, transmitted from the adjacent soil by soil-geotextile shear stress. The reinforcement tensile forces resulting from shear load transfer transmit soil loads from one location (unstable zone) in a soil mass to another location (stable zone) (3). Granular materials have zero shear strength when no confinement is provided. When confined, granular materials furnish excellent load bearing capacity. The load bearing capacity of the aggregate in an embankment is limited by the shear stress which develop at the aggregate-subgrade interface (19). A layer of fabric at this location can restrain interfacial aggregate movement thereby increasing the interfacial shear strength and corresponding load bearing capacity of the aggregate layers (19). The load bearing capacity of the aggregate is reduced differentially when talik zones develop at the toe of roadway embankments. Additional

shear stress induced by the geotextile between the aggregate and talik zone provides confinement for the aggregate, thereby increasing the load bearing capacity.

1.2 Objective

The primary objective of this research paper is to estimate the required geotextile properties for reinforcement of the Farmers Loop Road in Fairbanks, Alaska so that surface deformations can be kept within tolerable levels. Numerical analyses are used to evaluate the effect of tensile strength, elastic modulae, and location of the geotextile on the surface deformations of the road.

1.3 Purpose

The purpose of this research is to provide a technically sound and economically feasible alternative solution to the long term problems associated with thaw-unstable foundation soils underlying roadway pavements in permafrost regions such as Fairbanks, Alaska. The functional life of a roadway constructed on permafrost is decreased by the varying properties inherently associated with frozen soil. Substantial changes in bearing capacity, shear strength, and volume contribute to decreasing the functional use of the pavement. Methods, previously described, have been attempted to increase the long term functional life of a roadway but have been successful only in the short run. The main purpose of this project is to evaluate the properties of a geotextile which may be used to reduce the magnitude of embankment deformations given the progressive degradation of the permafrost beneath the embankment slope areas.

1.4 Research Approach

The success of any method of analysis largely depends upon how well the actual conditions may be modelled. For embankments constructed over permafrost four basic failure modes may be defined:

- (1) Slope failure within the embankment.
- (2) Rotational failure involving the embankment and the foundation soil.
- (3) Bearing capacity failure of the thaw-unstable talik.
- (4) Differential settlement of embankment as a result of the consolidation of talik zone under the side slopes.

Two computer programs are used to develop the required geotextile properties to evaluate stress distributions for each of these failure modes. STABGM: A computer program for slope stability analysis with circular surfaces and geotextile reinforcement (7) is used to evaluate the stress distributions for the first two modes of failure. Finite Element Analysis for Dams (FEADAM) and Soil Structure Interaction Program (SSTIPN) are used to analyze stress distributions and evaluate geotextile properties for the third and fourth modes of failure.

Much of the numerical analyses work to date has been applied to embankments overlying soft soils of one type. This literature will be used to develop a logical approach to the unique problems associated with roadway embankments constructed on permafrost.

CHAPTER II

SLOPE STABILITY ANALYSIS

The computer program for slope stability analysis with circular slip surfaces and geotextile reinforcement (STABGM) may be used to analyze internal and external stability of roadway embankments constructed on permafrost. Internal stability refers to the stability of the embankment slope alone. Thus, the circular slip analyses to be employed are restricted to failure through the toe of the embankment. External stability refers to the stability of the embankment and foundation soil combined. For embankments constructed on soft soil, this is the most likely mechanism of failure (3).

2.1 STABGM

The computer program STABGM is a version of the computer program STABR modified in December 1984, by J. M. Duncan and B. K. Low. In addition to the slope stability analysis capabilities provided by STABR, STABGM can be used to analyze slopes containing up to twenty layers of reinforcement, with the forces in the reinforcement layers varying along their lengths.

The program calculates the factors of safety for specified circles, or searches for the circular slip surface having the minimum

factor of safety, using the Bishop's Modified Method or Ordinary Method of slices. Briefly, stability analysis using the ordinary method of slices requires trial of a large number of assumed failure surfaces. The failure zone is divided into a series of vertical slices. It is assumed that each slice acts independently of its neighbor. There is no shear developed between them, and the normal pressure on each side of a slice produced by the adjoining slices are equal (2). A series of calculations are then evaluated to determine the resisting moment, related to soil strength, and the overturning moment, related to the weight of the soil mass. A factor of safety is then evaluated for each trial failure surface. That which has the smallest factor of safety is the most critical surface, i.e., the one on which failure is most likely to occur (20). The Bishop's Modified Method is a more refined solution. The effect of forces on the sides of each slice are taken into account. Through a series of trial and error calculations, a factor of safety is found. The ordinary method of slices yields results which are too conservative (4). The Bishop's Modified Method is more representative of actual conditions and is used for the slope stability analysis on the Farmers Loop Road. The program may be used for either total or effective stress analyses, or a combination of both, and with or without seismic forces.

2.1.1 Program Operation

The program consists of a main program (STABGM) and three subroutines (BISHOP, ROOT, and SOLU). The geometry of a slope is described in an X-Y coordinate system. The X coordinate (horizontal) increases from the top to the toe of the slope and the Y coordinate

(vertical) increases downward, as shown in Figure 2.1. The program is capable of handling irregular slope profiles, tension cracks, soil layers with different properties and nonuniform thickness, complicated pore pressure patterns, and irregular variations of undrained strength with depth.(7)

Up to 20 vertical sections may be used to define the geometry of the slope. The first and the last vertical sections, as well as the last soil layer boundary, should be well beyond the extent of any possible slip circle. The strength of the soil in a layer may be specified either by the cohesion intercept (c) and the friction angle (ϕ) which are constant throughout the layer, or by a curve of undrained shear strength versus depth. The total number of points describing the variation of undrained strength with depth may not exceed 20.(7)

If a search for the critical circle is desired, the program searches for the circle having the minimum factor of safety by either the Ordinary Method of Slices or Bishop's Modified Method. Either a horizontal line to which all circles are tangent or a specific point through which all circles pass must be defined. In addition, coordinates of the center of the first circle to be analyzed, and the final grid spacing desired in the search must be specified. The search starts with calculation of factors of safety for the specified circle center and for centers spaced symmetrically around the specified center, as shown in Figure 2.2. The centers are generated in the order shown, by rotating around the specified center with a spacing whose length is equal to twice the final grid spacing.(7)

If a factor of safety less than that at the center of rotation

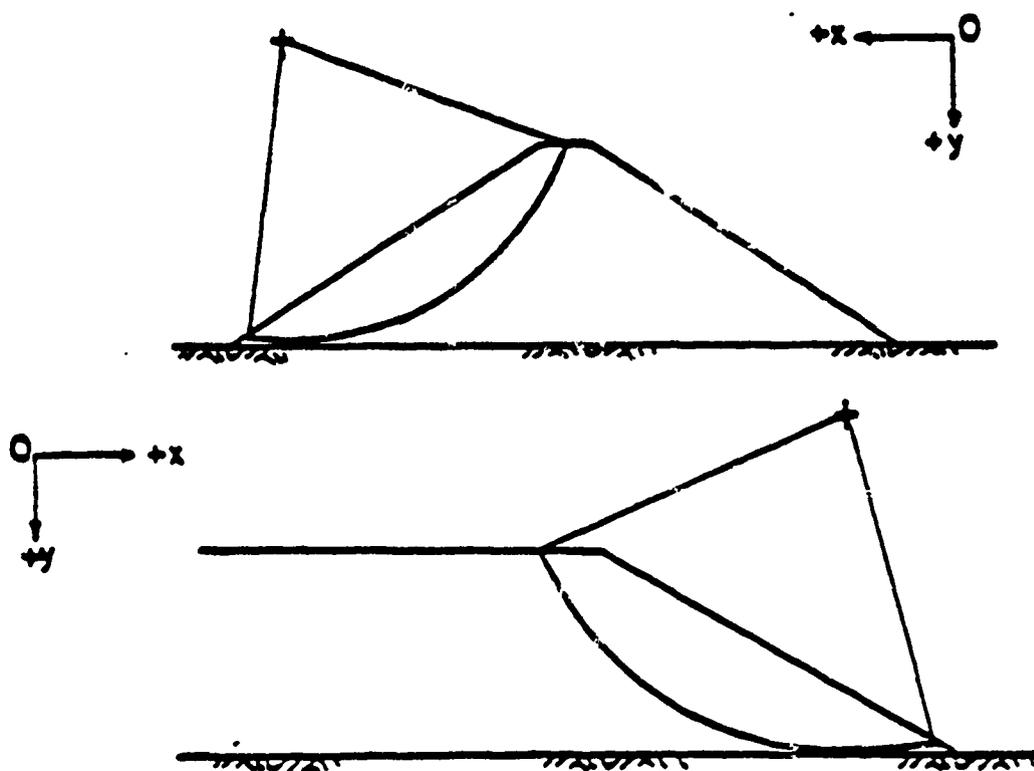


FIG. 2.1 COORDINATE SYSTEM USED IN STABGM

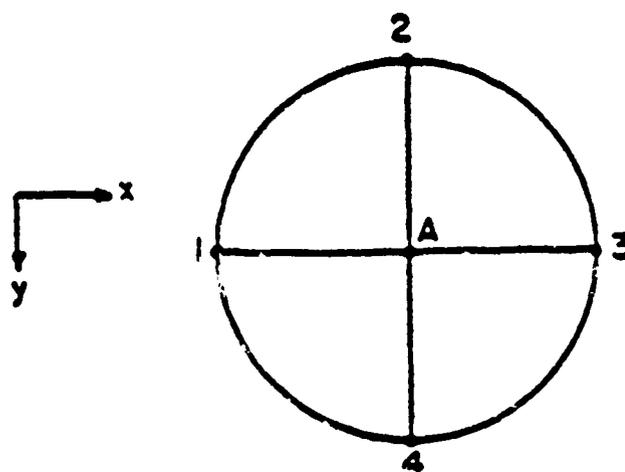


FIG. 2.2 FIRST CLOCKWISE ROTATION AROUND THE GIVEN CENTER A. THE ROTATION STARTS AT POINT 1, WITH RADIUS OF ROTATION TWICE THE FINAL GRID.

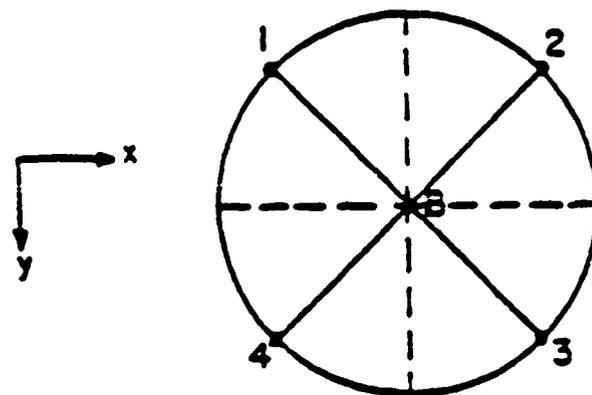


FIG. 2.3 THE 45-DEGREE CLOCKWISE ROTATION AROUND THE CENTER B. THE ROTATION STARTS AT POINT 1, WITH RADIUS OF ROTATION 1.414 TIMES THE FINAL GRID.

is found at any point, this point becomes the new center of rotation. If the difference between the factors of safety at the old and the new centers of rotation is equal to or less than 5% of the factor of safety at the new center of rotation, the spacing is reduced to the final grid spacing. If a full rotation is completed and no factor of safety smaller than that at the center of rotation has been found, a new clockwise rotation round the same center is initiated with the spacing being reduced to the final grid spacing.(7)

If a full rotation with the final grid spacing is completed and no factor of safety smaller than that at the center of rotation has been found, then another clockwise rotation around the same center is initiated. This rotation, however, starts at an angle of 45° with the horizontal. The increments of rotation are still 90° , thus filling in the corners of the final grid, as shown in Figure 2.3. If no smaller factor of safety is found in this rotation, the factor of safety at the center of rotation is assumed to be the minimum factor of safety, and the search is terminated.(7)

If the minimum factor of safety has not been found after 51 circles have been analyzed, the search is discontinued without printing out a minimum factor of safety. The program then proceeds to search for the minimum factor of safety for the next depth limiting tangent. When this occurs, the user should rerun the problem, specifying a new starting center for the search. It is frequently desirable to specify a rather large grid spacing for the initial search, so that the center of the critical circle can be located, at least approximately, by analyzing relatively few circles. Then a finer grid spacing may be

specified for a final search to locate the center of the critical circle with a higher degree of accuracy.(7)

With STABGM, the improvement in the factor of safety is considered to be due to the increased resisting moment provided by the reinforcement:

$$\Delta M_R = \sum_{i=1}^N [F_{Ri} (Y_{Ri} - Y_c)] \quad (1)$$

in which ΔM_R = resisting moment due to reinforcement

F_{Ri} = force in i th layer of reinforcement where it is intersected by the slip circle being analyzed

Y_{Ri} = Y - coordinate of reinforcement (down is positive)

Y_c = Y - coordinate of circle center

N = number of layers of reinforcement

The factor of safety of the reinforced slope is calculated as follows:

$$F_w = F_{w0} + \frac{\Delta M_R}{M_0} \quad (2)$$

in which F_w = factor of safety for a given circle with reinforcement

F_{w0} = factor of safety for the same circle without reinforcement, as calculated by the Ordinary Method of slices or Bishop's Modified Method

ΔM_R = resisting moment due to reinforcement, calculated using Equation 1

M_O = overturning moment for the circle

STABGM will search automatically for the circle with the minimum value of F_w to locate the circle with the minimum factor of safety.

Up to twenty horizontal layers of reinforcement can be specified, with as many as nine pairs of values of X-coordinate and force for each reinforcement layer to describe the variation of force along its length.

2.2 Reinforcement of Embankments on Weak Foundations

Geotextile reinforcement may be used at the interface between embankments and weak foundations to prevent failure of the foundation soil (3). Two performance criteria usually considered for normal embankments constructed on weak soil are adequate stability and acceptable total and differential settlement (3). The failure mechanisms associated with embankments constructed on permafrost are similar in principle but not in sequence. Initially, embankments constructed on permafrost are very stable. Permafrost is relatively strong as a foundation soil, but once thawed, the soil becomes very weak relative to its former strength. Additionally, thawing usually occurs under the side slopes of the embankment and not under the centerline which introduces the problem of nonuniform foundation soil properties and differential settlement after construction.

Internal stability can be impaired if the embankment itself lacks internal stability or if embankment failure results from

excessive deformation or failure of the foundation soil under the embankment load (3). Height of the embankment and the steepness of the side slopes govern the internal stability of an embankment. Both parameters also influence the magnitude and distribution of stresses on the foundation soil and, therefore, influence the stability of the foundation soil (3). This interrelation is directly applicable to embankments constructed over permafrost which has thawed. Upon thawing, the talik zone under the side slopes of the embankment loses bearing capacity resulting in settlement of the overlying side slopes. Consequently, the fill material may weaken and loosen thereby impairing embankment internal stability (3). Embankment internal stability can be improved by layers of reinforcement placed within the embankment (3,11,12,16,17,18,19).

The placement of a layer of reinforcement at the embankment/foundation soil interface affects embankment internal stability two ways. Upon thawing of the frozen soil under the side slopes, the embankment soil may slide along the geotextile interface. This mechanism is more likely to occur if the interface friction angle and adhesion are relatively low between the geotextile and the embankment soil (3). If lateral sliding does not occur, the reinforcement layer will aid in reducing lateral deformations of the embankment, thereby minimizing the risk of creating voids in the fill from cracking (3). In conventional design methodology, prevention of embankment failure due to lateral sliding is the first step (3,12).

The next step is to address foundation soil stability. External failure, or failure of the foundation soil, is said to occur as a

result of one of two mechanisms: slip surface failure or bearing capacity failure (3). Slip surface failure occurs when a portion of the embankment/foundation slides relative to the other stable part. If reinforced, the slip circle would likely go through the embankment, the reinforcement, and the foundation soil. A bearing capacity failure would result when the embankment punches through the foundation soil. This would occur in conventional cases when the reinforcement in the embankment is strong enough to hold the embankment together to prevent a slip surface from developing (3). In permafrost regions, the dynamic effect of frozen soil thawing under an existing embankment does not lend itself to such a structured approach. Failure of an embankment in these regions is likely to encompass a combination of all three failure mechanisms: lateral sliding, slip surface failure, and bearing capacity failure.

2.2 Initial Analysis

Verification of embankment stability as stated earlier must consider three failure mechanisms. For embankments constructed on permafrost, these failure mechanisms are interrelated. Once the frozen soil thaws under the side slopes of the embankment, bearing capacity is lost within the thaw-unstable talik. As the talik consolidates, failure planes may develop within the embankment. If a geotextile is present at the embankment/subgrade interface, the risk of lateral sliding on the outer part of the embankment increases and may add to the development of failure planes.

The evaluation of internal and external stability of the

embankment was initially evaluated using STABGM. The analyses of toe and deep stability were evaluated based upon none, one, two, three, and four layers of reinforcement placed within the embankment. Analysis of the embankment with no layers of geotextile reinforcement was also conducted to determine at what depth the most critical slip circle surface would be located (i.e., the depth of slip circle at which the lowest factor of safety was evaluated).

In addition to critical depths of slip circles, three distinct dimensions of soft thaw zone subgrade soil were also analyzed. The geometry of the soft thawed zone which created the most critical conditions (i.e., lowest factor of safety) in conjunction with the critical slip circle depth was then used throughout the remainder of the analysis. Once this critical depth and critical dimension of thaw zone was established, further analysis of stability with geotextile reinforcement was conducted.

Ideally, reinforcement layers should be placed in the direction of maximum tensile strain (3). For steep slopes, principal compressive strains are nearly vertical and principal tensile strains are nearly horizontal (3). Therefore, reinforcement layers placed horizontally provide a high degree of efficiency. Also, reinforcement is generally placed in horizontal layers to accommodate actual construction sequences.

2.3.1 Assumptions

Figure 2.4 depicts the conditions of the initial analysis. The following assumptions represent the initial analysis:

1. Height of embankment = 5 feet.

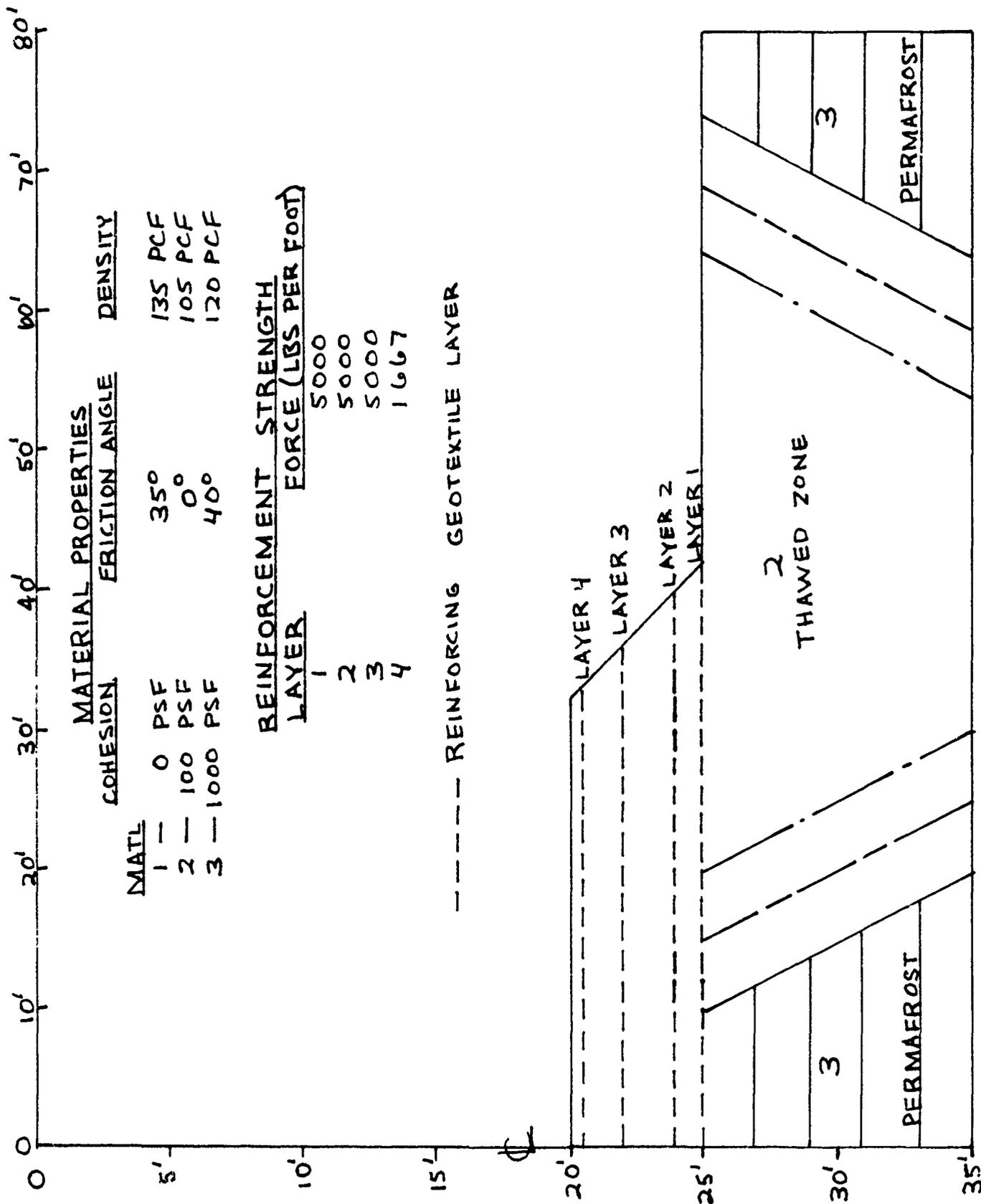


FIG. 2.4 STABGM INITIAL ANALYSIS

2. Slip circles analyzed were located tangentially at depths of 3, 5, and 10 feet below the bottom of the embankment.
3. Thaw zones (soft soil) were located 10, 15, and 20 feet from centerline, symmetric about the toe of the embankment.
4. No seismic activity was taken into account.
5. No pore pressures were taken into account (Total stress analysis).
6. Reinforced embankment stability analyses were conducted with geotextile layers located at 5, 4, 2, and 0.5 feet beneath the top of the embankment.
7. Material properties were modelled as crushed stone in the embankment, soft clay in the thaw zone, and stiff clay in the permafrost zone.

The geometry of the talik zones used in the analyses is consistent with measurements obtained by the Alaska Department of Transportation and Public Facilities (9,15,22). The material properties were modelled according to relative strength to each other. They do not represent actual material strengths. The embankment, crushed rock, was modelled as a complete unit and not separate layers of fill. The thaw zone was modelled as material with very little strength relative to the embankment and permafrost. Material properties selected are shown in Figure 2.4.

Embankment stability was initially analyzed with no layers of geotextile reinforcement. This was to evaluate at which depth and which dimensions of slip circles and thaw zones, respectively, provided critical conditions.

Analysis was then conducted with layers of geotextile reinforcement within the embankment. Embankment stability was then analyzed with one layer of reinforcement placed at the embankment/subgrade interphase. Additional analyses with two, three, and four layers of geotextile placed at 4, 2, and 0.5 feet beneath the embankment, respectively, were conducted to determine the impact on embankment stability. The geotextile reinforcement strength for the analyses with one, two, and three layers was 5,000 lbs per foot of width for each layer. The strength of the upper geotextile layer in the analysis with four layers of reinforcement was estimated to be 1667 lbs per foot of width. Mobilization of the full force in the geotextile layer close to the surface is not practical due to the minimal overburden pressure of the soil. Also, mobilization of the full force in all four layers was modelled to occur three feet from the end of each layer (i.e., zero force applied at endpoints and 5,000 lbs force applied at three feet in from each end for layer one).

2.3.2 Results of Initial Analysis

The analysis with no layers of reinforcement provided two important pieces of information which impacted the remaining analyses. First, the depth at which the critical slip circle occurred was 10 feet below the embankment/subgrade interphase (i.e., lowest factor of safety) (See Appendix A). Second, the dimensions of soft soil (thaw zone) at which critical slip circle conditions occurred were at both 10 and 15 feet from the centerline, symmetric about the toe (i.e., factors of safety were equal for both) (See Appendix A).

The analysis with three and four layers of reinforcement revealed that no appreciable gain in safety occurred as a result of the fourth layer.

2.3.3 Revisions

The following revisions were made to the initial conditions based upon the results obtained (See Figure 2.5):

1. Embankment depth was increased to seven feet.
2. Analysis was conducted with the thaw zone located 10 feet from centerline, symmetric about the toe of the embankment.
3. Analyses were conducted with none, one, two, and three layers of geotextile.
4. Stability analyses were conducted with slip circles located at a depth of 10 feet below the embankment (external stability) and through the toe of the embankment (internal stability).

The embankment depth was increased to seven feet to allow for a more conservative analysis. The greater the embankment height, the lower the factor of safety. Analyses of external stability were conducted to establish the required geotextile reinforcement to achieve a minimum factor of safety of 1.0. Analyses of internal stability were only conducted to compare factors of safety with external stability analyses.

2.4 Results of STABGM Analyses

The results of the revised analyses are summarized in Table 2.1. Geotextile reinforcement strengths of 5,000, 7,000, 9,000, and 8,000

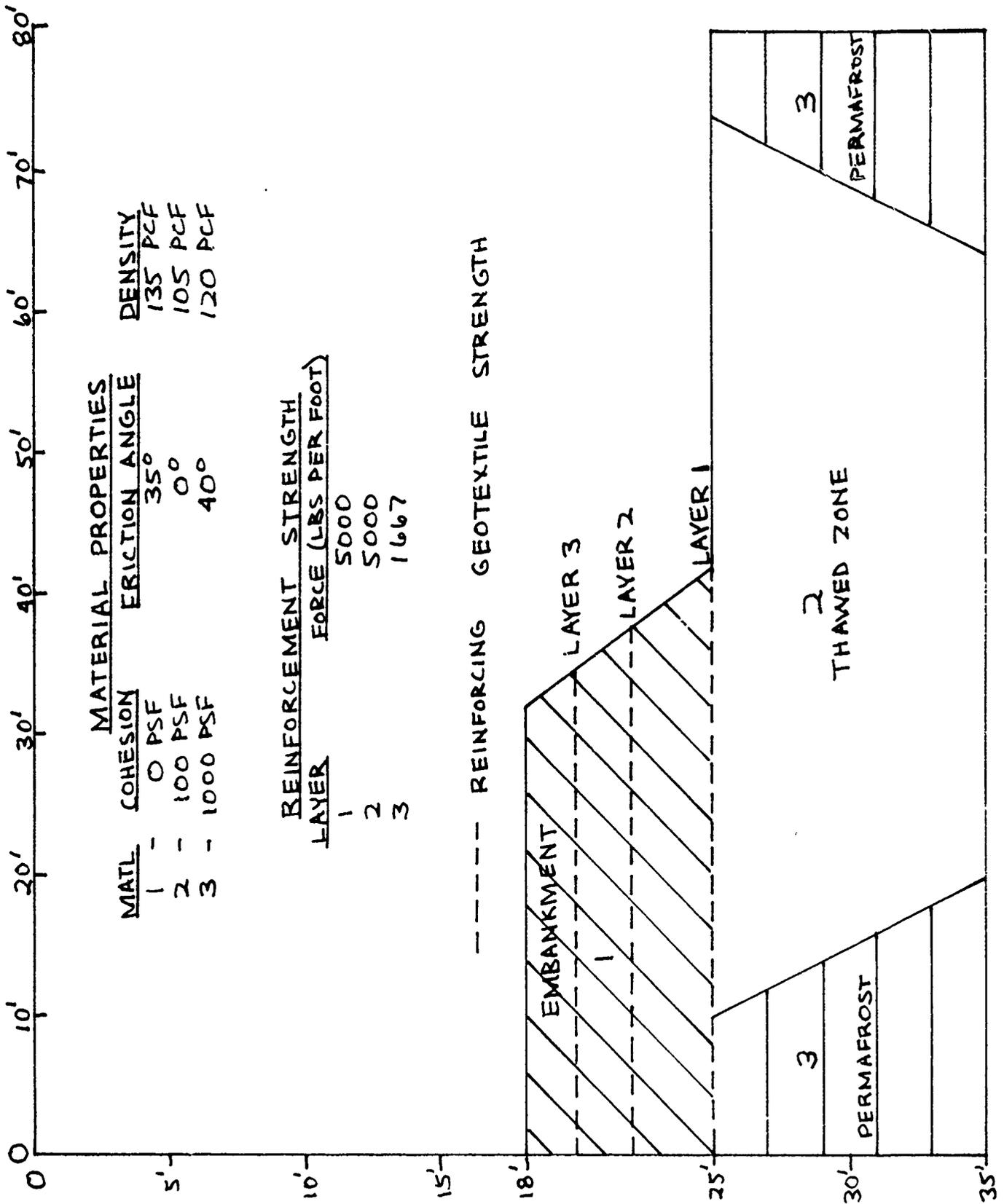


FIG. 2.5 STABGN. REVISED ANALYSIS

Table 2.1. Summary of Results

TANGENT SLIP CIRCLE:

<u>Number of Reinforcing Layers</u>	<u>Slip Circle Depth Below Embankment</u>	<u>Factor of Safety</u>	<u>Force of Layer</u>
None	3 ft	0.719	-
	5 ft	0.636	-
	10 ft	0.593 (critical)	-
One (7' below surface)	10 ft	1.043	8000 lbs
Two (4' and 7' below surface)	10 ft	1.035	5000 lbs
	10 ft	1.035	(each)
Three (2', 4' and 7' below surface)	10 ft	1.062	1666 lbs (Layer 3) 5000 lbs (Layer 1

TOE FAILURE:

<u>Reinforcing Layers</u>	<u>Factor of Safety</u>	<u>Force of Layer</u>
None	0.373	-
One (7' below surface)	0.598	5000 lbs
Two (4' and 7' below surface)	0.726	5000 (each)
Three (2', 4', and 7' below surface)	0.748	1666 lbs (Layer 3) 5000 lbs (Layer 1 and 2)

lbs per foot of width were analyzed to produce the strength required for a minimum factor of safety of 1.0 (See Appendix B). Analyses with two and three layers of reinforcement revealed that 5,000 lbs force was required to achieve a factor of safety of 1.0 (See Appendix B). It should be noted that with three layers of reinforcement, the top layer was modelled as mobilizing one third the strength due to the minimal overburden pressure. Also, there was minimal gain in factor of safety from the addition of the third layer. The analyses of internal stability revealed the layers of reinforcement strength required for a factor of safety of 1.0 in the external analyses were not strong enough to achieve the required minimum stability requirements (See Appendix B). These results indicate internal stability is more critical, however, the risk of failure for an embankment overlying soft material is more likely to occur in the soft subgrade soil (3).

CHAPTER III

NUMERICAL ANALYSES

Two computer programs were utilized to analyze the perplexing roadway conditions of the Farmer's Loop Road in Fairbanks, Alaska.

FEADAM84: A Computer Program for Finite Element Analysis of Dams was initially used to model existing roadway embankment conditions.

SSTIPN: Soil Structure Interactive Program was then used to model conditions for a reinforced roadway embankment constructed on permafrost.

3.1 FEADAM84 Analysis

FEADAM84 is an incremental finite element program for two-dimensional, plane strain analysis of earth and rockfill dams and slopes. It calculates the stresses, strains, and displacements due to incremental embankment construction and/or load application. The non-linear and stress history dependent stress-strain and volumetric strain properties of soils are approximated using a hyperbolic model modified by Seed and Duncan (1984).(8)

This program is a modified version of the program FEADAM developed by Duncan, Wong, and Ozawa (1980). The original incremental analysis procedure was coded in the program LSBILD by Kulhawy, Duncan, and Seed (1969). The subsequent program ISBILD, by Ozawa and Duncan (1973) incorporated (a) isoparametric elements with incompatible displacement modes developed by Wilson et al. (1971), (b) a more accurate

procedure for assigning initial stresses to elements, and (c) more efficient computational techniques, including a new equation solver, developed by Wilson (1971). The program FEADAM, by Duncan, Wong, and Ozawa (1980) incorporated (a) a new model for stress-dependent bulk moduli, and (b) new criteria for differentiation between primary loading behavior and stiffer, elastic unloading/reloading behavior. The unloading-reloading model was subsequently deleted by Wong and Seed (1982), who also modified the bulk modulus model.(8)

In its present form, FEADAM84 incorporates the following modifications to the earlier program FEADAM:

- (a) A modified version of the model for unloading/reloading behavior has been reinstated on an optional basis,
- (b) New criteria are employed for determination of whether a given soil element is in a state corresponding to primary loading or unloading-reloading behavior,
- (c) A modified lower bound constraint is incorporated in the stress-dependent bulk modulus model,
- (d) The new program has the capability to model weightless, linear elastic soil elements with zero initial stresses.

3.1.1 Non-linear Incremental Finite Element Methodology

A successive-increment procedure is used for approximating non-linear stress and stress history dependent behavior of soil, in which progressive loading is divided into a number of small increments, and the soil behavior is assumed to be linear within each increment. The modulus values used to model each soil element are re-evaluated each increment in accordance with (a) the stresses in the element, and (b)

the previous stress history of the element.

The incremental stress-strain relationship for an isotropic material under plane strain conditions may be expressed as follows:

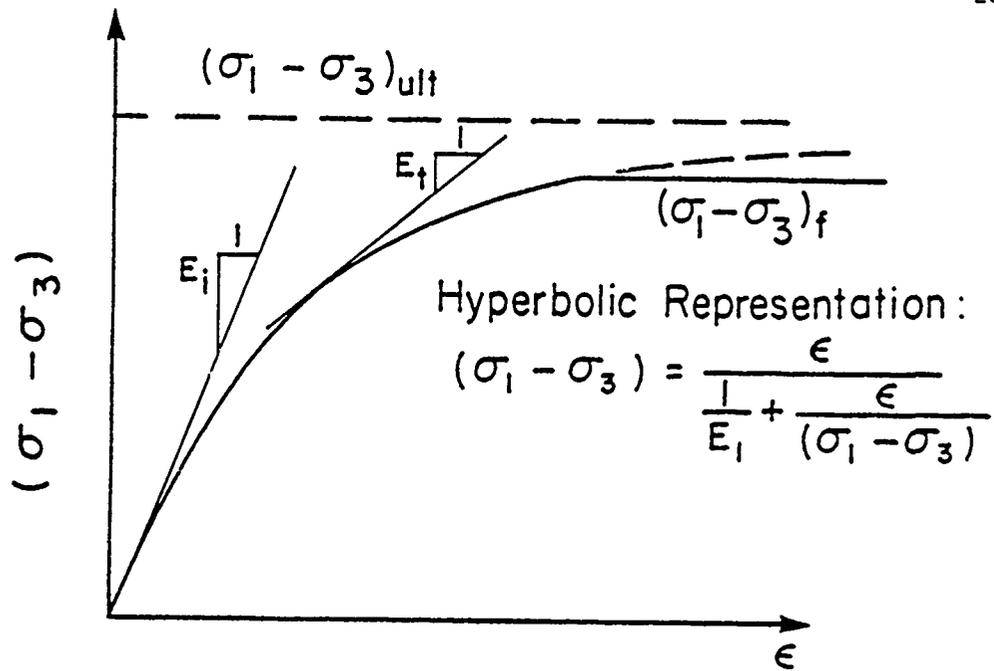
$$\begin{array}{rcccccc} \Delta\sigma_x & & (3B + E) & (3B - E) & 0 & \Delta\epsilon_x \\ \Delta\sigma_y & = \frac{3B}{9B - E} & (3B - E) & (3B + E) & 0 & \Delta\epsilon_y \\ \Delta\tau_{xu} & & 0 & 0 & E & \Delta\gamma_{xy} \end{array} \quad (3)$$

where $\Delta\sigma_x$ and $\Delta\sigma_y$ are the incremental stresses in the x and y directions respectively and $\Delta\tau_{xy}$ is the incremental shear stress. $\Delta\epsilon_x$ and $\Delta\epsilon_y$ are the incremental strains in the x and y directions respectively and $\Delta\gamma_{xy}$ is the incremental shear strain. E and B are Young's modulus and bulk modulus respectively. (1)

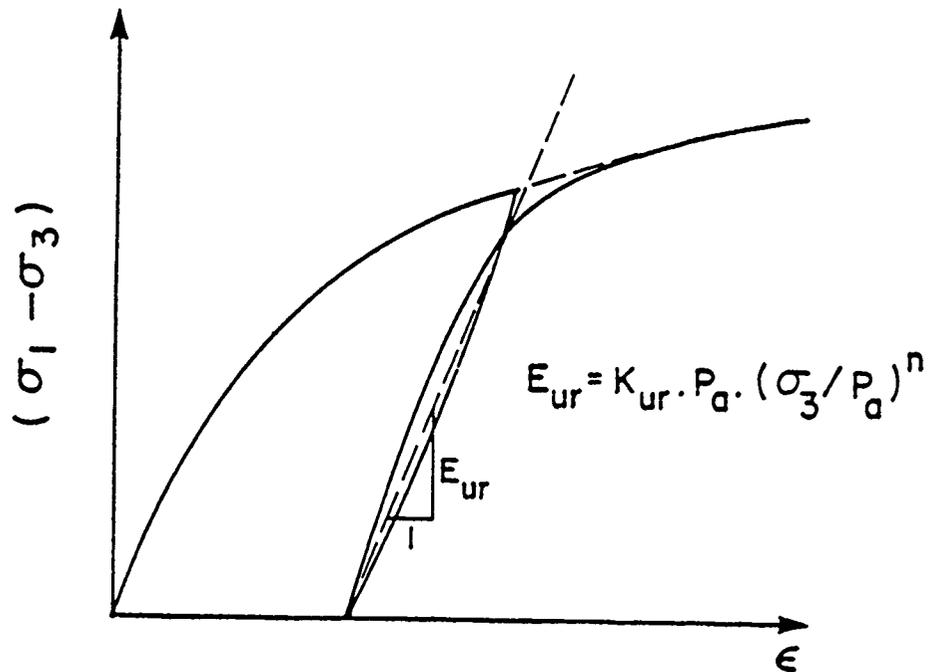
The non-linear, stress-dependent and stress-history-dependent soil behavior model is a modified version of the hyperbolic stress-strain, strength, and volumetric strain model proposed by Duncan et al. (1980). The original (1980) model has been modified in order to : a) provide improved modelling of soil behavior during unloading/reloading, b) eliminate a source of potential computational instability for some types of incremental loading paths, and c) provide improved modelling of bulk moduli at low stress levels and high confining stress.(8)

The original (1980) model assumes that stress-strain curves for soils at a given confining stress (σ_3) can be approximated as hyperbolas shown in Figure 3.1(a). The hyperbola in this figure can be represented by an equation of the form

$$(\sigma_1 - \sigma_3) = \frac{\epsilon}{\frac{1}{E_i} + \frac{\epsilon}{(\sigma_1 - \sigma_3)_{ult}}} \quad (4)$$

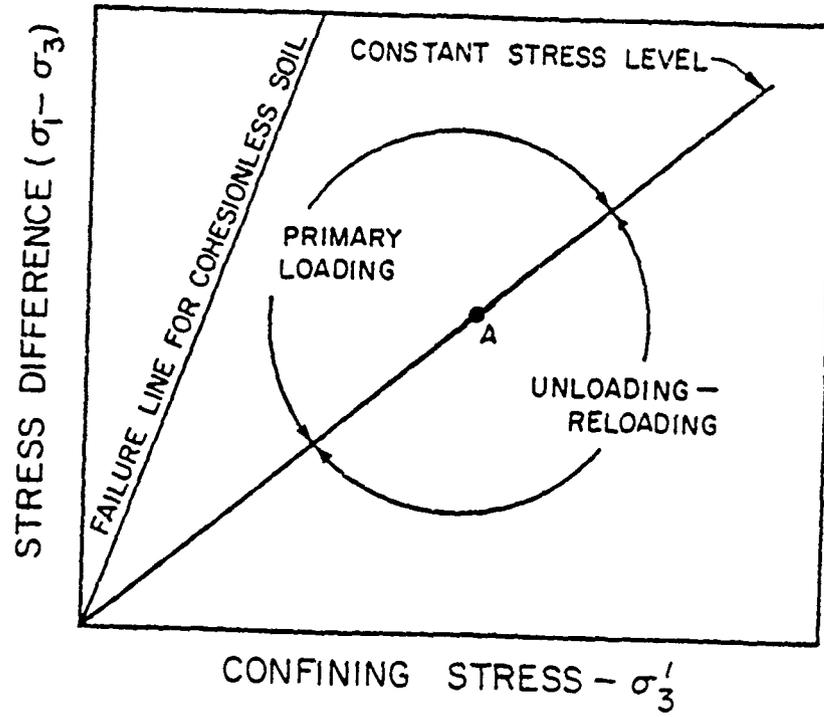


(a) Hyperbolic Representation of Stress-Strain Curve for Primary Loading.

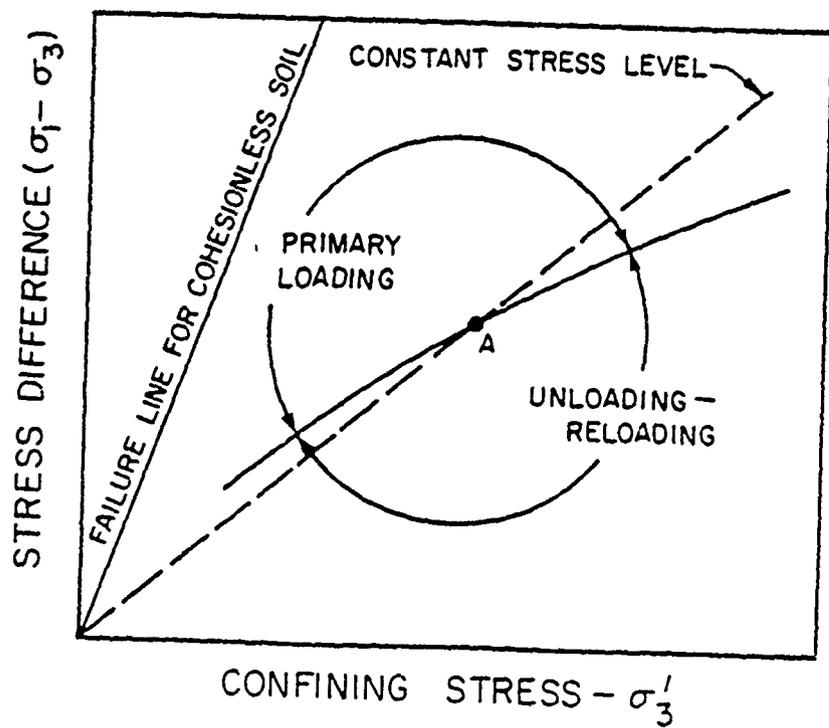


(b) Linear Unloading-Reloading Stress-Strain Relationship

FIG. 3.1 HYPERBOLIC MODEL FOR STRESS-STRAIN BEHAVIOR



a) Stress Level Criteria



b) Stress State Criteria

FIG. 3.2 COMPARISON BETWEEN STRESS LEVEL AND STRESS STATE CRITERIA FOR ASSIGNMENT OF UNLOADING-RELOADING MODULI

where E_i is the initial tangent modulus, and $(\sigma_1 - \sigma_3)_{\infty}$ is the asymptotic value of deviatoric stress. In order that the hyperbolic curve should fit the actual stress-strain curve of a given soil as closely as possible $(\sigma_1 - \sigma_3)_{ult}$ is always greater than the compressive strength of the soil $(\sigma_1 - \sigma_3)_f$. The two values are related by a constant failure ration (R_f) as

$$(\sigma_1 - \sigma_3)_f = R_f(\sigma_1 - \sigma_3)_{ult} \quad (5)$$

and R_f is typically between 0.6 to 0.9 for most soils. When the deviatoric stress exceeds $(\sigma_1 - \sigma_3)_f$, a failure condition is assumed as shown in Figure 3.1(a). The deviatoric stress at failure is determined by the familiar Mohr-Coulomb failure criteria as

$$(\sigma_1 - \sigma_3)_f = \frac{2c \cdot \cos\phi + 2\sigma_3 \cdot \sin\phi}{1 - \sin\phi} \quad (6)$$

where c and ϕ are the cohesion intercept and friction angle of the soil. The model allows variation of ϕ as a function of σ_3 as

$$\phi = \phi_0 - \Delta\phi \cdot \log_{10}\left(\frac{\sigma_3}{P_a}\right) \quad (7)$$

where ϕ_0 = the soil friction angle at confining stress of $\sigma_3 = P_a$,
 $\Delta\phi$ = the reduction in ϕ for each 10-fold increase in σ_3 , and
 P_a = atmospheric pressure, introduced in order to make the model independent of the system of units chosen.

The instantaneous slope of the hyperbolic stress-strain curve shown in Figure 3.1(a) is the tangent modulus E_t , which is a function of confining stress (σ_3) and stress level (SL), and which can be expressed as

$$E_t = [1 - (R_f \cdot SL)]^2 \cdot K \cdot P_a \cdot (\sigma_3/P_a)^n \quad (8)$$

where K, n = Model parameters (constants) relating to the initial modulus (E_i , see Figure 3.1(a)) to the confining stress σ_3 as

$$E_i = K \cdot P_a \cdot (\sigma_3/P_a)^n \quad (9)$$

and SL = Stress Level, defined as the ratio

$$SL = (\sigma_1 - \sigma_3)/(\sigma_1 - \sigma_3)_f \quad (10)$$

The tangent modulus modelled according to Equation 8 increases with increasing confining stress (σ_3) and decreases with increasing stress level (SL).(8)

When the current stress level was less than the previous maximum stress level ($SL_{\max \text{ past}}$), the unmodified (1980) model assumed the soil to be no longer in a state of primary loading, but rather in an unloading-reloading state. Unloading and reloading were modelled as linear and elastic as shown in Figure 3.1(b). The unloading-reloading modulus were modelled as a function only of σ_3 according to the equation

$$E_{ur} = K_{ur} \cdot P_a (\sigma_3/P_a)^n \quad (11)$$

where K_{ur} is typically 1.2 to 3 times greater than K (the modulus parameter determining E_i).(8)

The bulk modulus of the soil is assumed to be independent of stress level, and can be expressed as a function only of σ_3 as

$$B = K_B \cdot P_a \cdot (\sigma_3/P_a)^m \quad (12)$$

where K_B and m are dimensionless parameters (constants). Modelling the bulk modulus (B) as being independent of stress level results in modelling an increase in Poisson's ratio (ν) with increasing stress level because the soil modulus (E_t) decreases with increasing stress level, and Poisson's ratio can be expressed as

$$\nu = 1/2 - E_t/6B \quad (13)$$

The bulk modulus in the (1980) model was at all times constrained such that $E_t/3 < B < 17 E_t$, which in effect constrained Poisson's ratio such that $0.0 < \nu < 0.49$ (see Equation 13).(8)

Together, the soil modulus (E_t or E_{ur}) and the bulk modulus (B) define the stress-strain and volumetric strain behavior of a given material. This relatively simple, straightforward hyperbolic model may be formulated and applied in terms of either total stress (σ) or effective stress (σ') by using input parameters appropriate either for total or effective stress soil behavior.

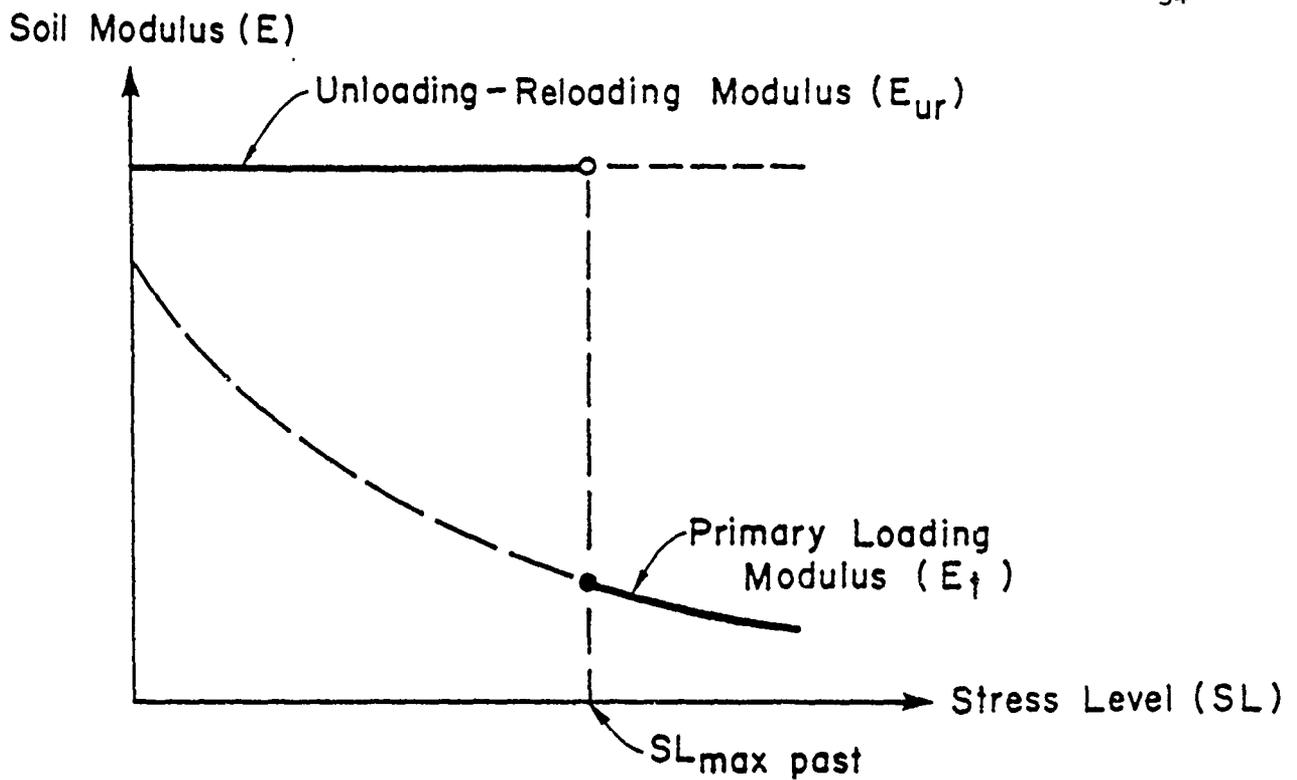
The unmodified (1980) model suffered from two problems concerning unloading-reloading.

The first of the two problems with the previous model for unloading-reloading moduli is that the simple stress level criteria used for differentiating between primary loading and unloading-reloading behavior does not appear adequate for many purposes. The line of constant stress level in Figure 3.2(a) represents the division, according to the earlier (1980) model, between primary loading and unloading-reloading stress paths for an element of soil currently existing at a stress condition represented by point A, where the stress level at A equals the maximum stress level achieved so far ($SL_{\max \text{ past}}$). This division between primary loading and unloading-reloading is defined by assuming unloading-reloading moduli for all conditions where $SL < L_{\max \text{ past}}$. Subsequent investigations have indicated that this simple stress level criteria for assignment of unloading-reloading moduli should be modified in order to include consideration of variations in the confining stress (σ_3) as well as variations in stress level. The new (modified) model therefore distinguishes between primary loading and unloading-reloading based on "stress state" (SS) defined as

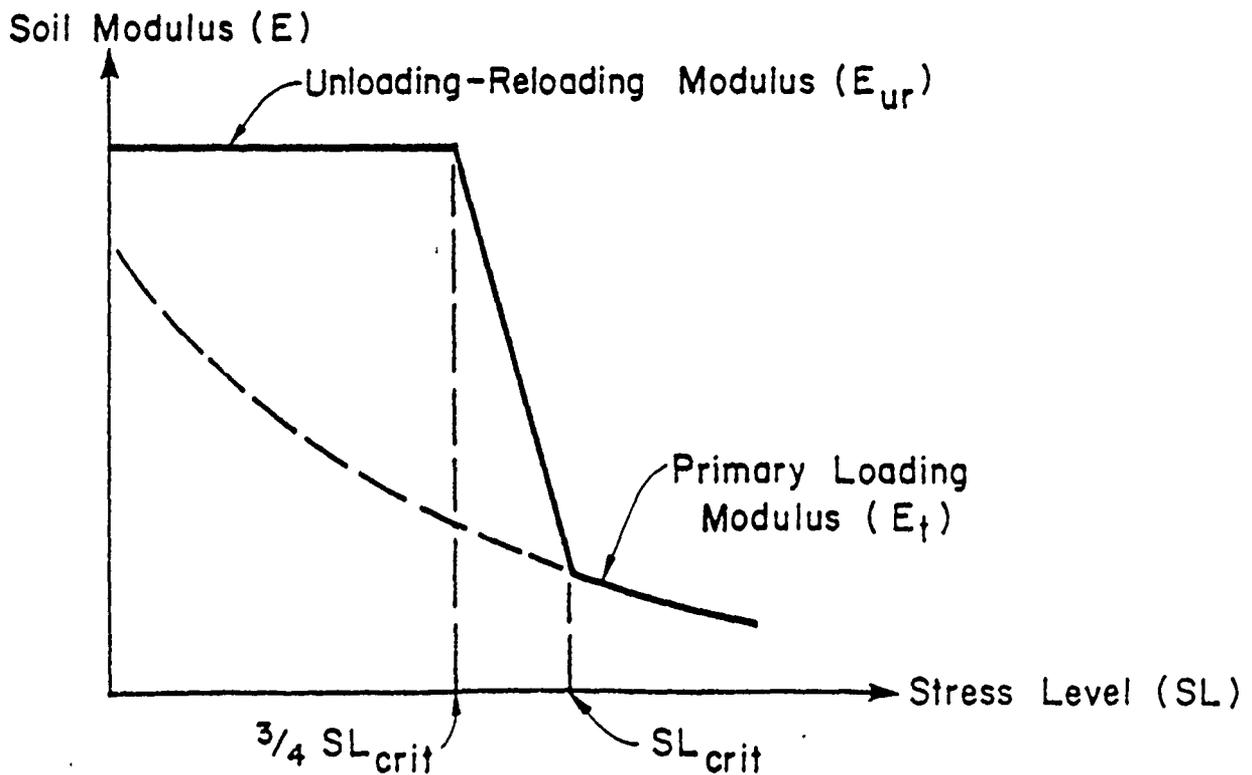
$$SS = SL \sqrt[4]{(\sigma_3/P_a)} \quad (14)$$

where $SL = \text{Stress level} = (\sigma_1 - \sigma_3)/(\sigma_1 - \sigma_3)_f$, and

$P_a = \text{atmospheric pressure}$, introduced in order to allow taking the fourth power radical of a dimensionless number.



a) Previous Model for Unloading-Reloading Moduli



b) Proposed Model for Unloading-Reloading Moduli

FIG. 3.3 MODELLING UNLOADING-RELOADING MODULI WITHOUT INTRODUCING COMPUTATIONAL INSTABILITY

The new model assumes primary loading moduli (E_t) are appropriate when the current stress state is greater than or equal to the maximum previous stress state ($SS > SS_{\max \text{ past}}$), and otherwise assumes unloading-reloading moduli (E_{ur}). Based on this criteria, the critical stress level (SL_{crit}) above which primary loading behavior is assumed for a given current confining stress (σ_3) may be calculated as

$$SL_{\text{crit}} = \frac{SS_{\max \text{ past}}}{\sqrt[4]{\sigma_3/P_a}} \quad (15)$$

Figure 3.2(b) illustrates the division between primary loading and unloading-reloading stress paths based on the new stress state criteria for the same initial conditions as were shown in Figure 3.2(a). The stress space is now divided by a line which is slightly concave downwards, and inclined slightly away from the constant stress level line.

The second, and most serious, problem with the previous model for unloading-reloading behavior is that it can lead to computational instability during incremental analyses. Figure 3.3(a) illustrates the soil moduli resulting from application of the unmodified 1980 model. At all stress levels greater than or equal to the maximum previous stress level ($SL_{\max \text{ past}}$) the primary loading modulus (E_t) is applied, and at all lower stress levels the unloading-reloading modulus (E_{ur}) is applied. The sharp discontinuity shown to occur in this figure at $SL_{\max \text{ past}}$ can result in a sudden change in modulus by a factor of more than 10 or 20 at this point. This can lead to computational instability during incremental finite element analyses, as soil elements incrementally subjected to loading at fairly constant stress levels may, as a

result of minor fluctuations in stress level, cross back and forth across the discontinuity experiencing large and unwarranted sudden changes in modulus. Attempts to minimize the impact of this source of potential computational instability by setting the boundary between primary loading and unloading-reloading at $0.95 SL_{\max \text{ past}}$ were unsuccessful.(8)

In order to prevent this potential source of computational instability, the new model provides for a less abrupt transition between unloading-reloading and primary loading moduli as illustrated in Figure 3.3(b), and described below:

1. The transition from unloading-reloading to primary loading is based on the new stress state criteria as described previously. Knowing the stress state ($SS_{\max \text{ past}}$) which defines the boundary between primary loading and unloading-reloading behavior, the stress level (SL_{crit}) at which primary loading begins for a given confining stress (σ_3) can be determined using Eq. 15.
2. When $SL > SL_{\text{crit}}$, the primary loading modulus (E_t) as determined by Eq. 8 is used.
3. When $SL < 3/4 SL_{\text{crit}}$, the unloading-reloading modulus (E_{ur}) as determined by Eq. 11 is used.
4. When $3/4 SL_{\text{crit}} < SL < SL_{\text{crit}}$, the modulus used is derived by linear interpolation between E_{ur} at $3/4 SL_{\text{crit}}$ and E_t at SL_{crit} , as shown in Figure 3.3(b).

This continuous linear transition between E_{ur} and E_t eliminates the sudden E_t/E_{ur} discontinuity, and thus reduces the associated

potential for computational instability inherent in the previous model. The price of this elimination of a source of computational instability is that the new model underestimates, to some extent, the soil modulus during the initial stages of the transition from primary loading to unloading behavior. This underestimation is, however, of much less significance than the previous instability. In addition, the new model (a) provides (correctly) for a modulus in the range $3/4 SL_{crit} < SL < SL_{crit}$ which is significantly higher than E_t during both unloading and reloading, (b) provides an appropriate modulus (E_{ur}) in the range $SL < 3/4 SL_{crit}$, (c) provides an appropriate modulus (E_t) in the range $SL > SL_{crit}$, and (d) provides an improved model for soil behavior during the transition from reloading to primary loading.(8)

The previous (1980) model had a tendency, under certain conditions, to underestimate the bulk modulus (B) and thus to underestimate minor principal stresses as well as soil strength and stiffness. This problem occurred most frequently in elements with very low stress levels and high confining stresses. In order to overcome tendency of the bulk modulus model to underestimate Poisson's ratio at low stress levels for some soils, the lower bound constraint on the new model has been modified such that

$$B_{min} > (E_t/3) \left\{ \frac{2 - \sin\phi}{\sin\phi} \right\} \quad \text{for } \phi > 2.3^\circ \quad (16)$$

$$\text{and} \quad B_{min} = 17 E_t \quad \text{for } \phi < 2.3^\circ \quad (16a)$$

This supercedes the unmodified (1980) lower bound constraint of $B_{\min} > E_t/3$ which constrained only against negative values of Poisson's ratio. When $\phi < 2.3^\circ$, ν_t is thus set equal to 0.49, as is appropriate for a cohesive soil under undrained conditions. For $\phi > 2.3^\circ$, this simple lower bound constraint on B has the effect of constraining Poisson's ratio be greater than or equal to ν_{\min} where

$$\nu_{\min} = \frac{1 - \sin\phi}{2 - \sin\phi} \quad (17)$$

3.1.2 Analysis Procedure

The program FEADAM84 calculates the stresses, strains, and displacements in embankments, simulating the actual sequence of construction operations and post-construction loading conditions. The non-linear and stress dependent stress-strain properties of the soils are approximated by performing the analysis in increments.

An increment may consist of the placement of a new layer on the embankment, or of application of loads to a complete embankment. Each increment is analyzed twice (a two-iteration process), the first time using modulus values based on the stresses at the beginning of the increment, and the second time using modulus values based on the average stresses during the increment. The changes in stress, strain, and displacement calculated during the second iteration of each increment are added to the stresses, strains, and displacements at the beginning of the increment to give the total or cumulative values up to that stage of the analysis.(8)

During each increment, each element is checked to determine if it is in a state of primary loading, unloading/reloading, tensile failure, or shear failure, and is modelled accordingly as follows:

- a) Primary Loading. Primary loading moduli are used when the stress state of an element is greater than the maximum previous stress state of the element, and the minor principal stress (σ_3) is positive. If σ_3 is positive but less than 0.05 times atmospheric pressure, the modulus values are computed using σ_3 equal to 0.05 atmospheric pressure. If no unloading-reloading elastic modulus number is specified for a given soil type, then all elements of that type are modelled with primary loading moduli regardless of stress state, unless the element is in a state of tensile or shear failure.(8)
- b) Elastic Unloading. Unloading-reloading modulus values (E_{UR}) are used when the current stress state of an element falls below the previous maximum stress state. Linear elastic (stress-state-independent) materials may also be modelled using the program FEADAM84, and these are never considered to be in an unloading-reloading condition.(8)
- c) Tensile Failure. Tensile failure occurs when σ_3 is negative. During the first iteration, the assigned bulk modulus (B) is computed using σ_3 equal to 0.1 times atmospheric pressure and the soil modulus (E) is set equal to one-tenth of the bulk modulus. If at the end of the first iteration the element is still undergoing tensile failure, both of the

moduli (E and B) are reduced to one-tenth of the values used in the first iteration. Linear elastic materials are not subject to tensile failure.(8)

- d) Shear Failure. Shear failure occurs when the stress level of an element exceeds 95% of its shear strength. Modulus values are modelled using Equation 6, but corresponding to a condition where the product of stress level (SL) times the failure ration (R_f) equals 0.95, resulting in very low elastic moduli. Linear elastic materials are not subject to shear failure.(8)

3.1.3 Initial Analysis

FEADAM84, in its present form, is not capable of calculating internal forces and displacements of structural elements such as a geotextile placed in a roadway embankment. However, FEADAM84 is capable of modelling the failure mechanisms and stress distributions of existing roadway embankments constructed on permafrost. Analysis of geotextile reinforced roadway embankments will be accomplished with the use of SSTIPN obtained from Dr. J. M. Duncan at Virginia Tech. (See Appendix C)

3.1.3.1 Summary of Analysis. Two material properties can be specified within the FEADAM84 program: (1) a linear elastic material which is not subject to shear failure and which can take tension, and (2) a stress dependent non-linear elasto-plastic material which obeys the hyperbolic stress-strain and volumetric relationships described in Section 3.1.1.

Linear-elastic materials are not applicable to this analysis and will not be discussed. For the stress dependent, non-linear elastoplastic soil materials, a total of nine parameters are required as summarized in Table 3.1.

Table 3.1 Summary of the Hyperbolic Parameters

Parameter	Name	Function
K, K_{ur}	Modulus number	Relate E_i and E_{ur} to σ_3
n	Modulus exponent	
c	Cohesion intercept	Relate $(\sigma_1 - \sigma_3)_f$ to σ_3
$\phi, \Delta\phi$	Friction angle parameters	
R_f	Failure ratio	Relates $(\sigma_1 - \sigma_3)_{ult}$ to $(\sigma_1 - \sigma_3)_f$
K_b	Bulk modulus number	Value of B/P_a at $\sigma_3 = P_a$
m	Bulk modulus exponent	Change in B/P_a for ten-fold increase in σ_3

All of these parameters can be evaluated from a series of triaxial compression tests. These procedures are described by Duncan and Wong (1974).

Three steps are required to accurately model the existing failure mode of the Farmer's Loop Road in Fairbanks, Alaska. First, the question of "What were the causes of roadway embankment failure?" must be answered. Second, an appropriate representation of the roadway embankment geometry including foundation soil must be defined. Third, an accurate representation of the roadway embankment performance through the selection of soil parameter values to be input into the program.

The failure mechanisms associated with roadway embankments overlying weak foundations have been described in Sections 1.1 and 2.2. The failure of the Farmer's Loop Road is directly related to the seasonal thawing of the frozen foundation soil underlying the side slopes of the embankment. This degradation can be further amplified by the insulating effect of snow over the sideslopes which prevents the complete freeze-back of the active layer (13). The consequential consolidation of the thaw unstable permafrost results in differential settlement of the roadway embankment showing up as longitudinal cracks along the wearing surface (13, 14), as shown in Figure 1.1.

The roadway embankment geometry selected for the numerical analyses (See Figure 3.4) is taken from the State of Alaska Department of Transportation and Public Facilities (DOT&PF) Typical Cross Section of Farmer's Loop Road (See Figure 3.5) and the Engineering Geology and Soils Report (1985). The actual roadway embankment geometry varies along the length of the road and is a function of the amount of cut and fill, grade, and superelevation of curves. The geometry selected in the analysis represents a typical section along the road.

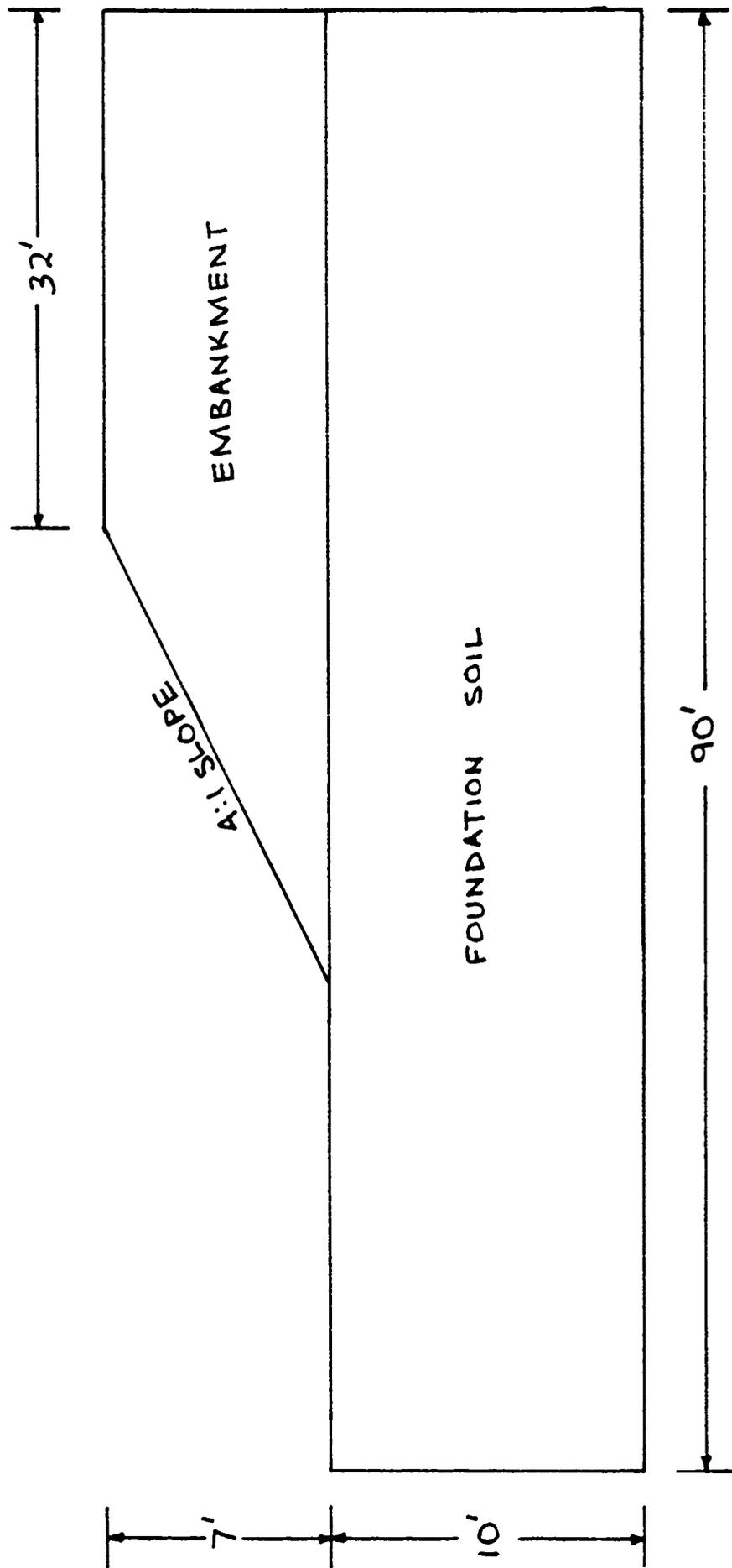
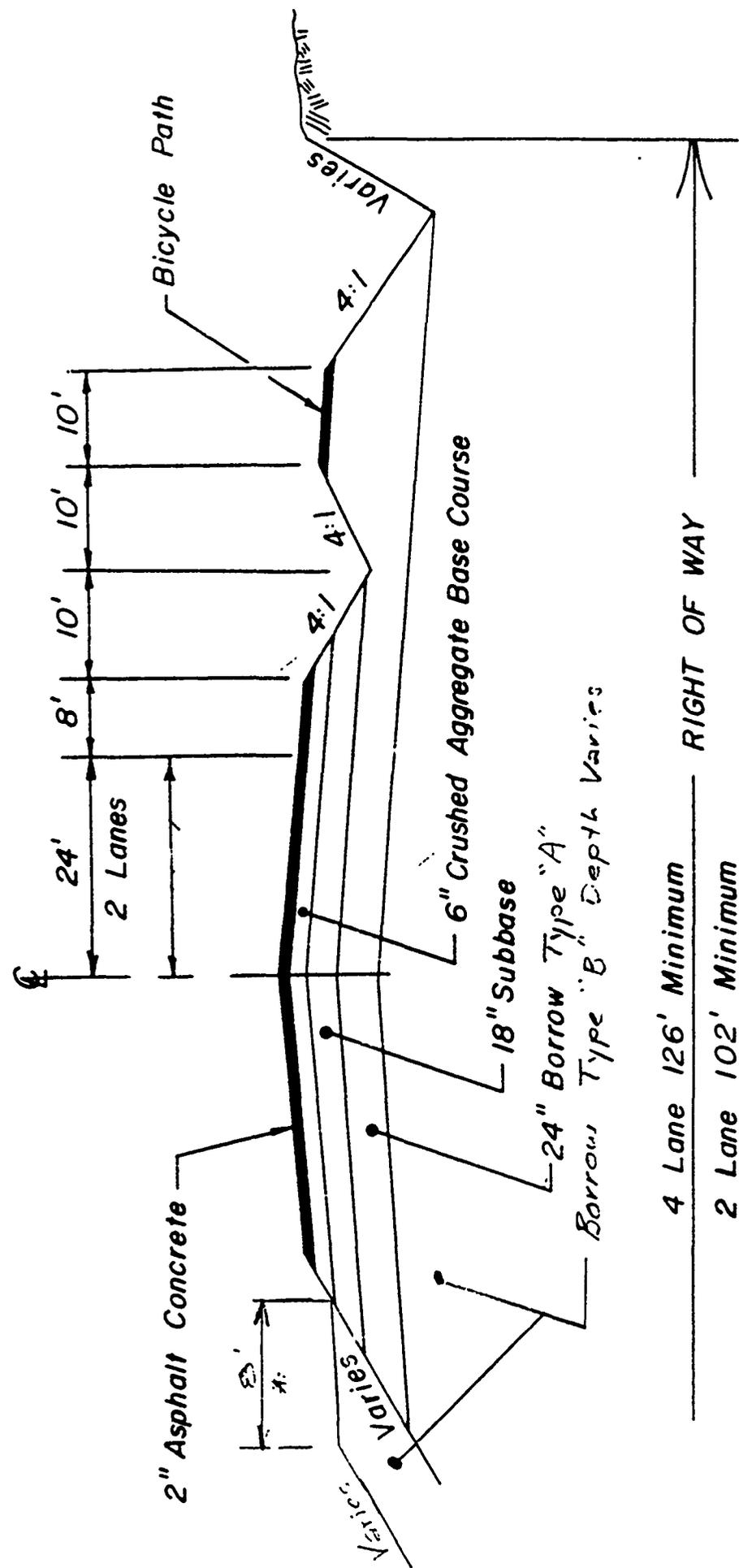


FIG. 3.4 EMBANKMENT GEOMETRY USED IN NUMERICAL ANALYSES



4 Lane 126' Minimum
 2 Lane 102' Minimum
 RIGHT OF WAY

TYPICAL SECTION
 Farmers Loop Road 0 to 8 Mile
 RS - 0644 (13)

* Studying Form

FIG. 3.5

Modelling embankment performance is primarily dependent upon the proper selection of soil parameter values to be evaluated by the program. The soil parameters used in the hyperbolic model may be obtained from triaxial compression tests. Three soil types are used in modelling embankment performance in both FEADAM84 and SSTIPN. The roadway embankment is modelled as one type for convenient use in the program. Permafrost and thaw-unstable permafrost parametric values are estimated from Duncan et al. (1974) due to insufficient availability of test data on these types of soil materials. Since actual data are not available for the thaw-unstable permafrost, the objective is to obtain relative strength data for the three soil types used to model existing conditions. The three soil types modelled are crushed rock for the embankment, soft clay for the thaw unstable permafrost, and hard clay for the permafrost. The parametric values obtained for the crushed rock were obtained from Schauz (1981). The data obtained for soft and hard clay were obtained from Duncan et al. (1974). The values obtained for all three soil types are summarized in Table 3.2.

Table 3.2 Soil Parametric Values Used in FEADAM84 and SSTIPN

<u>Parameter</u>	<u>Crushed Rock</u>	<u>Soft Clay</u>	<u>Hard Clay</u>
K	6000.0	40.0	1000.0
K_{ur}	8000.0	80.0	1500.0
n	0.5	0.3	0.4
R_f	0.5	0.9	0.7
K_b	1500.0	20.0	500.0
m	0.6	0.2	0.5
c	0 psf	100 psf	1000 psf
ϕ	35°	0°	40°
$\Delta\phi$	0°	0°	0°
γ	135 pcf	106 pcf	120 pcf
K_o	0.5	0.5	0.5

The relative locations of the soil types and mesh used in the finite element analysis are shown in Figure 3.6. Location of the talik zone used in the analyses is consistent with data obtained by the State of Alaska DOT&PF (13, 14, 19). For most cases reasonably accurate results can be achieved by using eight or more layers in the mesh, and adding one layer at a time to the embankment. However, six layers were used to investigate the relative accuracy of the model with that of Figure 1.1. Six layers also allowed more flexibility in areas which required some refinement (See Figure 3.7). Figure 3.8 represents the deformed state of the original model. The failure mode associated with the Farmer's Loop Road can be detected in Figure 3.8. The deformed geometry plot is performed using Program GRAPH (See Appendix D). The consolidation of talik zone and severe surface deformation of the top of the embankment are clear. Through analysis of this figure, the material properties are reasonably accurate for further use in SSTIPN.

3.2 SSTIPN Analysis

SSTIPN calculates stresses, strain and displacements in soil elements and internal forces and displacements in structural elements by means of analyses which simulate the actual sequence of construction. The non-linear and stress-dependent stress-strain properties of the soils are approximated using the procedures developed by Kulhawy, Duncan, and Seed (1969). The structural materials are assumed to behave linearly. The soil model used is predecessor to the existing soil model used in FEADAM84. The purpose of utilizing SSTIPN is to analyze the effects of placing different structural materials within a

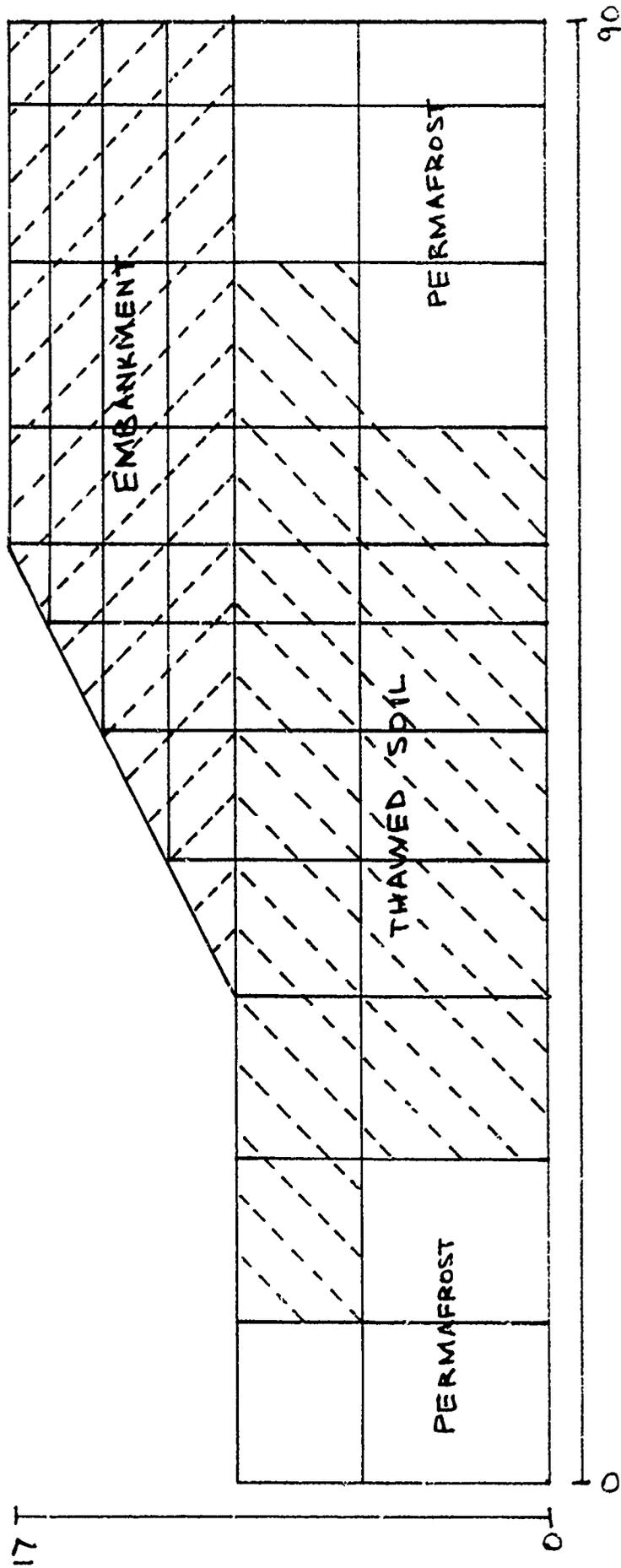


FIG. 3.6 FEADAMB4 EMBANKMENT MODEL

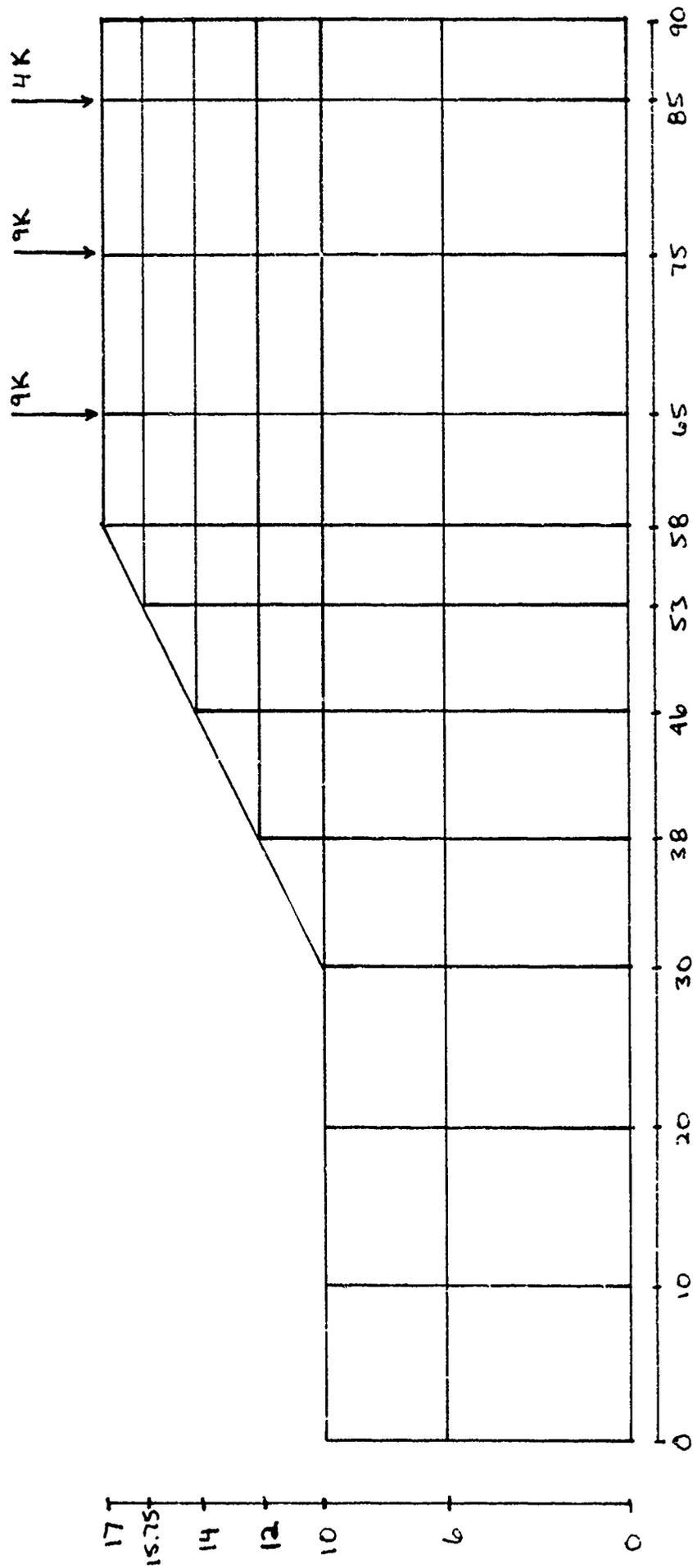


FIG. 3.7 FEADAM84 EMBANKMENT GEOMETRY

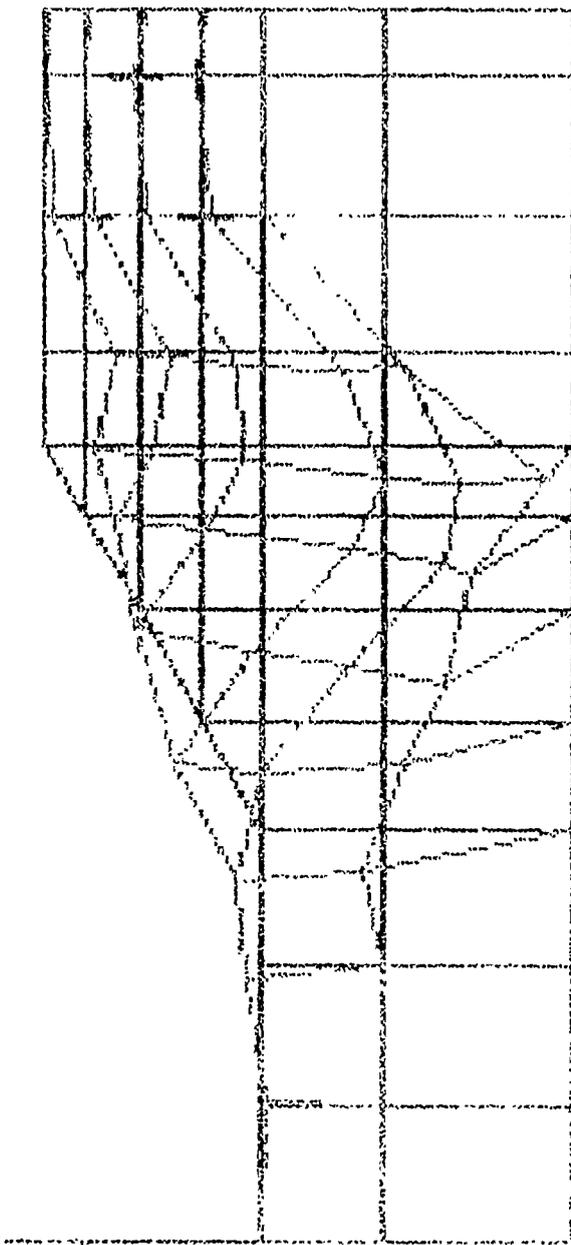


FIG. 3.8 DEFORMED EMBANKMENT GEOMETRY OF FEADAM84 ANALYSIS

roadway embankment constructed on permafrost.

3.2.1 Assumptions

The results of FEADAM84 reveal some important points. First, the soil parameters selected for the analyses are appropriate and accurately model roadway embankment performance in permafrost regions. The deformed roadway embankment model, Figure 3.8, depicts the distressed condition as a result of thaw-unstable foundation soil located beneath the side slopes of the embankment. Second, the geometry and location of the soil types are relatively accurate within the model. The relative location of the soil material properties, Figure 3.6, allows the roadway embankment model to behave consistently with DOT & PF articles (9, 21).

The conditions of the SSTIPN analyses are as follows:

1. Embankment geometry is identical to both STABGM and FEADAM84 analyses. Foundation soil height = 10 feet. Embankment soil height = 7 feet. Total number of soil elements if increased from 48 to 78 elements.
2. Five different strength modulae are analyzed for the geotextile properties. In addition, five different conditions are analyzed for each separate geotextile strength modulus.
3. Soil/geotextile interface is modelled for no slip between soil elements and structural elements.

Figure 3.9 depicts the mesh that is used in the SSTIPN analyses. The foundation and embankment numbers of layers are increased to three

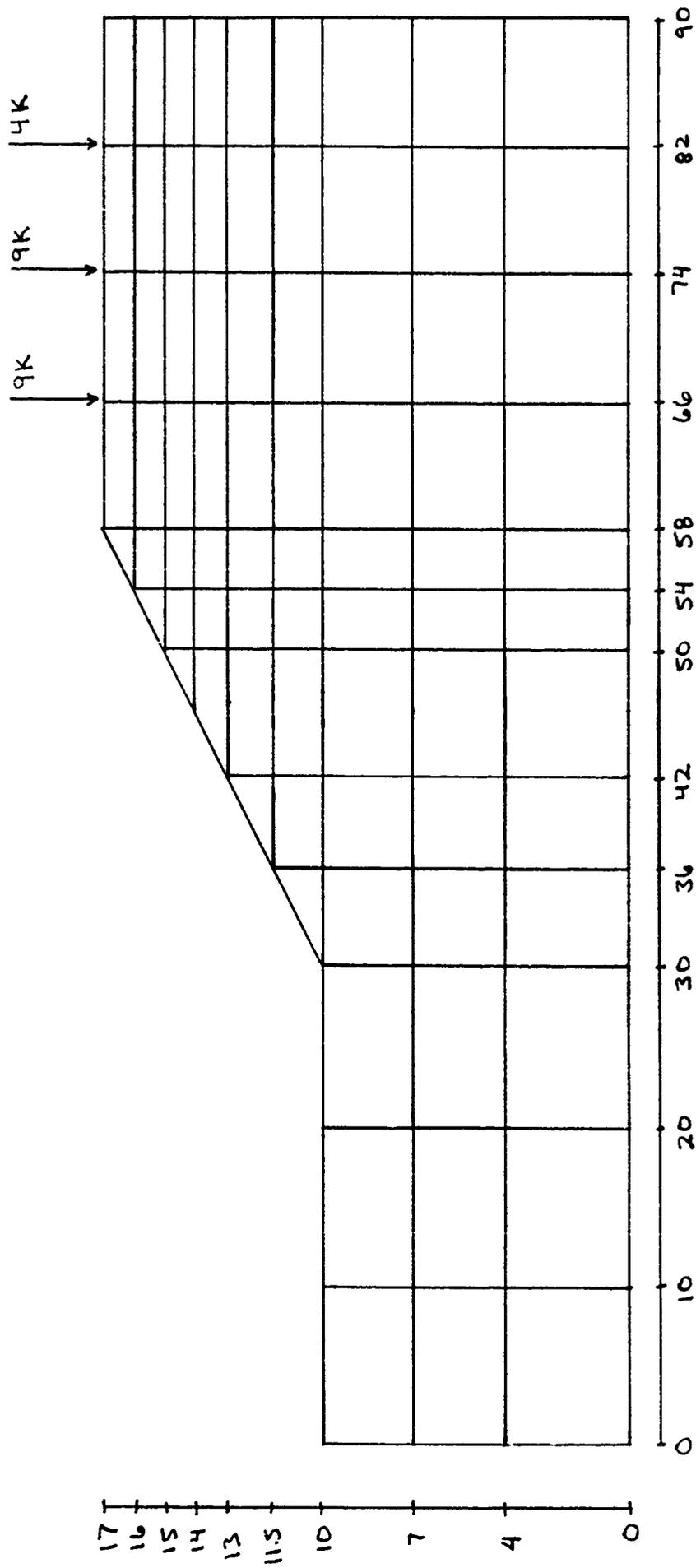


FIG. 3.9 SSTIP EMBANKMENT GEOMETRY

and six, respectively to provide more accurate results of displacements and stresses. Figure 3.10 depicts the location of the three soil material types in the analyses. The location of the thaw-unstable material beneath the side slopes is consistent with articles from the state of Alaska DOT&PF (9, 15, 21).

The five geotextile strength modulus are displayed in Figure 3.11. Tensile strengths of 1000, 2000, and 4000 pounds per inch with strains of 5 and 20 percent are evaluated. SSTIPN does not differentiate between modulae with different strain. SSTIPN calculates the stiffness of each member in force per unit width without consideration to percent elongation. Therefore, a material with strength of 1000 pounds per inch at 5 percent strain is identical to a material with a strength of 4000 pounds per inch at 20 percent strain.

The five conditions analyzed for each strength modulae are as follows:

1. Three layers of geotextile are located within the embankment. The first layer located at the embankment/foundation interface and the second and third layers located three and five feet above the embankment/foundation interface, respectively.
2. Three layers of geotextile with identical locations as the first condition with a pre-tension of 1000 pounds per foot on all three layers.
3. One layer of geotextile located at the embankment/foundation interface.
4. One layer of geotextile located at the embankment/foundation

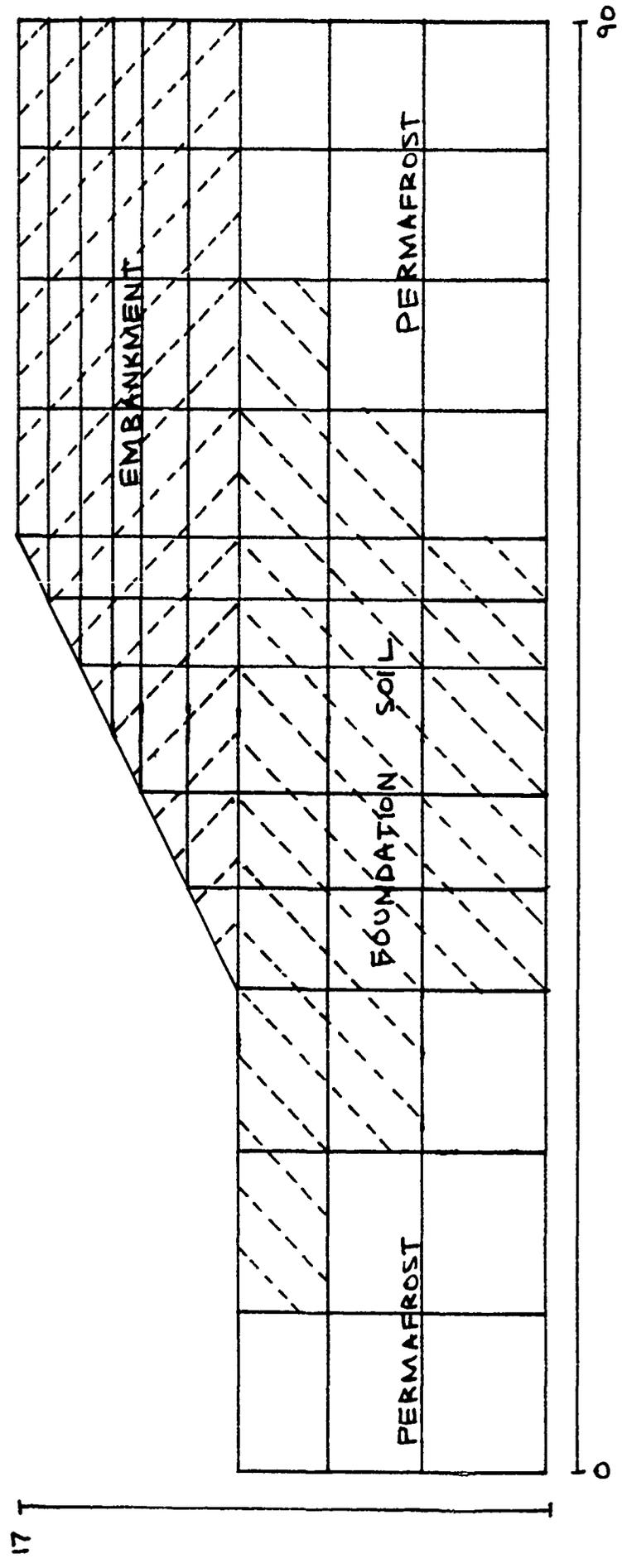
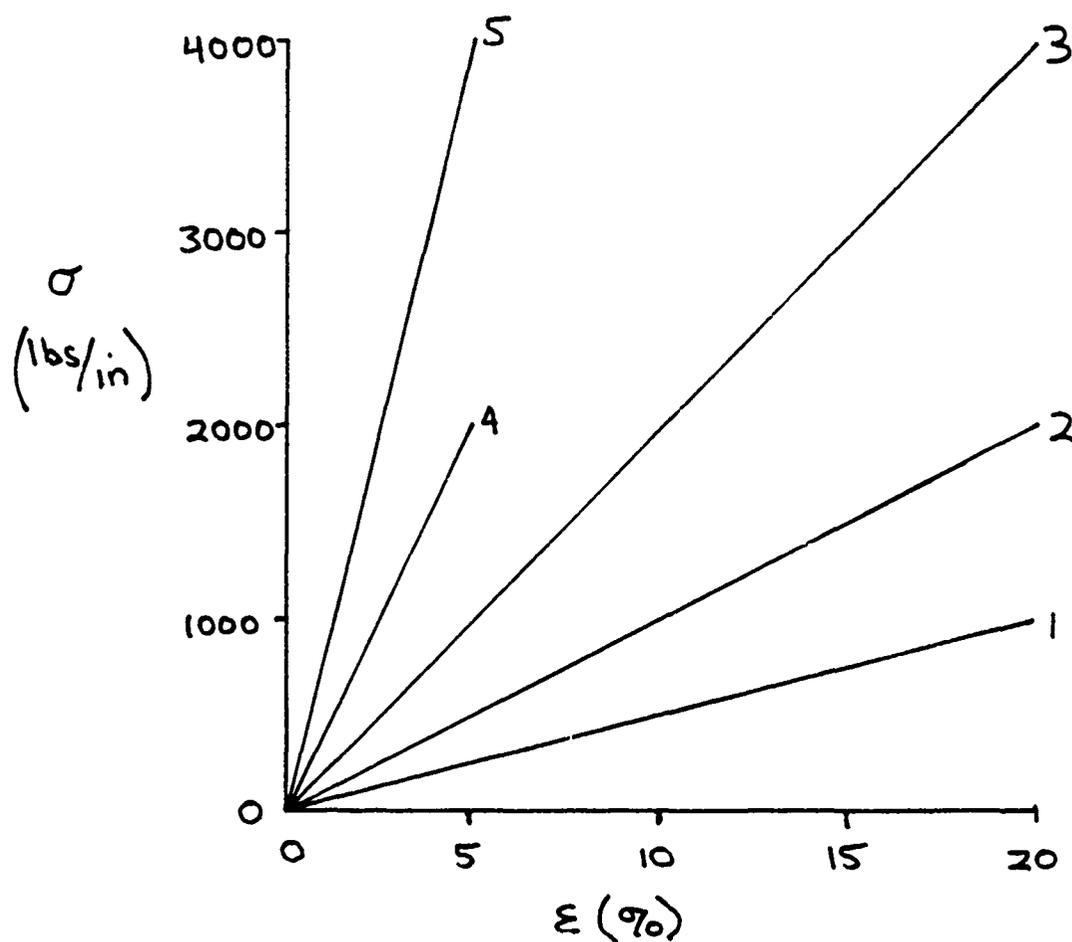


FIG. 3.10 SSTIPN EMBANKMENT MODEL



<u>MODULUS</u>	<u>LOAD (σ)</u> <u>(lb/in)</u>	<u>STRAIN (ϵ)</u> <u>(%)</u>	<u>STIFFNESS</u> <u>(lbs/ft of width)</u>	<u>TONS/ft of width)</u>
1	1000	20	60,000	30
2	2000	20	120,000	60
3	4000	20	240,000	120
4	2000	5	480,000	240
5	4000	5	960,000	480

$$\text{STIFFNESS} = \frac{(1000 \text{ lb/in})(12 \text{ in/ft})}{(.01)(20\%)} = 60,000 \text{ lbs/ft of width}$$

FIG. 3.11 GEOTEXTILE STRENGTH MODULAE

interface with a pre-tension of 1000 pounds per foot.

5. One layer of geotextile located at the embankment/foundation interface with a pre-tension of 1000 pounds per foot. The geotextile extends 10 feet beyond the toe of the embankment. SSTIPN has the capability of modelling friction between soil and structural elements. The geotextile/soil interface is modelled with total friction, i.e., no slip between the soil and geotextile. The overall accuracy increases only by about 5 percent with the addition of friction elements. Modelling without friction elements is convenient for repetitive analyses and does not reduce the accuracy enough to cause a significant difference.

3.2.2 Results of SSTIPN Analysis

The analysis of SSTIPN results includes obtaining displacement and stress data from the output to evaluate the effect of each different modulae given the different conditions. The analysis of the roadway embankment with three layers of geotextile did not provide reliable results due to instability of SSTIPN in calculating displacements. Therefore, analyses of the five modulae with the first two conditions mentioned will no longer be discussed.

Displacement data extracted from SSTIPN output for all five different strength modulae with the last three conditions are plotted using the GRAPH in Appendix D. Figures 3.12 through 3.27 depict the deformed geometry for each case. Figures 3.28 through 3.43 the deviatoric stress distribution for the identical conditions used to plot the deformed geometry.

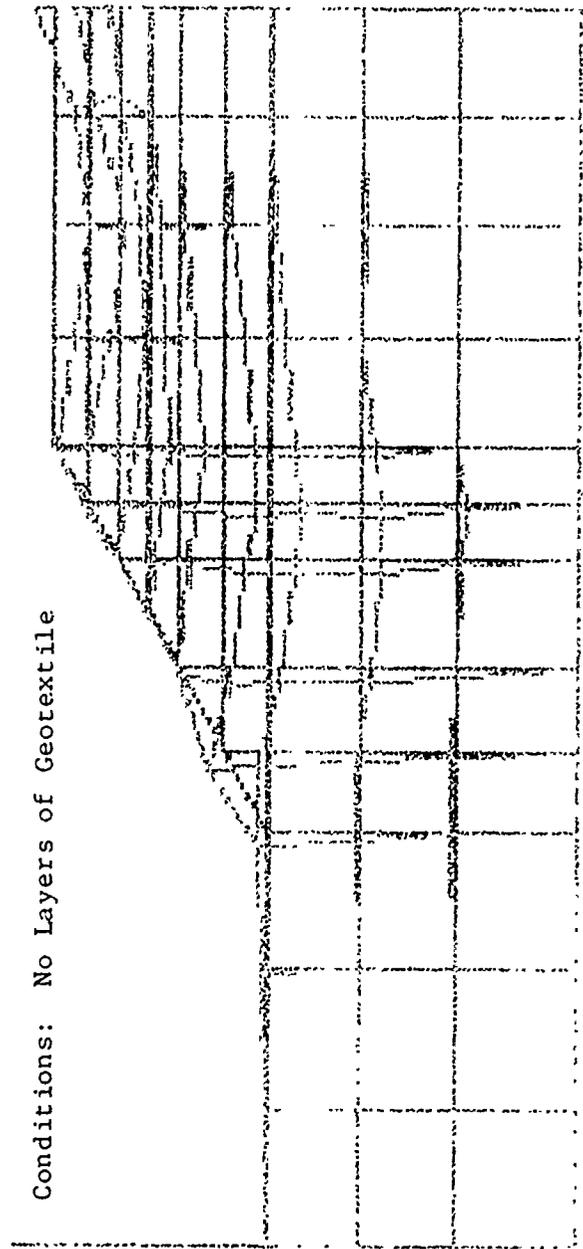


FIG. 3.12 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

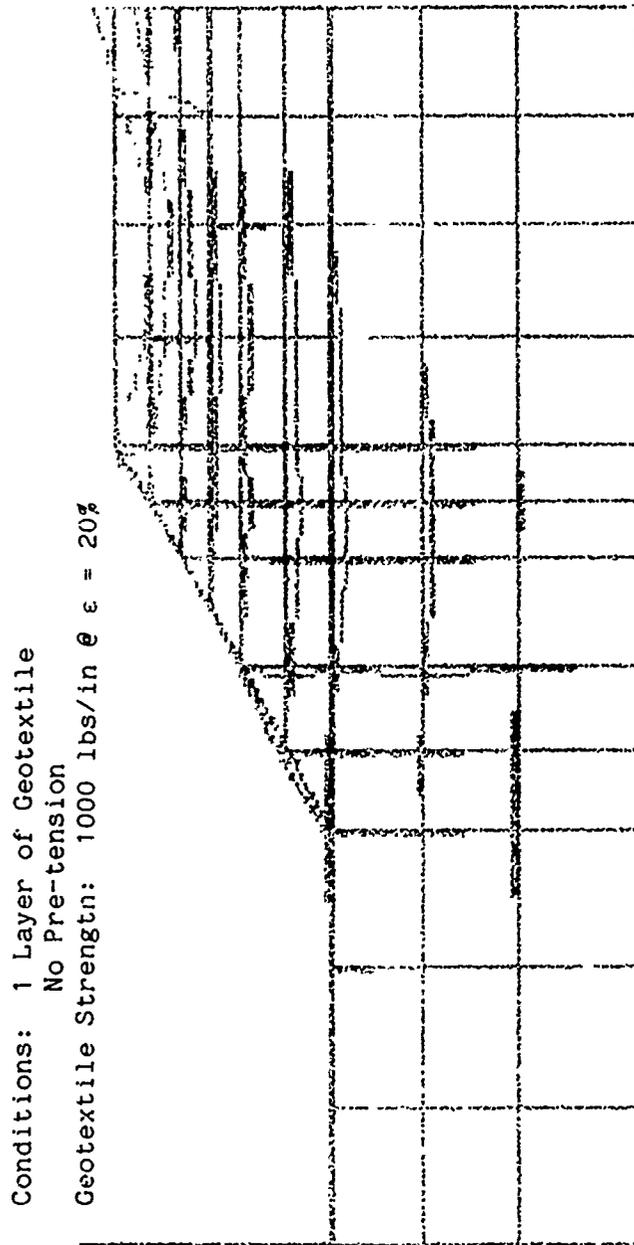


FIG. 3.13 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
Pre-tension = 1000 lbs/ft
Geotextile Strength: 1000 lbs/in @ $\epsilon = 20\%$

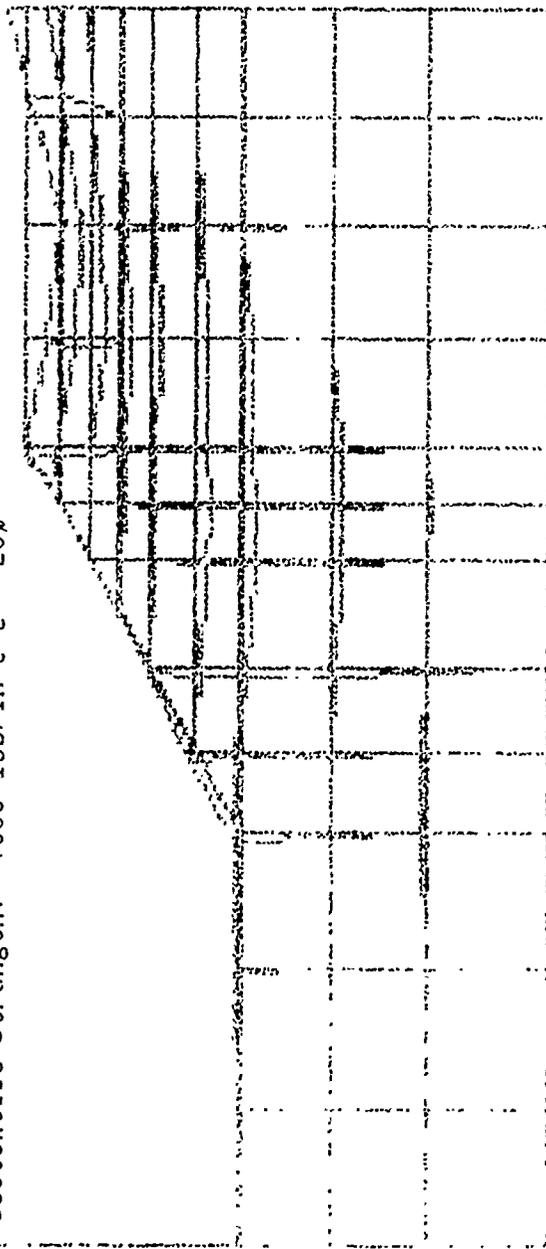


FIG. 3.14 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile Extended 10 Ft
Beyond Toe of Embankment
Pre-tension = 1000 lbs/ft
Geotextile Strength: 1000 lbs/in @ $\epsilon = 20\%$

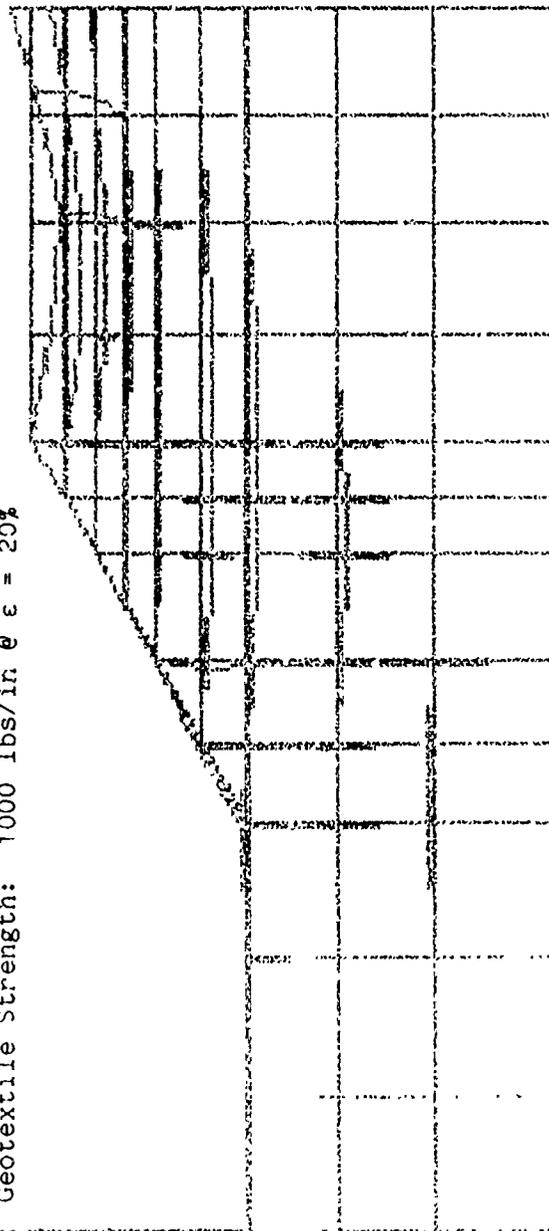


FIG. 3.15 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
No Pre-tension
Geotextile Strength: 2000 lbs/in @ $\epsilon = 20\%$

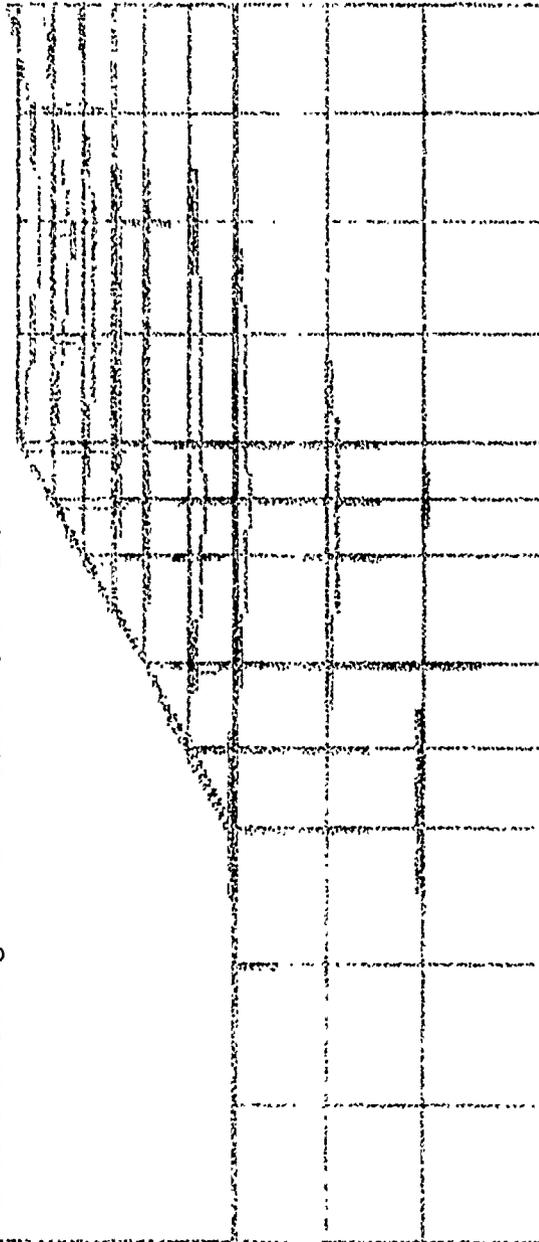


FIG. 3.16 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
Pre-tension = 1000 lbs/ft
Geotextile Strength: 2000 lbs/in @ $\epsilon = 20\%$

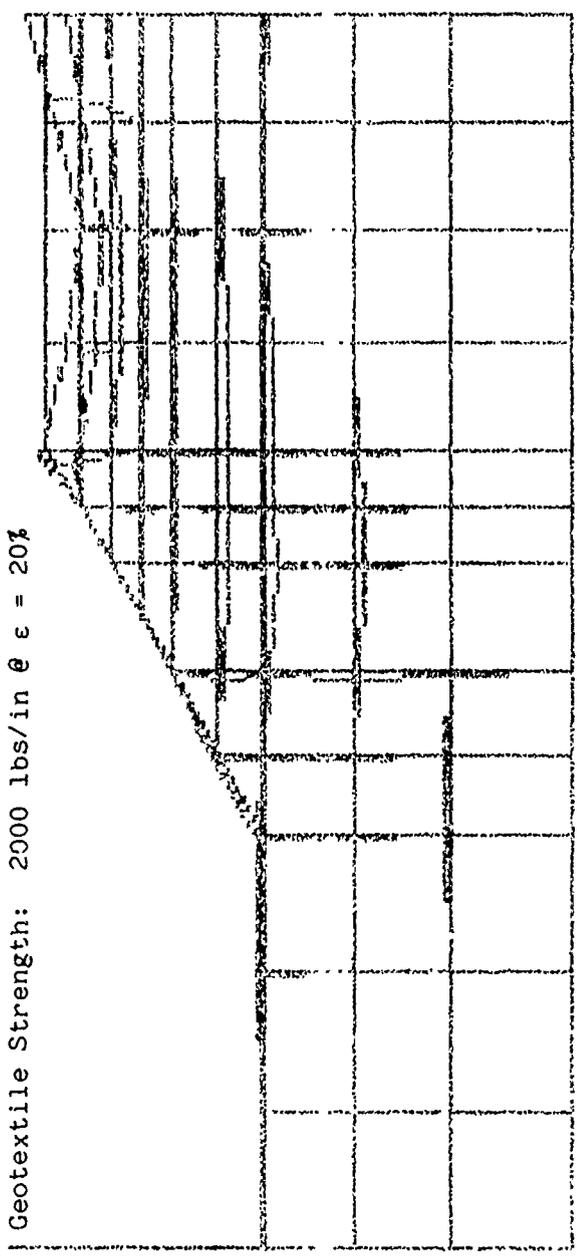


FIG. 3.17 DEFORMED EMBANKMENT GEOMETRY OF SSIIPN ANALYSIS

Conditions: 1 Layer of Geotextile Extended 10 Ft
Beyond Toe of Embankment
Pre-tension = 1000 lbs/ft
Geotextile Strength: 2000 lbs/in @ $\epsilon = 20\%$



FIG. 3.18 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
No Pre-tension
Geotextile Strength: 2000 lbs/in @ $\epsilon = 5\%$

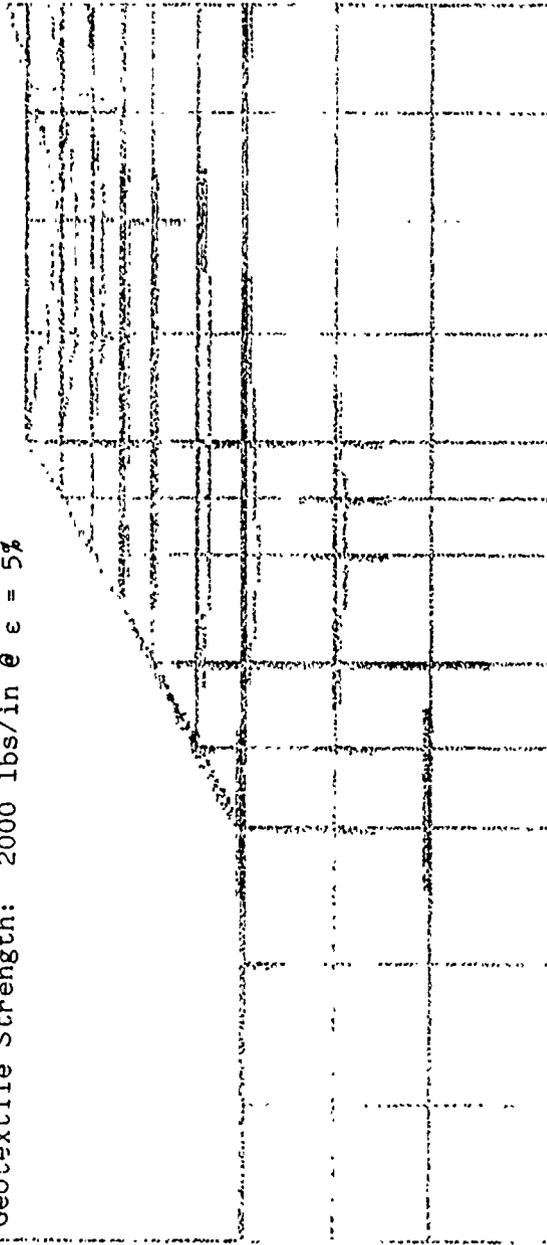


FIG. 3.19 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
Pre-tension = 1000 lbs/ft
Geotextile Strength: 4000 lbs/in @ $\epsilon = 20\%$

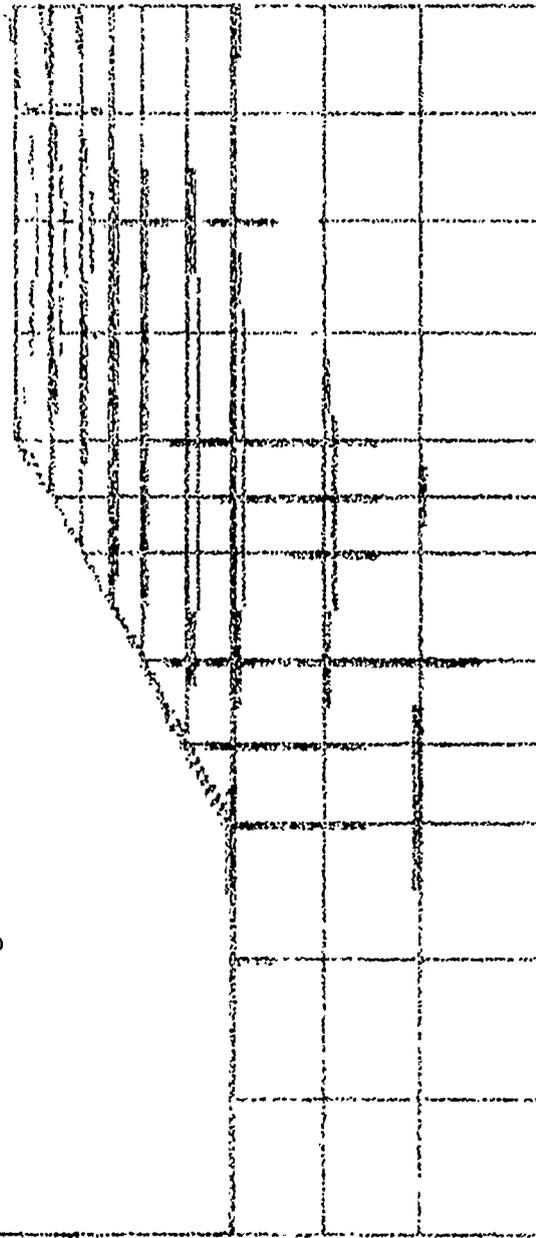


FIG. 3.20 DEFORMED EMBANKMENT GEOMETRY OF SST:PN ANALYSIS

Conditions: 1 Layer of Geotextile Extended 10 Ft
Beyond Toe of Embankment
Pre-tension = 1000 lbs/ft
Geotextile Strength: 4000 lbs/in @ $\epsilon = 20\%$

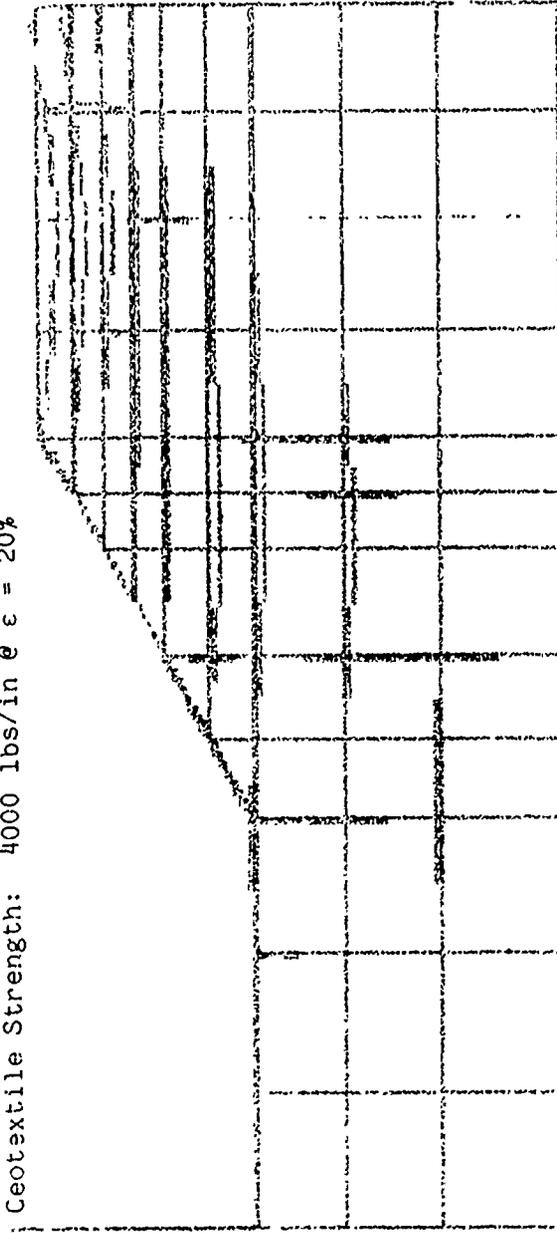


FIG. 3.21 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
No Pre-tension
Geotextile Strength: 4000 lbs/in @ $\epsilon = 20\%$

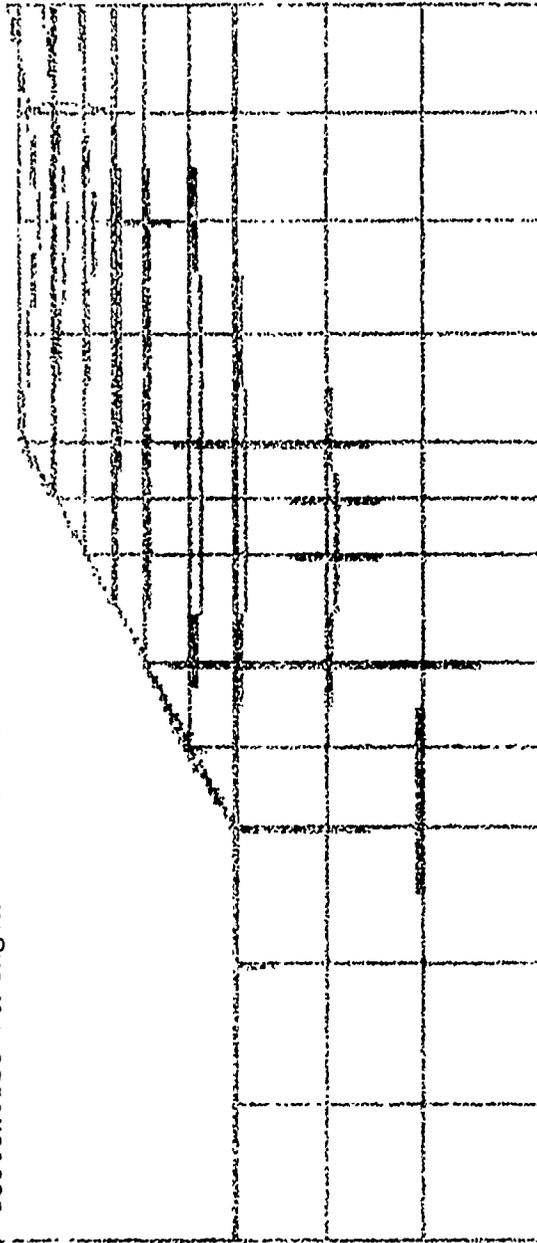


FIG. 3.22 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
Pre-tension = 1000 lbs/ft
Geotextile Strength: 2000 lbs/in @ $\epsilon = 5\%$

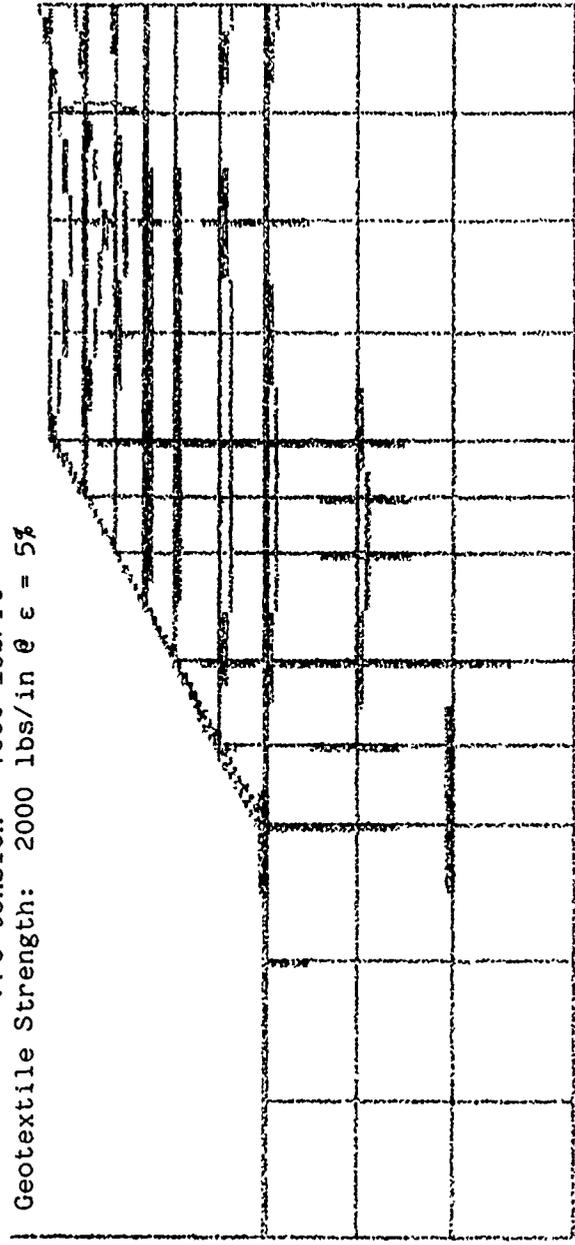


FIG. 3.23 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile Extended 10 Ft
Beyond Toe of Embankment
Geotextile Strength: 2000 lbs/in @ $\epsilon = 5\%$

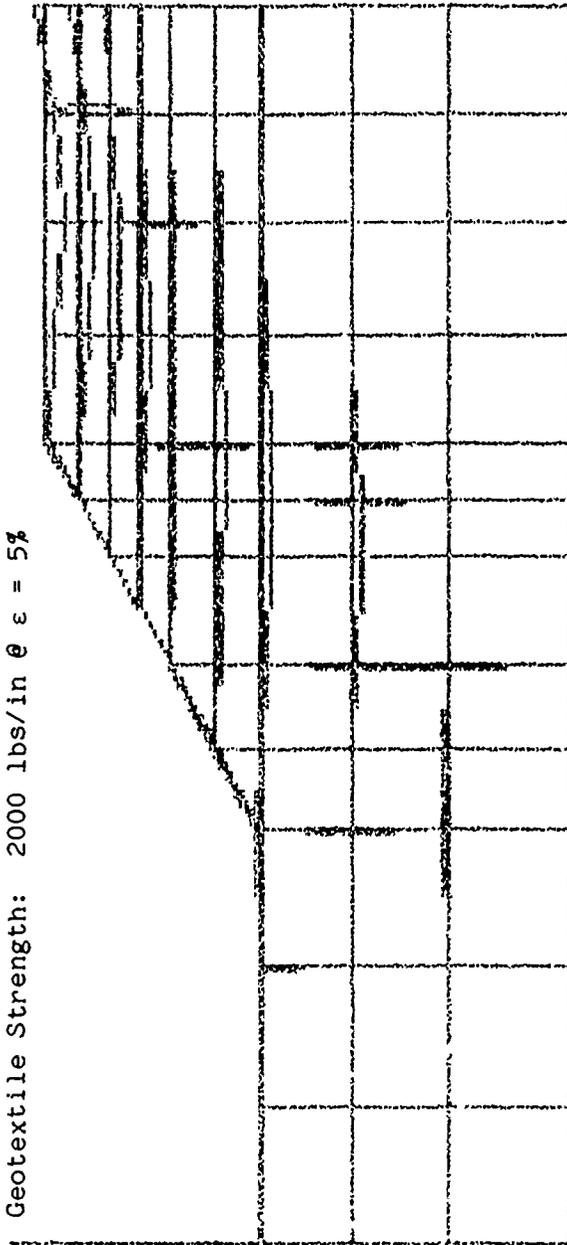


FIG. 3.24 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
No Pre-tension
Geotextile Strength: 4000 lbs/in @ $\epsilon = 5\%$

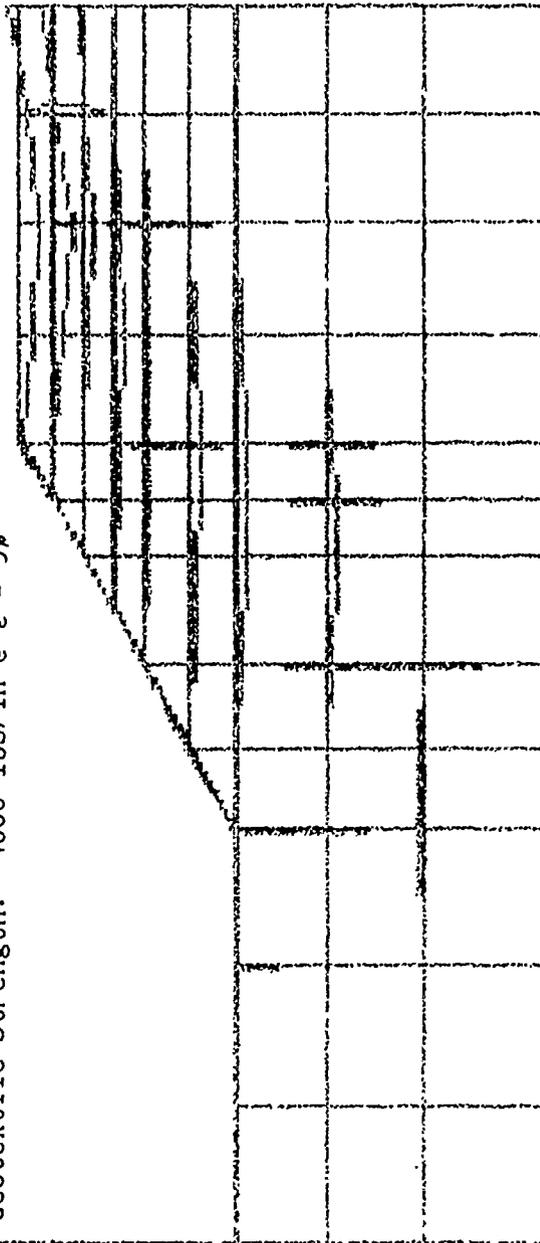


FIG. 3.25 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
Pre-tension = 1000 lbs/ft
Geotextile Strength: 4000 lbs/in @ $\epsilon = 5\%$

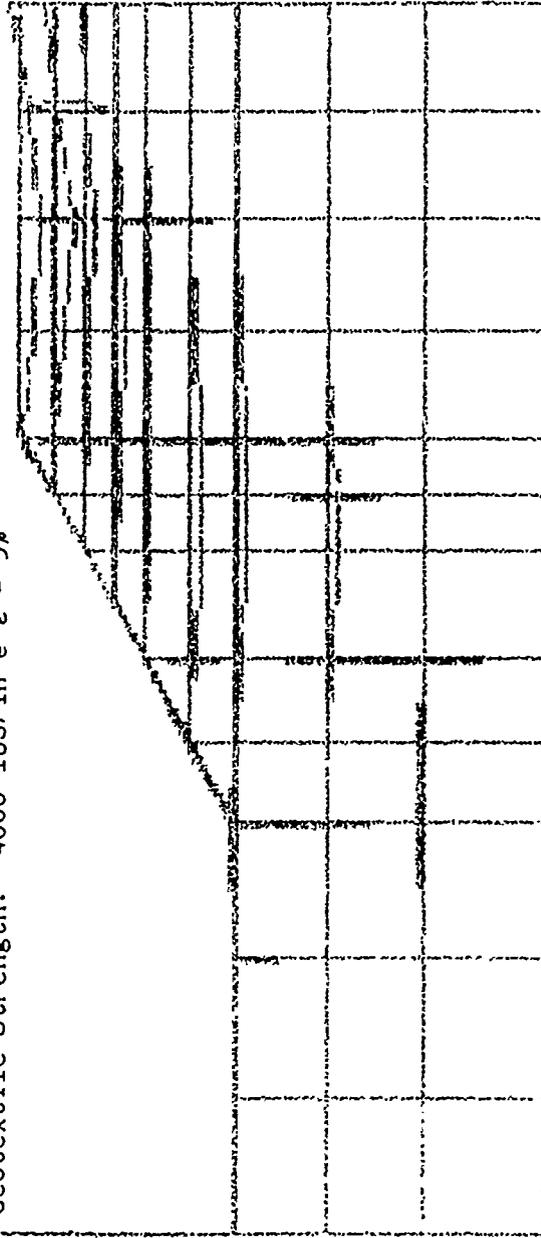


FIG. 3.26 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile Extended 10 Ft
Beyond Toe of Embankment
Geotextile Strength: 4000 lbs/in @ $\epsilon = 5\%$

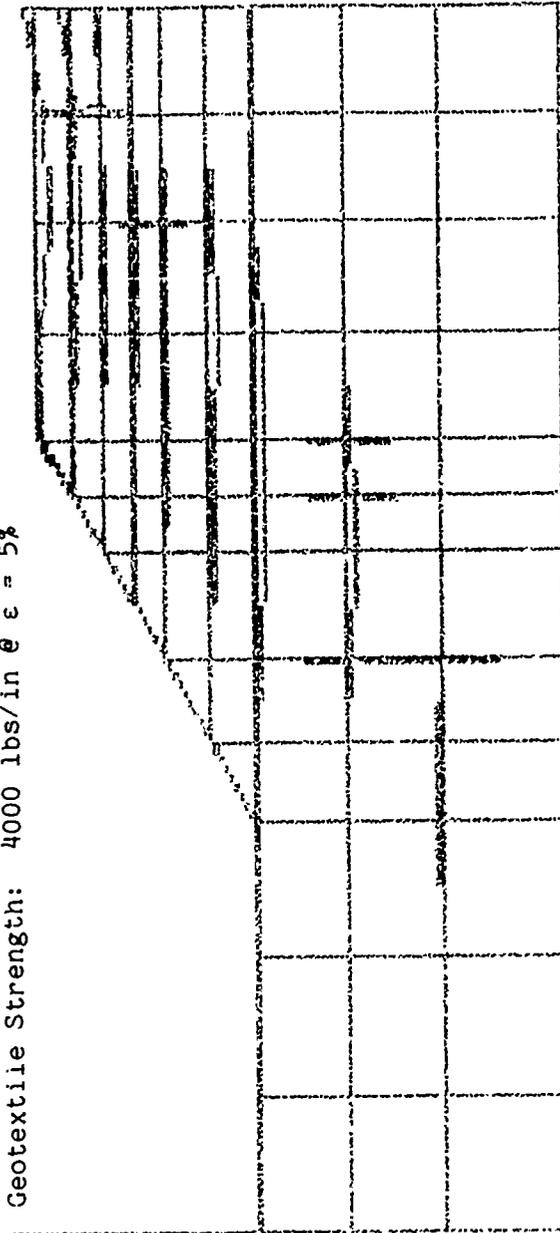


FIG. 3.27 DEFORMED EMBANKMENT GEOMETRY OF SSTIPN ANALYSIS

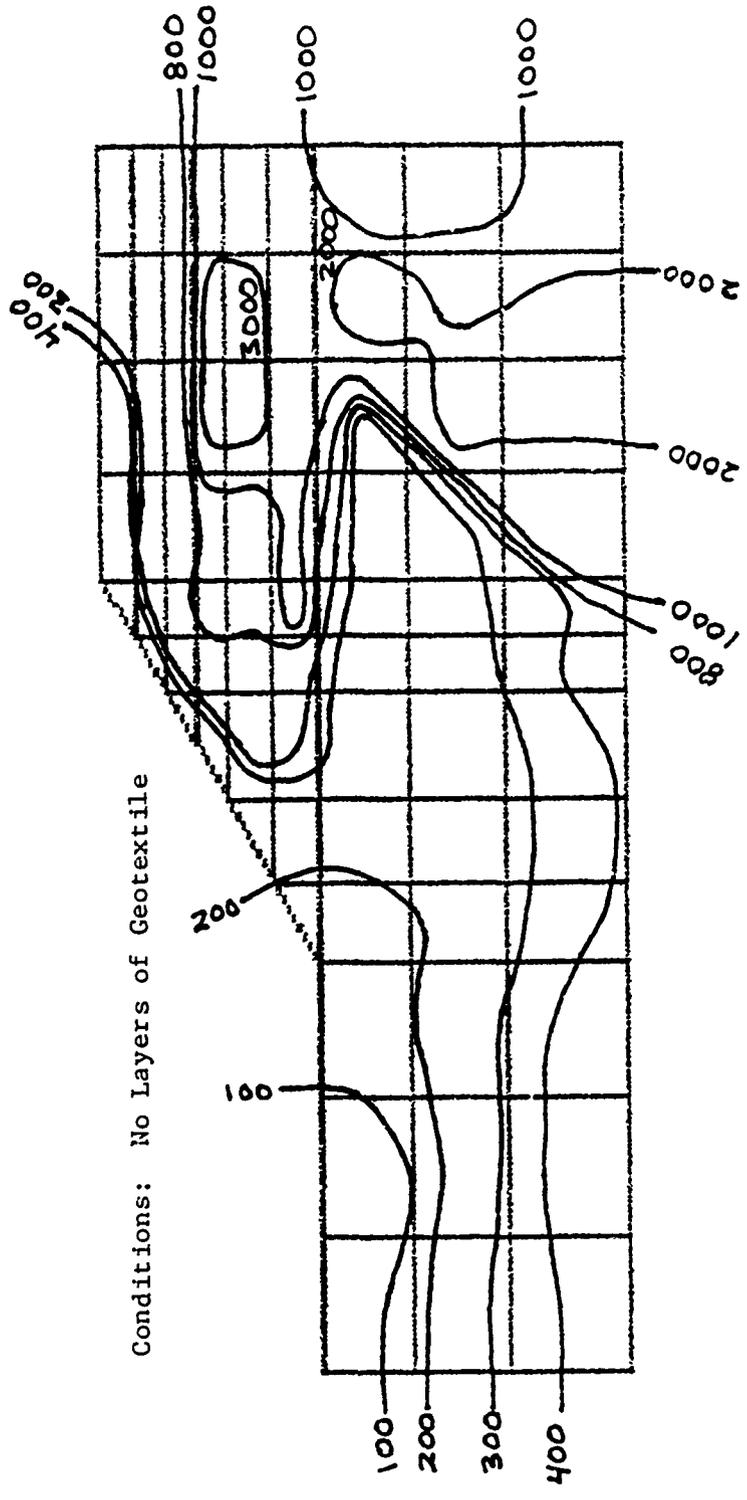


FIG. 3.28 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
 No Pre-tension
 Geotextile Strength: 1000 lbs/in @ $\epsilon = 20\%$

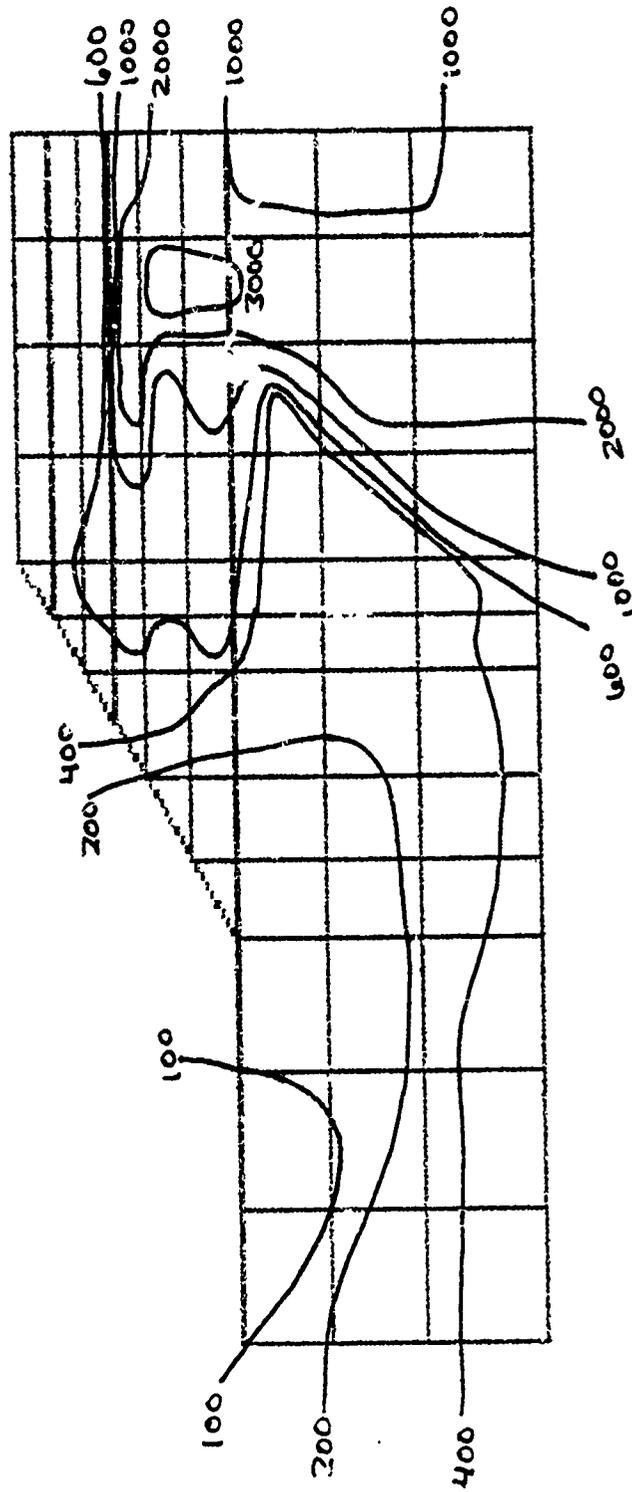


FIG. 3.29 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Condition: 1 Layer of Geotextile
 Pre-tension = 1000 lbs/ft
 Geotextile Strength: 1000 lbs/in @ $\epsilon = 20\%$

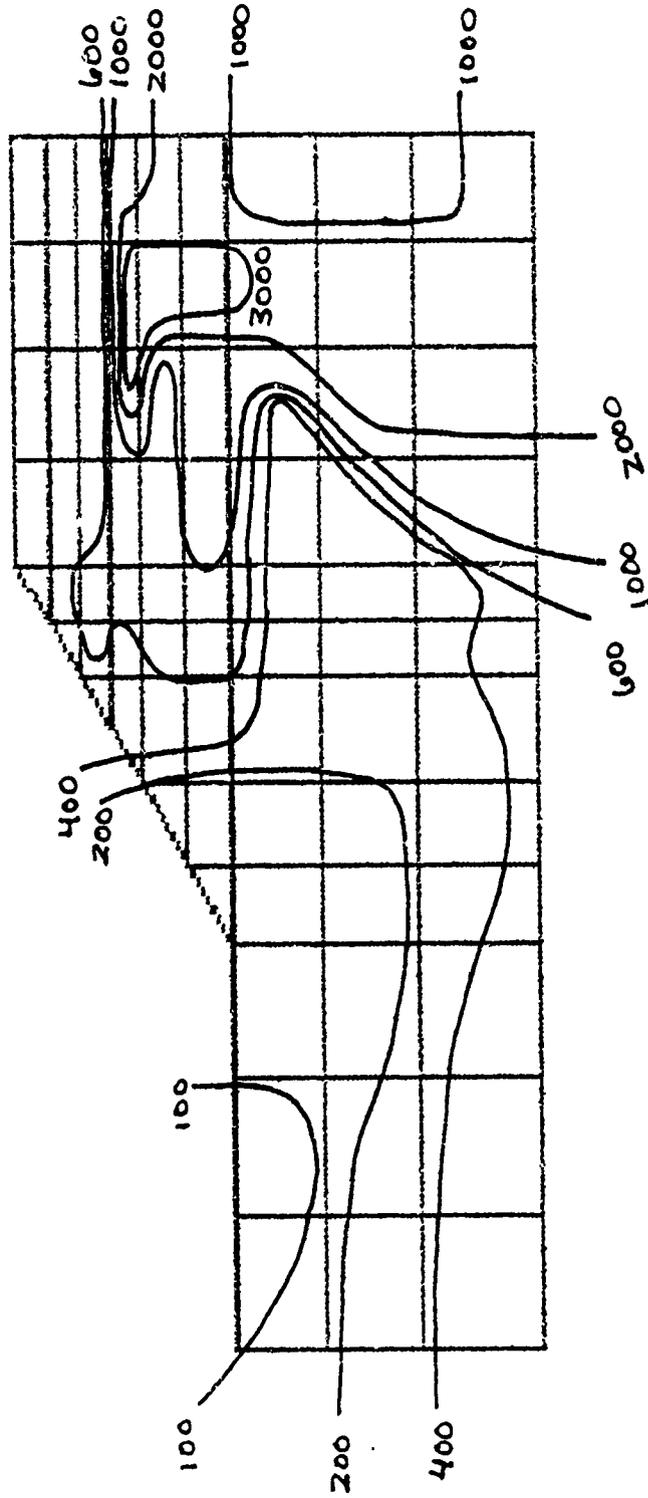


FIG. 3.30 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile Extended
 10 ft Beyond Toe of Embankment
 Pre-tension = 1000 lbs/ft
 Geotextile Strength: 1000 lbs/in @ $\epsilon = 20\%$

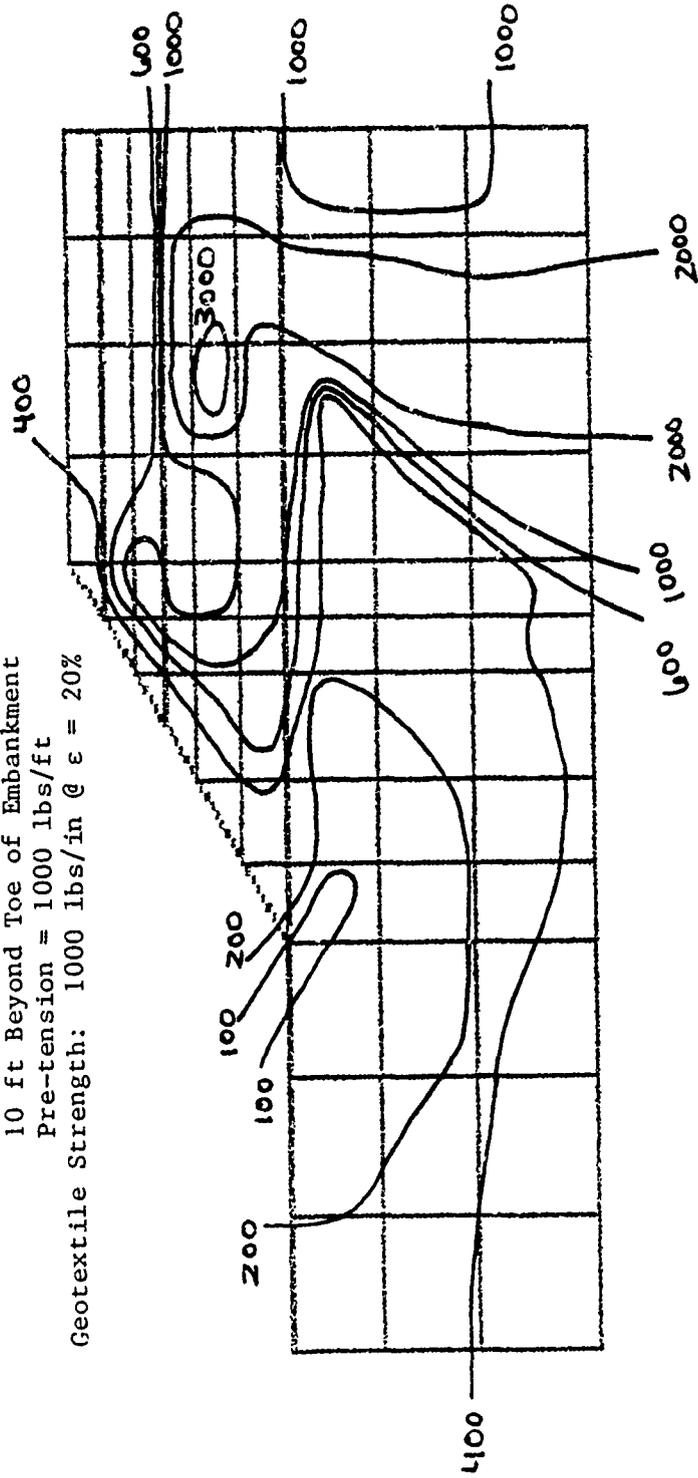


FIG. 3.31 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
No Pre-tension
Geotextile Strength: 2000 lbs/in @ $\epsilon = 20\%$

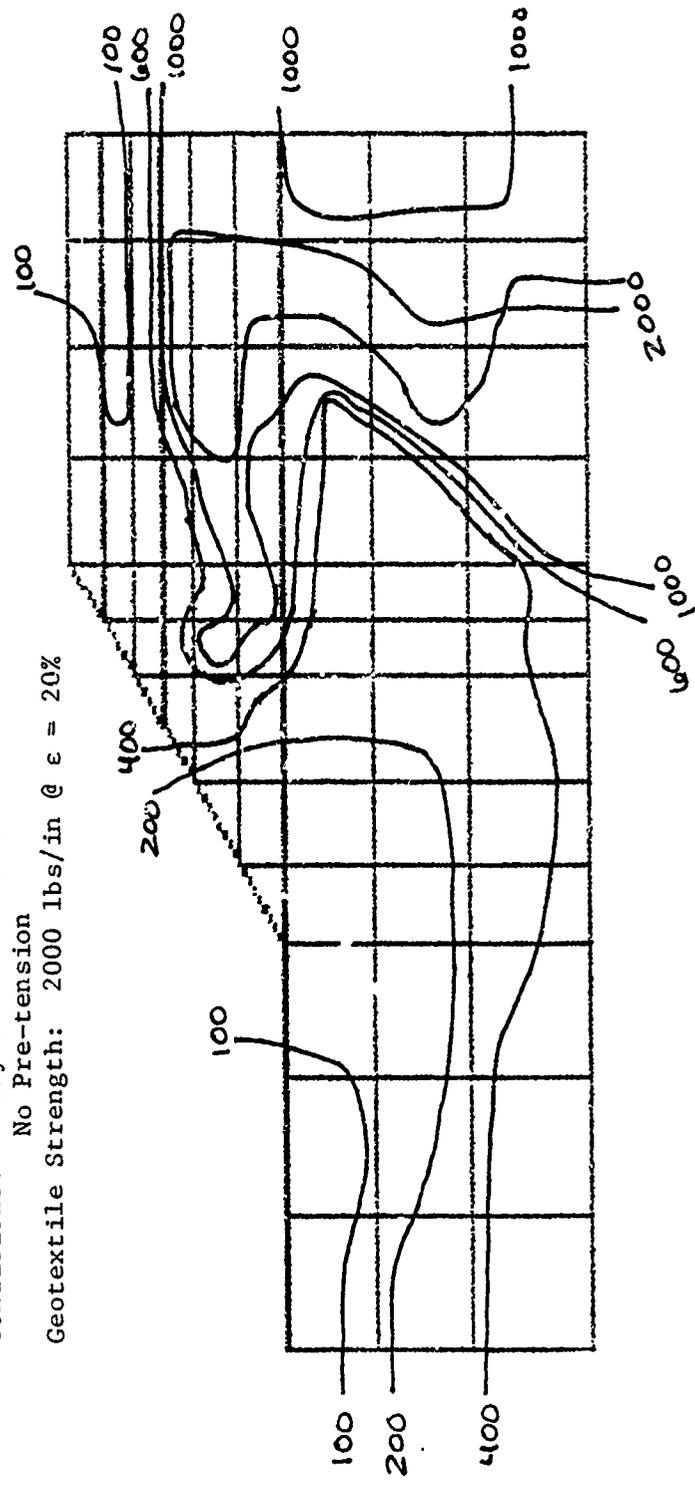


FIG. 3.32 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

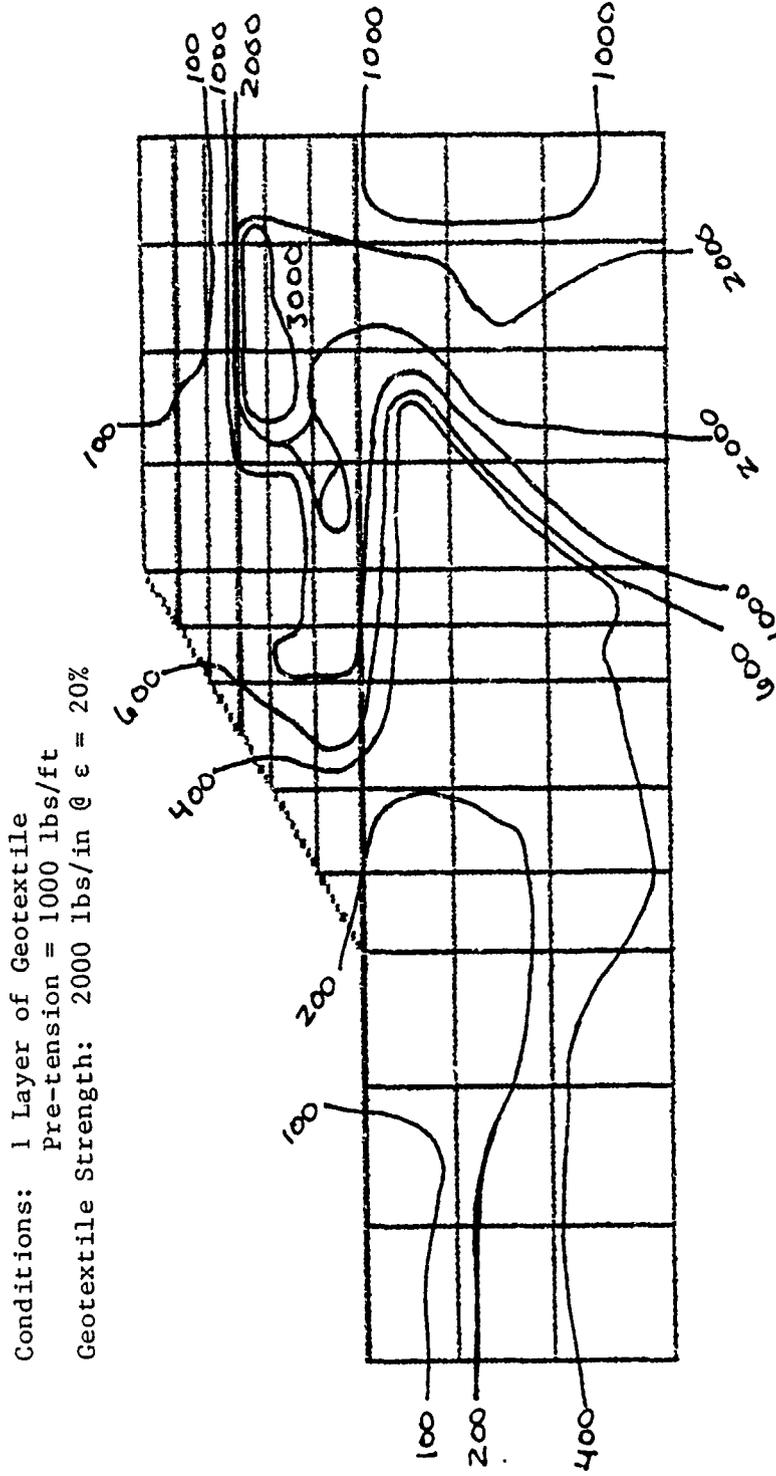


FIG. 3.33 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
No Pre-tension
Geotextile Strength: 4000 lbs/in @ $\epsilon = 20\%$

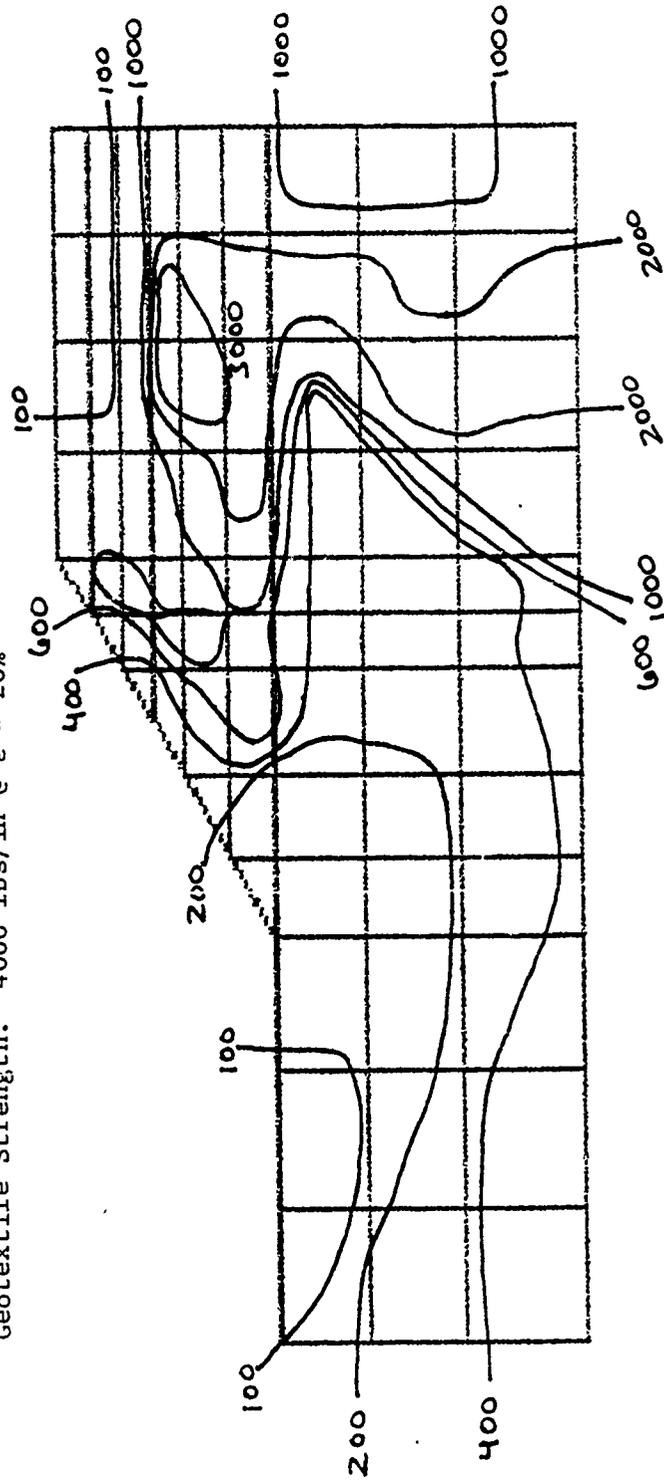


FIG. 3.35 DEVIATORIC STRESS CONTOUR OF SSTIP ANALYSIS

Conditions: 1 Layer of Geotextile
 Pre-tension = 1000 lbs/ft
 Geotextile Strength: 4000 lbs/in @ $\epsilon = 20\%$

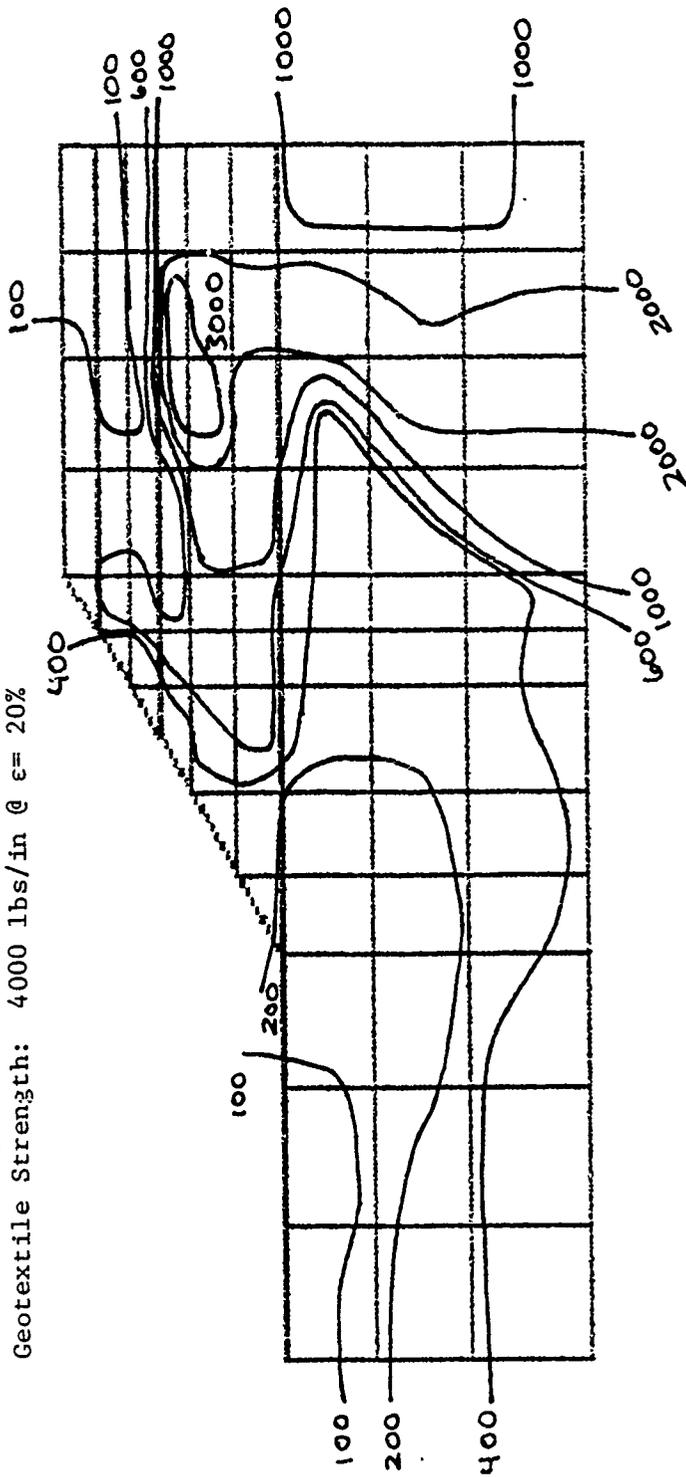


FIG. 3.36 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile Extended 10 Ft
 Beyond Toe of Embankment
 Pe-tension = 1000 lbs/ft
 Geotextile Strength: 4000 lbs/in @ $\epsilon = 20\%$

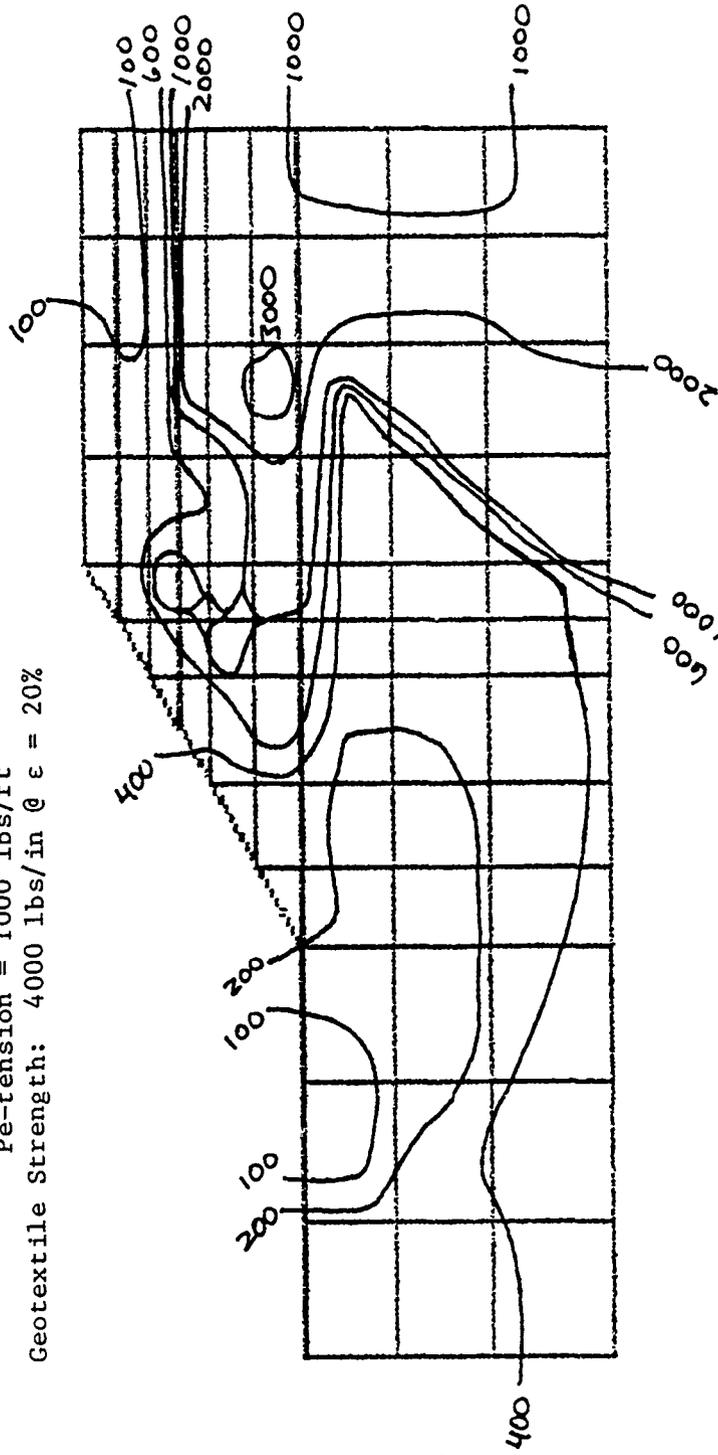


FIG. 3.37 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
Pre-tension = 1000 lbs/ft
Geotextile Strength: 2000 lbs/inc @ $\epsilon = 5\%$

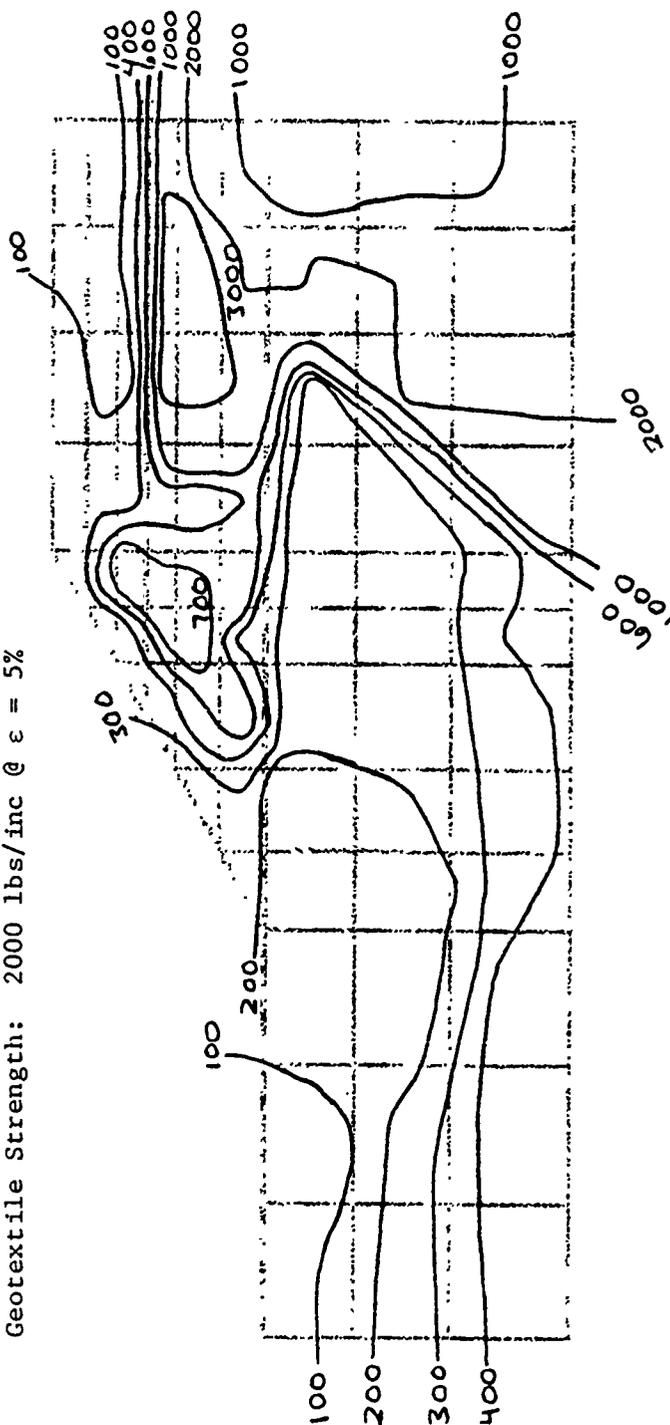


FIG. 3.39 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
No Pre-tension
Geotextile Strength: 4000 lbs/in @ $\epsilon = 5\%$

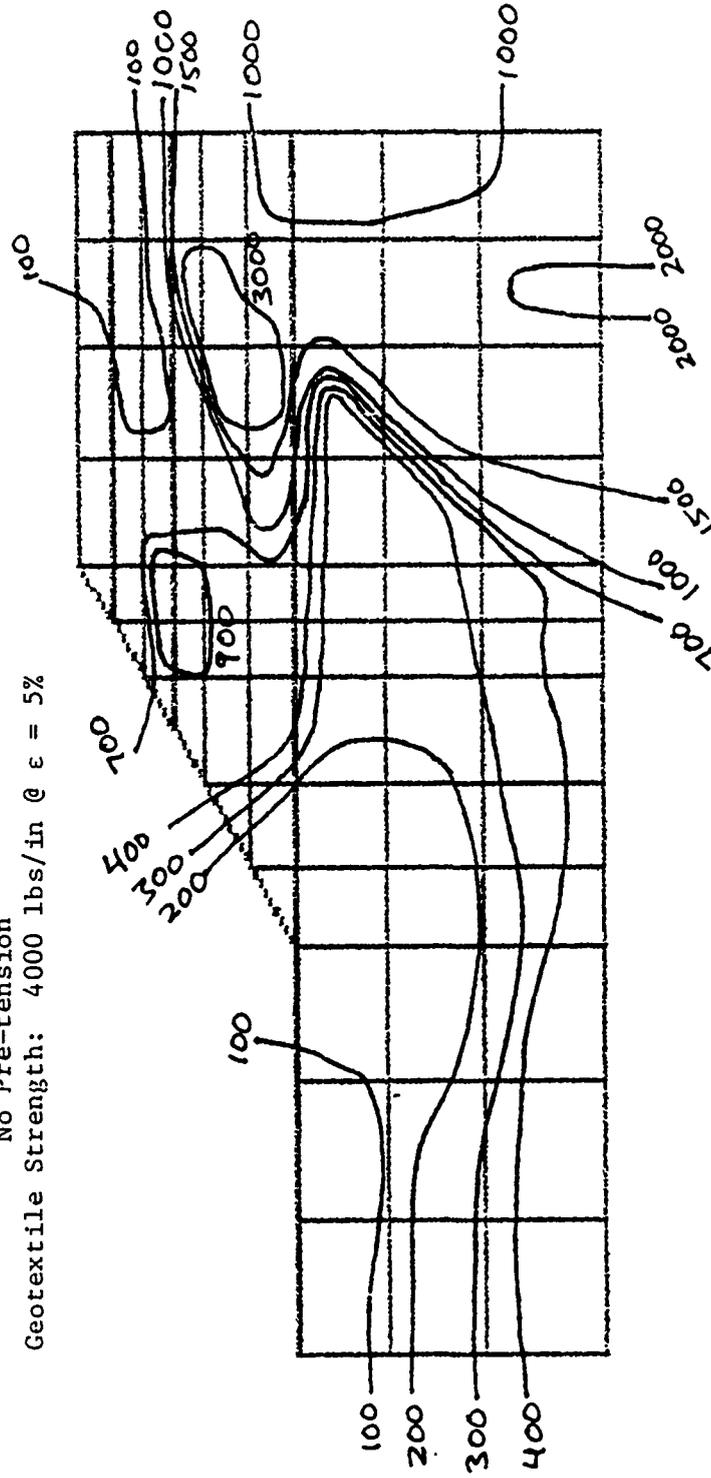


FIG. 3.41 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile
Pre-tension = 1000 lbs/ft
Geotextile Strength: 4000 lbs/in @ $\epsilon = 5\%$

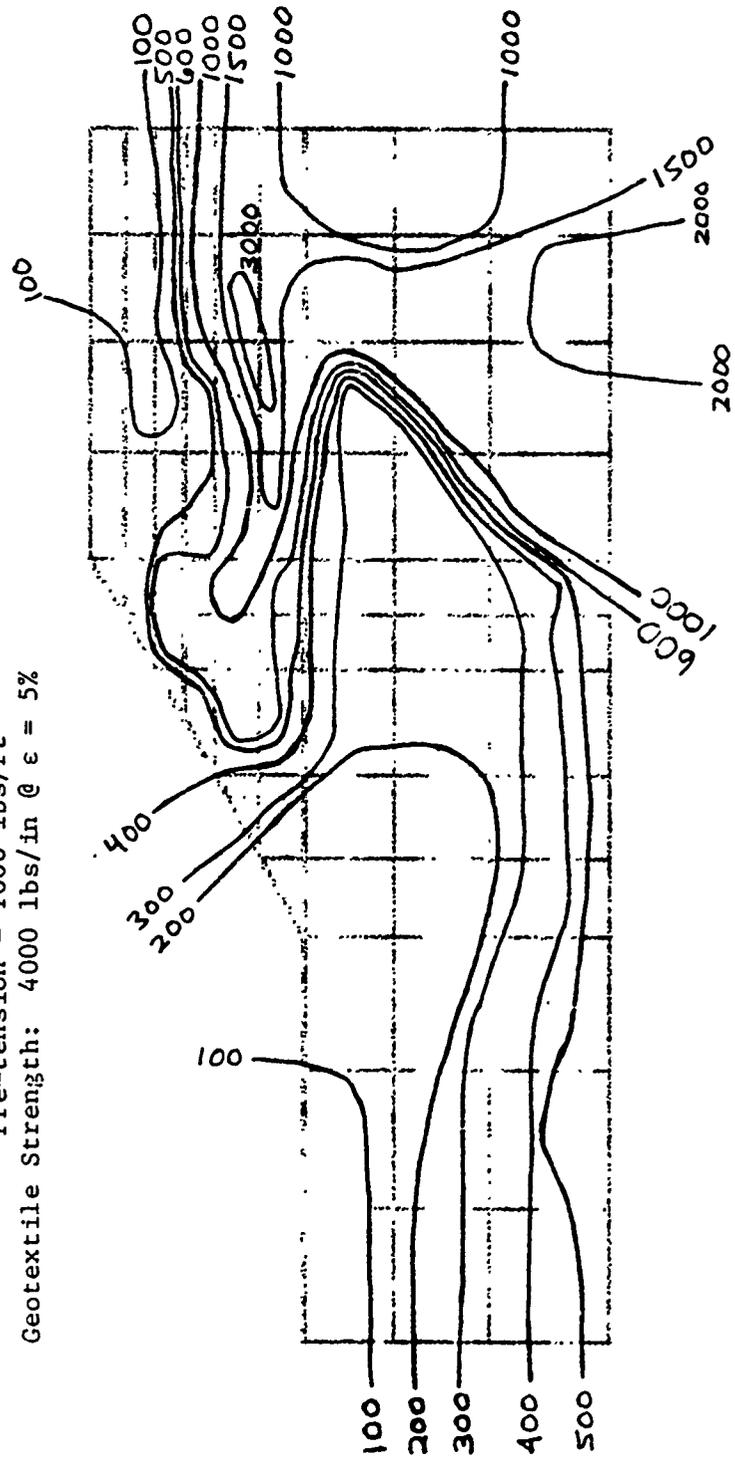


FIG. 3.42 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

Conditions: 1 Layer of Geotextile Extended 10 Ft
Beyond Toe of Embankment
Pre-tension = 1000 lbs/ft
Geotextile Strength: 4000 lbs/in @ $\epsilon = 5\%$

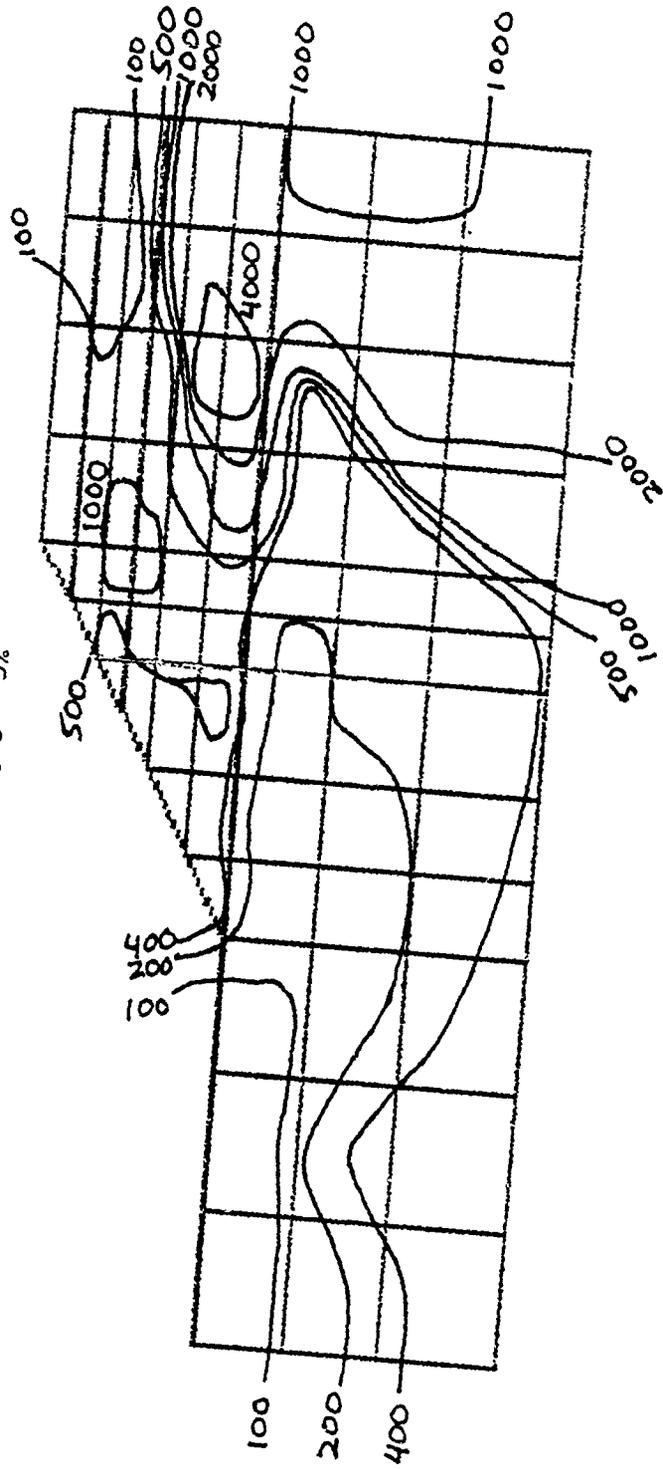


FIG. 3.43 DEVIATORIC STRESS CONTOUR OF SSTIPN ANALYSIS

3.2.2 Conclusions and Recommendations

There are two major factors which determine whether or not a numerical analysis is reliable. First, existing conditions must be modelled properly. Geometry and material properties must be as close to actual conditions as possible to accurately simulate behavior of the model. Second, the program must be able to evaluate conditions which may exist in the field. The soil model must be able to determine appropriate values for strength to simulate actual conditions. The hyperbolic model used in SSTIPN models soil conditions very accurately up to the point of failure. Once failure of the soil results, the model is not capable of simulating accurate conditions. Appendix E contains detail results of all analyses. Close observation of the analyses reveals the occurrence of shear and tensile failure within soil elements located within the thaw-unstable soil and roadway embankment. Failure occurs within the foundation soil from the placement of the embankment's first layer. With the placement of the additional five layers of embankment soil, an increasing number of soil elements enter a failure mode. Observation of stress contours for all conditions do not reveal a major redistribution in stress throughout the embankment and foundation as a result of the addition of a geotextile and increases of strength in the geotextile. The stress contours of the analysis with no layers of geotextile is not very different from the analysis with one layer of geotextile with a strength of 4000 pounds per inch at 5 percent strain. The inability of the hyperbolic model to accurately simulate soil conditions after failure is the primary cause for the difference. With soil elements in

a failure mode throughout each analysis, the computation of stresses within the embankment apparently do not differ very much. In the final analysis, the resulting stress distributions are similar.

Observation of the deformed roadway embankment models, Figures 3.12 through 3.27, reveal subtle differences between each condition. The inability of the hyperbolic model to simulate soil properties after failure has the same effect on displacements as it does on stress computation. However, the diagrams display more displacement with weaker geotextile properties. This indicates the positive effect of stronger geotextile properties. It is difficult to assess which property has the most positive effect due to the programs inability to distinguish between different strain values. However, geotextiles which have higher strength and higher modulae tend to maintain embankment integrity and reduce the failure mechanisms associated with embankment stability. The more the embankment is allowed to deform over the thaw-unstable region, the more the risk for longitudinal cracking in the wearing surface.

The addition of a soil berm and geotextile extension outside the toe of the embankment causes the degradation of thawing to occur beneath the berm more than the sideslopes of the embankment. The result is consolidation of the thaw-unstable soil beneath the berm more than beneath the sideslopes of the embankment. This increases embankment stability and reduces the risk of longitudinal cracking in the wearing surface.

Through further observation of geotextile forces in Appendix E, the failure capacity is not attained in any analysis. This indicates

that geotextile integrity is not impaired and will not fail under these conditions.

A comparison of Figure 3.8 and 3.12 reveals a discrepancy between the resulting displacements in the FEADAM84 and SSTIPN analysis. The soil model employed in the SSTIPN program is an older version of the model used in the FEADAM84 program. Displacements of the embankment model should have been similar given the same condition. The FEADAM84 deformed geometry, Figure 3.8, shows more displacement than the SSTIPN deformed geometry, Figure 3.12. The increased number of soil elements in the SSTIPN analysis should add more flexibility to the model. From the results obtained, the opposite occurs. The cause of this discrepancy is primarily attributed to the different generation of hyperbolic soil model.

Also, proper modelling of soil conditions after failure is best achieved with a plasticity model. The addition of this type of soil model in this program would greatly enhance its effectiveness in evaluating roadway embankments constructed over weak soil.

BIBLIOGRAPHY

1. Andrawes, K. Z., Mashhour, M. M., McGown, A., and Wilson-Fahmy, R. F., "The Finite Element Method of Analysis Applied to Soil-Geotextile Systems." Second International Conference of Geotextiles, Las Vegas, U.S.A. (August, 1982) : 695.
2. Bell, A. L., Green, H. M., and Laverty, K. "Factors Influencing the Selection of Woven Polypropylene Geotextiles for Earth Reinforcement." Second International Conference of Geotextiles, Las Vegas, U.S.A. (August, 1982) : 689.
3. Bonafante, R., Holtz, R. D., and Giroud, J. P. "Soil Reinforcement Design Using Geotextiles and Geogrids (Draft)." Geotextile Testing and the Design Engineer Symposium. 24 June 1985.
4. Das, Braja M. Principles of Geotechnical Engineering. Duxbury Press, Boston, Massachusetts, 1985.
5. Duncan, J. M., and Wong, Kai S., "Hyperbolic Stress-Strain Parameters for Nonlinear Finite Element Analyses of Stresses and Movements in Soil Masses." National Science Foundation Grant GK-35894, University of California at Berkeley, 1974.
6. Duncan, J. M., Dickens, J., Kulwany, Seed, Wong, K. S., "Users' Guide for SSTIPN." January 1979.
7. Duncan, J. M., and Wong, K. S., "The Tensar Corporation Users Manual for STABGM." Virginia Polytechnic Institute and State University, 1984.
8. Duncan, J. M., Seed, R. B., Wong, K. S., and Ozawa, Y., "FEADAM84: A Computer Program for Finite Element Analysis of Dams." Geotechnical Engineering Research Report, Stanford University, 1984.
9. Esch, D. C., "Evaluation of Experimental design Features for Roadway Construction Over Permafrost." Permafrost: Fourth International Conference. (July, 1983) : 283.
10. Esch, D. C., and Meltattie, R. L., "Benefits of a Peat Underlay Used in Road Construction on Permafrost." Permafrost: Fourth International Conference. (July, 1983) : 826.

11. Ingold, T. S., "An Analytical Study of Geotextile Reinforced Embankments." Second International Conference of Geotextiles, Las Vegas, U.S.A. (August, 1982) : 683.
12. Ingold, T. S., "Design Concepts for Reinforced Embankments Over Soft Clays." Geotextiles Technology. 1984.
13. Jeyapalan, J. K., and Lytton, R. L., "Stress Reduction in Flexible Culverts Due to Overlays of Geofabric." Second International Conference of Geotextiles, Las Vegas, U.S.A. (August, 1982) : 701.
14. Johnston, G. H., ed. Permafrost: Engineering Design and Construction. Toronto: John Wiley & Sons, 1981.
15. Martinson, M. A., "Engineering Geology and Soils Report on Center-line Soils for Farmers Loop Road." State of Alaska Department of Transportation and Engineering Geology Section. January, 1985.
16. Rowe, R. K., "The Analysis of an Embankment Constructed on a Geotextile." Second International Conference on Geotextiles, Las Vegas, U.S.A. (August, 1982) : 677.
17. Rowe, R. K., "Reinforced Embankments: Analysis and Design." ASCE Journal of the Geotechnical Engineering Division. (February, 1984) : 231.
18. Rowe, R. K., MacLean, M. D., and Soderman, K. L., "Analysis of a Geotextile-Reinforced Embankment Constructed on Peat." Canadian Geotechnical Journal. (April, 1984) : 563.
19. Schauz, W., "Performance of Fabric Reinforced Aggregate-Soil Systems Under Repeated Loading." Ph.D. dissertation, Georgia Institute of Technology, 1981.
20. Sowers, G. F. Introductory Soil Mechanics and Foundations: Geotechnical Engineering. New York: Macmillan Publishing Co., Inc., 1979.
21. Waite, M., and Morgan, C. L. Graphics Primer for the IBM PC. Berkeley, California: Osborne/McGraw Hill, 1983.
22. Zarling, J. P., Connor, B., and Goering, D. J., "Air Duct Systems for Roadway Stabilization Over Permafrost Areas." Permafrost: Fourth International Conference. (July, 1983) : 1463.

APPENDIX A

STABGM INITIAL ANALYSIS

Figure 2.4 depicts the conditions of the initial STABGM analysis. Detailed results of the initial analyses are contained herein. Minimum factor of safety data from analyses with no layers of reinforcement with different foundation geometry are enclosed. Comparison of minimum factor of safety data with three and four layers of reinforcement are also enclosed.

Program STABR -- Version 2.84 (MS-DOS)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0,

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	29.0	35.0
BOUNDARY	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 0 LEVEL (a)

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS (BISHOP)	FS (ONS)	dfs (SS)
1	28.0	6.0	37.0	22.0	CENTER BELOW	INTERPOLATED OF	
2	28.0	6.0	35.0	22.0	CENTER BELOW	INTERPOLATED OF	
3	28.0	10.0	37.0	18.0	.000	.003	.000
4	28.0	6.0	41.0	22.0	CENTER BELOW	INTERPOLATED OF	
5	28.0	6.0	37.0	24.0	CENTER BELOW	INTERPOLATED OF	

	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS (BISHOP)	FS (OMS)	dfs (BS)
6	28.0	10.0	35.0	18.0	1.037	.921	.000
7	28.0	12.0	37.0	16.0	.988	.909	.000
8	28.0	10.0	39.0	18.0	1.037	.921	.000
9	28.0	8.0	37.0	20.0	.977	.853	.000
10	28.0	12.0	35.0	16.0	1.041	.950	.000
11	28.0	12.0	39.0	16.0	1.041	.950	.000
12	28.0	8.0	39.0	20.0	1.076	.927	.000
13	28.0	8.0	35.0	20.0	1.101	.927	.000

F.S. MINIMUM= .966 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS (BISHOP)	FS (OMS)	dfs (BS)
1	30.0	10.0	35.0	20.0	.932	.842	.000
2	30.0	10.0	31.0	20.0	1.702	1.305	.000
3	30.0	14.0	35.0	16.0	.914	.847	.000
4	30.0	10.0	37.0	20.0	.932	.842	.000
5	30.0	6.0	35.0	24.0	CENTER BELOW INTERPOLATED CR.		
6	30.0	14.0	35.0	16.0	1.012	.926	.000
7	30.0	16.0	35.0	14.0	.930	.872	.000
8	30.0	14.0	37.0	16.0	.886	.823	.000
9	30.0	12.0	35.0	18.0	.910	.831	.000
10	30.0	16.0	37.0	14.0	.906	.852	.000
11	30.0	14.0	39.0	16.0	.914	.847	.000
12	30.0	12.0	37.0	18.0	.874	.802	.000
13	30.0	12.0	35.0	18.0	.910	.831	.000
14	30.0	12.0	39.0	18.0	.910	.831	.000
15	30.0	10.0	37.0	20.0	.882	.802	.000
16	30.0	14.0	35.0	16.0	.914	.847	.000
17	30.0	14.0	39.0	16.0	.914	.847	.000
18	30.0	10.0	39.0	20.0	.932	.842	.000
19	30.0	10.0	35.0	20.0	.932	.842	.000

F.S. MINIMUM= .874 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS (BISHOP)	FS (OMS)	dfs (BS)
1	35.0	15.0	35.0	20.0	.860	.817	.000
2	35.0	15.0	31.0	20.0	1.030	.869	.000
3	35.0	19.0	35.0	16.0	.837	.794	.000
4	35.0	15.0	39.0	20.0	.860	.817	.000
5	35.0	11.0	35.0	24.0	CENTER BELOW INTERPOLATED CR.		
6	35.0	19.0	35.0	16.0	.875	.833	.000
7	35.0	21.0	35.0	14.0	.839	.799	.000
8	35.0	19.0	37.0	16.0	.800	.757	.000
9	35.0	17.0	35.0	18.0	.812	.767	.000
10	35.0	21.0	37.0	14.0	.809	.769	.000
11	35.0	19.0	39.0	16.0	.837	.794	.000
12	35.0	17.0	37.0	18.0	.829	.785	.000
13	35.0	21.0	35.0	14.0	.839	.799	.000
14	35.0	21.0	39.0	14.0	.839	.799	.000
15	35.0	17.0	39.0	18.0	.842	.797	.000
16	35.0	17.0	35.0	18.0	.842	.797	.000

F.S. MINIMUM= .826 FOR THE CIRCLE OF CENTER (37.0, 18.0)

Program STABR -- Version 2.84 (MS-DOS)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 15 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0.

GEOMETRY

SECTIONS	.0	15.0	18.0	20.0	25.0	32.0	42.0	59.0	64.0	66.0	69.0
T. CRACKS	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 0 LEVEL (s)

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	138.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 15 feet from centerline

NUMBER	TANGENT	RADIUS	(%) CENTER	(%) CENTER	F3 (BISHOP)	F2 (OMS)	F1S (BS)
1	28.0	6.0	37.0	22.0	CENTER BELOW	INTERPOLATED OF	
2	28.0	6.0	37.0	22.0	CENTER BELOW	INTERPOLATED OF	
3	28.0	10.0	37.0	18.0	.000	.000	.000
4	28.0	6.0	41.0	22.0	CENTER BELOW	INTERPOLATED OF	
5	28.0	6.0	37.0	22.0	CIRCLE	INTERPOLATED OF	

6	28.0	10.0	35.0	18.0	1.007	.921	.000
7	28.0	12.0	37.0	16.0	.988	.909	.000
8	28.0	10.0	39.0	18.0	1.007	.921	.000
9	28.0	8.0	37.0	20.0	.977	.853	.000
10	28.0	12.0	35.0	16.0	1.041	.950	.000
11	28.0	12.0	39.0	16.0	1.041	.950	.000
12	28.0	8.0	39.0	20.0	1.076	.927	.000
13	28.0	8.0	35.0	20.0	1.101	.927	.000

F.S. MINIMUM= .966 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 15 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	30.0	10.0	35.0	20.0	.932	.840	.000
2	30.0	10.0	31.0	20.0	1.702	1.305	.000
3	30.0	14.0	35.0	16.0	.914	.847	.000
4	30.0	10.0	39.0	20.0	.932	.842	.000
5	30.0	6.0	35.0	24.0	CENTER	BELOW INTERPOLATED CR.	
6	30.0	14.0	33.0	16.0	1.012	.926	.000
7	30.0	16.0	35.0	14.0	.930	.870	.000
8	30.0	14.0	37.0	16.0	.986	.823	.000
9	30.0	12.0	35.0	18.0	.910	.831	.000
10	30.0	16.0	37.0	14.0	.906	.852	.000
11	30.0	14.0	39.0	16.0	.914	.847	.000
12	30.0	12.0	37.0	18.0	.874	.802	.000
13	30.0	12.0	35.0	18.0	.910	.831	.000
14	30.0	12.0	39.0	18.0	.910	.831	.000
15	30.0	10.0	37.0	20.0	.882	.802	.000
16	30.0	14.0	35.0	16.0	.914	.847	.000
17	30.0	14.0	39.0	16.0	.914	.847	.000
18	30.0	10.0	39.0	20.0	.932	.842	.000
19	30.0	10.0	35.0	20.0	.932	.842	.000

F.S. MINIMUM= .874 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 15 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	35.0	15.0	35.0	20.0	.960	1.117	.000
2	35.0	15.0	31.0	20.0	7.176	2.075	.000
3	35.0	19.0	35.0	16.0	.837	.791	.000
4	35.0	15.0	39.0	20.0	.860	.810	.000
5	35.0	11.0	35.0	24.0	CENTER	BELOW INTERPOLATED CR.	
6	35.0	19.0	33.0	16.0	1.176	1.035	.000
7	35.0	21.0	35.0	14.0	.962	1.010	.000
8	35.0	19.0	37.0	16.0	.925	.782	.000
9	35.0	17.0	35.0	18.0	.942	.797	.000
10	35.0	21.0	37.0	14.0	.927	.750	.000
11	35.0	19.0	39.0	16.0	.937	.751	.000
12	35.0	17.0	37.0	18.0	.926	.780	.000
13	35.0	21.0	35.0	14.0	1.002	1.110	.000
14	35.0	21.0	39.0	16.0	.937	.787	.000
15	35.0	17.0	39.0	18.0	1.042	.797	.000
16	35.0	17.0	35.0	18.0	.917	.787	.000

F.S. MINIMUM= .925 FOR THE CIRCLE OF CENTER (37.0, 18.0)

Program STABR -- Version 2.84 (MS-DOS)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 20 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0,

GEOMETRY

SECTIONS	.0	20.0	23.0	25.0	30.0	32.0	42.0	54.0	59.0	61.0	64.0
T. CRACKS	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 0 LEVEL(s)

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	135.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 20 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	PS - P15M1	PS - OMS1	PS - BS1
1	28.0	5.0	37.0	22.0	CENTER BELOW	INTERPOLATED OF	
2	28.0	5.0	37.0	22.0	CENTER BELOW	INTERPOLATED OF	
3	28.0	10.0	37.0	19.0	PERF	1.000	1.000
4	28.0	5.0	41.0	22.0	CENTER BELOW	INTERPOLATED OF	
5	28.0	5.0	41.0	22.0	CENTER BELOW	INTERPOLATED OF	

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS (BISHOP)	FS (OMS)	dfs (BS)
6	28.0	10.0	35.0	18.0	1.037	.921	.000
7	28.0	12.0	37.0	16.0	.988	.909	.000
8	29.0	10.0	39.0	18.0	1.037	.921	.000
9	28.0	8.0	37.0	20.0	.977	.853	.000
10	28.0	12.0	35.0	16.0	1.041	.950	.000
11	28.0	12.0	39.0	18.0	1.041	.950	.000
12	28.0	8.0	39.0	20.0	1.076	.927	.000
13	28.0	8.0	35.0	20.0	1.101	.927	.000

F.S. MINIMUM= .966 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 20 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS (BISHOP)	FS (OMS)	dfs (BS)
1	30.0	10.0	35.0	20.0	.932	.842	.000
2	30.0	10.0	31.0	20.0	2.089	1.319	.000
3	30.0	14.0	35.0	16.0	.914	.847	.000
4	30.0	10.0	39.0	20.0	.932	.843	.000
5	30.0	6.0	35.0	24.0	CENTER BELOW INTERPOLATED OF		
6	30.0	14.0	33.0	16.0	1.249	1.234	.000
7	30.0	16.0	35.0	14.0	.930	.872	.000
8	30.0	14.0	37.0	16.0	.886	.823	.000
9	30.0	12.0	35.0	18.0	.910	.831	.000
10	30.0	16.0	37.0	14.0	.906	.852	.000
11	30.0	14.0	39.0	16.0	.914	.847	.000
12	30.0	12.0	37.0	18.0	.874	.802	.000
13	30.0	12.0	35.0	18.0	.910	.831	.000
14	30.0	12.0	39.0	18.0	.910	.831	.000
15	30.0	10.0	37.0	20.0	.883	.802	.000
16	30.0	14.0	35.0	16.0	.914	.847	.000
17	30.0	14.0	39.0	16.0	.914	.847	.000
18	30.0	10.0	39.0	20.0	.932	.843	.000
19	30.0	10.0	35.0	20.0	.932	.842	.000

F.S. MINIMUM= .874 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 3 feet, 20 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS (BISHOP)	FS (OMS)	dfs (BS)
1	35.0	15.0	35.0	20.0	4.538	4.062	.000
2	35.0	15.0	31.0	20.0	6.673	7.596	.000
3	35.0	19.0	35.0	16.0	5.054	4.571	.000
4	35.0	15.0	39.0	20.0	.860	.818	.000
5	35.0	11.0	35.0	24.0	CENTER BELOW INTERPOLATED OF		
6	35.0	15.0	37.0	20.0	.860	.849	.000
7	35.0	17.0	39.0	16.0	.845	.797	.000
8	35.0	15.0	41.0	20.0	.917	.868	.000
9	35.0	17.0	39.0	22.0	CENTER BELOW INTERPOLATED OF		
10	35.0	17.0	37.0	18.0	1.836	1.743	.000
11	35.0	19.0	39.0	16.0	.938	.921	.000
12	35.0	17.0	41.0	18.0	.884	.877	.000
13	35.0	19.0	37.0	16.0	3.349	3.109	.000
14	35.0	19.0	41.0	16.0	.937	.906	.000
15	35.0	15.0	41.0	20.0	.717	.658	.000
16	35.0	15.0	37.0	20.0	.860	.849	.000

F.S. MINIMUM= .842 FOR THE CIRCLE OF CENTER (39.0, 19.0)

0 BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

0permafrost depth 5 feet, 20 feet from centerline

0CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

0SEISMIC COEFFICIENT S1,S2 = .00, .00

0UNIT WEIGHT OF WATER = 62.40

0 SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

0ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0,

0GEOMETRY

0 SECTIONS	.0	20.0	23.0	25.0	30.0	32.0	42.0	54.0	59.0	61.0	64.0
T. CRACKS	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

0REINFORCING FORCE DATA AT 3 LEVEL(s)

0 Y= 20.50 NO. OF FORCE POINTS= 4
 X FORCE
 33.0 .0
 30.0 1666.0
 3.0 1666.0
 .0 .0

0 Y= 22.00 NO. OF FORCE POINTS= 4
 X FORCE
 36.0 .0
 33.0 5000.0
 3.0 5000.0
 .0 .0

0 Y= 24.00 NO. OF FORCE POINTS= 4
 X FORCE
 40.0 .0
 37.0 5000.0
 3.0 5000.0
 .0 .0

0SOIL PROPERTIES

0 LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	175.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

0 BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

0permafrost depth 5 feet, 20 feet from centerline

0 NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	COEFFICIENT	FS (ONS)	IFB (EE)
1	28.0	6.0	37.0	22.0	CENTER BELOW	INTERPOLATED	0
2	28.0	6.0	33.0	22.0	CENTER BELOW	INTERPOLATED	0

3	28.0	10.0	37.0	18.0	3.282	3.183	2.31
4	28.0	4.0	41.0	22.0	CENTER BELOW INTERPOLATED C		
5	28.0	2.0	37.0	26.0	CIRCLE OUTSIDE SLOPE		
6	28.0	10.0	35.0	18.0	3.493	3.378	2.45
7	28.0	12.0	37.0	16.0	3.502	3.423	2.5
8	28.0	10.0	39.0	18.0	3.494	3.378	2.45
9	28.0	8.0	37.0	20.0	3.908	2.794	1.91
10	28.0	9.0	35.0	20.0	3.197	3.023	2.07
11	28.0	8.0	39.0	20.0	3.166	3.017	2.05
12	28.0	6.0	37.0	22.0	CENTER BELOW INTERPOLATED C		
13	28.0	10.0	35.0	18.0	3.493	3.378	2.45
14	28.0	10.0	39.0	18.0	3.494	3.378	2.45
15	28.0	6.0	39.0	22.0	CENTER BELOW INTERPOLATED C		
16	28.0	6.0	35.0	22.0	CENTER BELOW INTERPOLATED C		

OF.S. MINIMUM= 2.908 FOR THE CIRCLE OF CENTER (37.0, 20.0)

Q BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

Openmafrost depth 5 feet, 20 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(ONS)	dfs(BS)
1	30.0	8.0	35.0	22.0	CENTER BELOW INTERPOLATED OF		
2	30.0	8.0	31.0	22.0	CENTER BELOW INTERPOLATED OF		
3	30.0	12.0	35.0	18.0	2.379	2.300	1.46
4	30.0	8.0	39.0	22.0	CENTER BELOW INTERPOLATED OF		
5	30.0	4.0	35.0	26.0	CIRCLE OUTSIDE SLOPE		
6	30.0	12.0	37.0	18.0	2.687	2.583	1.64
7	30.0	14.0	35.0	16.0	2.563	2.495	1.64
8	30.0	12.0	37.0	18.0	2.291	2.218	1.41
9	30.0	10.0	35.0	20.0	2.084	1.995	1.15
10	30.0	10.0	33.0	20.0	2.475	2.347NEG.RESIST	
11	30.0	10.0	37.0	20.0	1.980	1.999	1.09
12	30.0	8.0	35.0	22.0	CENTER BELOW INTERPOLATED OF		
13	30.0	12.0	37.0	18.0	2.291	2.218	1.41
14	30.0	10.0	39.0	20.0	2.084	1.995	1.15
15	30.0	8.0	37.0	22.0	CENTER BELOW INTERPOLATED OF		
16	30.0	12.0	35.0	18.0	2.379	2.300	1.46
17	30.0	12.0	39.0	18.0	2.379	2.300	1.46
18	30.0	8.0	39.0	22.0	CENTER BELOW INTERPOLATED OF		
19	30.0	8.0	35.0	22.0	CENTER BELOW INTERPOLATED OF		

OF.S. MINIMUM= 1.980 FOR THE CIRCLE OF CENTER (37.0, 20.0)

Q BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

Openmafrost depth 5 feet, 20 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(ONS)	dfs(BS)
1	35.0	15.0	35.0	22.0	CENTER BELOW INTERPOLATED OF		
2	35.0	13.0	31.0	22.0	CENTER BELOW INTERPOLATED OF		
3	35.0	17.0	35.0	18.0	5.442	4.951	.61
4	35.0	13.0	39.0	22.0	CENTER BELOW INTERPOLATED OF		
5	35.0	9.0	35.0	26.0	CENTER BELOW RIGHT INTERSEC		
6	35.0	17.0	32.0	18.0	7.086	6.374	.62
7	35.0	19.0	35.0	16.0	5.780	5.301	.72
8	35.0	17.0	37.0	18.0	2.236	2.262	.62
9	35.0	19.0	35.0	20.0	4.377	4.301	.47
10	35.0	19.0	37.0	16.0	4.084	3.874	.73
11	35.0	17.0	39.0	18.0	1.474	1.409	.67
12	35.0	15.0	37.0	20.0	1.398	1.388	.47
13	35.0	13.0	35.0	20.0	4.377	4.301	.47
14	35.0	15.0	39.0	20.0	1.303	1.265	.44
15	35.0	17.0	37.0	20.0	CENTER BELOW INTERPOLATED C		
16	35.0	17.0	35.0	18.0	5.442	4.951	.61
17	35.0	17.0	39.0	18.0	1.474	1.409	.67
18	35.0	13.0	39.0	22.0	CENTER BELOW INTERPOLATED C		
19	35.0	11.0	35.0	22.0	CENTER BELOW INTERPOLATED C		

OF.S. MINIMUM= 1.278 FOR THE CIRCLE OF CENTER (37.0, 20.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 20 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0,

GOMETRY

SECTIONS	.0	20.0	23.0	25.0	30.0	32.0	42.0	54.0	59.0	61.0	64.0
T. CRACKS	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	20.0	20.0	20.0	20.0	20.0	20.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 4 LEVEL(S)

Y= 20.50 NO. OF FORCE POINTS= 4
 X FORCE
 33.0 .0
 30.0 1666.0
 3.0 1666.0
 .0 .0

Y= 22.00 NO. OF FORCE POINTS= 4
 X FORCE
 36.0 .0
 33.0 5000.0
 3.0 5000.0
 .0 .0

Y= 24.00 NO. OF FORCE POINTS= 4
 X FORCE
 40.0 .0
 37.0 5000.0
 3.0 5000.0
 .0 .0

Y= 25.00 NO. OF FORCE POINTS= 4
 X FORCE
 42.0 .0
 39.0 5000.0
 3.0 3000.0
 .0 .0

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 20 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(QMS)	dFS(BS)
1	28.0	6.0	37.0	22.0	CENTER BELOW INTERPOLATED CR		
2	28.0	6.0	33.0	22.0	CENTER BELOW INTERPOLATED CR		
3	28.0	10.0	37.0	18.0	4.778	4.679	3.811
4	28.0	6.0	41.0	22.0	CENTER BELOW INTERPOLATED CR		
5	28.0	2.0	37.0	26.0	CIRCLE OUTSIDE SLOPE		
6	28.0	10.0	35.0	18.0	5.081	4.965	4.044
7	28.0	12.0	37.0	16.0	4.962	4.883	3.974
8	28.0	10.0	39.0	18.0	5.081	4.965	4.045
9	28.0	8.0	37.0	20.0	4.473	4.349	3.496
10	28.0	8.0	35.0	20.0	4.897	4.722	3.795
11	28.0	8.0	39.0	20.0	4.876	4.727	3.799
12	28.0	6.0	37.0	22.0	CENTER BELOW INTERPOLATED CR		
13	28.0	10.0	35.0	18.0	5.081	4.965	4.044
14	28.0	10.0	39.0	18.0	5.081	4.965	4.045
15	28.0	6.0	39.0	22.0	CENTER BELOW INTERPOLATED CR		
16	28.0	6.0	35.0	22.0	CENTER BELOW INTERPOLATED CR		

F.S. MINIMUM= 4.473 FOR THE CIRCLE OF CENTER (37.0, 20.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 20 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(QMS)	dFS(BS)
1	30.0	8.0	35.0	22.0	CENTER BELOW INTERPOLATED CR		
2	30.0	8.0	31.0	22.0	CENTER BELOW INTERPOLATED CR		
3	30.0	12.0	35.0	18.0	3.328	3.249	2.418
4	30.0	8.0	39.0	22.0	CENTER BELOW INTERPOLATED CR		
5	30.0	4.0	35.0	26.0	CIRCLE OUTSIDE SLOPE		
6	30.0	12.0	33.0	16.0	3.752	3.649	2.718
7	30.0	14.0	35.0	16.0	3.520	3.452	2.608
8	30.0	12.0	37.0	18.0	3.204	3.124	2.337
9	30.0	10.0	35.0	20.0	3.019	2.929	2.088
10	30.0	10.0	33.0	20.0	3.574	3.479	2.479
11	30.0	10.0	37.0	20.0	2.868	2.799	1.984
12	30.0	8.0	35.0	22.0	CENTER BELOW INTERPOLATED CR		
13	30.0	12.0	37.0	18.0	3.006	3.134	2.332
14	30.0	10.0	39.0	18.0	3.019	2.929	2.088
15	30.0	8.0	37.0	22.0	CENTER BELOW INTERPOLATED CR		
16	30.0	12.0	35.0	18.0	3.328	3.249	2.418

17	30.0	12.0	39.0	18.0	3.328	3.249	2.417
18	30.0	8.0	39.0	22.0	CENTER BELOW INTERPOLATED CR		
19	30.0	8.0	35.0	22.0	CENTER BELOW INTERPOLATED CR		

F.S. MINIMUM= 2.869 FOR THE CIRCLE OF CENTER (37.0, 20.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 20 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(B9)
1	35.0	13.0	35.0	22.0	CENTER BELOW INTERPOLATED CR		
2	35.0	13.0	31.0	22.0	CENTER BELOW INTERPOLATED CR		
3	35.0	17.0	35.0	18.0	5.838	5.347	1.008
4	35.0	13.0	39.0	22.0	CENTER BELOW INTERPOLATED CR		
5	35.0	9.0	35.0	26.0	CENTER BELOW RIGHT INTERSECT		
6	35.0	17.0	33.0	18.0	7.489	6.778	1.027
7	35.0	19.0	35.0	16.0	6.202	5.722	1.148
8	35.0	17.0	37.0	18.0	2.656	2.763	1.020
9	35.0	15.0	35.0	20.0	5.333	4.857	.795
10	35.0	19.0	37.0	16.0	4.511	4.301	1.162
11	35.0	17.0	39.0	18.0	1.882	1.837	1.039
12	35.0	15.0	37.0	20.0	1.654	1.644	.795
13	35.0	15.0	35.0	20.0	5.333	4.857	.795
14	35.0	15.0	39.0	20.0	1.671	1.628	.610
15	35.0	13.0	37.0	22.0	CENTER BELOW INTERPOLATED CR		
16	35.0	17.0	35.0	18.0	5.838	5.347	1.008
17	35.0	17.0	39.0	18.0	1.882	1.837	1.039
18	35.0	13.0	39.0	22.0	CENTER BELOW INTERPOLATED CR		
19	35.0	13.0	35.0	22.0	CENTER BELOW INTERPOLATED CR		

F.S. MINIMUM= 1.654 FOR THE CIRCLE OF CENTER (37.0, 20.0)

APPENDIX B

RESULTS OF STABGM ANALYSIS

Figure 2.5 depicts the conditions of the revised STABGM analysis. Detailed results of the revised analysis are contained herein. Analysis with no layers of reinforcement display the critical factor of safety exists at a depth of 10 feet below the embankment/foundation soil interface. Analyses with one layer of reinforcement are enclosed. Strengths of 5,000, 7,000, 9,000, and 8,000 pounds per foot were analyzed to obtain a minimum factor of safety of 1.0 at a strength of 8,000 pounds per foot. Analyses with two and three layers of reinforcement are also enclosed. Internal stability analyses with none, one, two, and three layers of reinforcement are enclosed for comparison with external stability analyses.

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0,

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	35.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 0 LEVEL (s)

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dfs(BS)
1	28.0	6.0	37.0	22.0	CENTER BELOW	INTERPOLATED	CF
2	28.0	6.0	37.0	22.0	CENTER BELOW	INTERPOLATED	CF
3	28.0	10.0	37.0	18.0	.719	.643	.000
4	28.0	6.0	41.0	22.0	CENTER BELOW	INTERPOLATED	CF
5	28.0	2.0	37.0	26.0	CIRCLE	OUTSIDE	SLOPE
6	28.0	10.0	35.0	18.0	.770	.679	.000
7	28.0	12.0	37.0	16.0	.710	.482	.000
8	28.0	10.0	39.0	18.0	.770	.679	.000

9	28.0	8.0	37.0	20.0	CENTER BELOW INTERPOLATED CR		
10	28.0	12.0	35.0	18.0	.779	.710	.000
11	28.0	12.0	39.0	16.0	.779	.710	.000
12	28.0	8.0	39.0	20.0	CENTER BELOW INTERPOLATED CR		
13	28.0	8.0	35.0	20.0	CENTER BELOW INTERPOLATED CR		

F.S. MINIMUM= .719 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED CR		
2	30.0	10.0	31.0	20.0	CENTER BELOW INTERPOLATED CR		
3	30.0	14.0	35.0	16.0	.668	.616	.000
4	30.0	10.0	39.0	20.0	CENTER BELOW INTERPOLATED CR		
5	30.0	6.0	35.0	24.0	CENTER BELOW INTERPOLATED CR		
6	30.0	14.0	33.0	16.0	.739	.679	.000
7	30.0	16.0	35.0	14.0	.682	.642	.000
8	30.0	14.0	37.0	16.0	.646	.600	.000
9	30.0	12.0	35.0	18.0	.665	.600	.000
10	30.0	16.0	37.0	14.0	.665	.628	.000
11	30.0	14.0	39.0	16.0	.668	.716	.000
12	30.0	12.0	37.0	18.0	.636	.580	.000
13	30.0	12.0	35.0	18.0	.663	.600	.000
14	30.0	12.0	39.0	18.0	.663	.600	.000
15	30.0	10.0	37.0	20.0	CENTER BELOW INTERPOLATED CR		
16	30.0	14.0	35.0	16.0	.668	.616	.000
17	30.0	14.0	39.0	16.0	.668	.616	.000
18	30.0	10.0	39.0	20.0	CENTER BELOW INTERPOLATED CR		
19	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED CR		

F.S. MINIMUM= .636 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED CR		
2	35.0	15.0	31.0	20.0	CENTER BELOW INTERPOLATED CR		
3	35.0	19.0	35.0	16.0	.601	.566	.000
4	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED CR		
5	35.0	11.0	35.0	24.0	CENTER BELOW INTERPOLATED CR		
6	35.0	19.0	37.0	16.0	.608	.589	.000
7	35.0	21.0	35.0	14.0	.605	.572	.000
8	35.0	19.0	37.0	16.0	.697	.559	.000
9	35.0	17.0	35.0	18.0	.605	.567	.000
10	35.0	21.0	37.0	14.0	.596	.565	.000
11	35.0	19.0	39.0	16.0	.601	.566	.000
12	35.0	17.0	37.0	18.0	.595	.559	.000
13	35.0	21.0	35.0	14.0	.605	.572	.000
14	35.0	21.0	39.0	14.0	.605	.572	.000
15	35.0	17.0	39.0	18.0	.605	.567	.000
16	35.0	17.0	35.0	18.0	.605	.567	.000

F.S. MINIMUM= .593 FOR THE CIRCLE OF CENTER (37.0, 18.0)

CRITICAL FACTOR OF SAFETY - 10 FEET BELOW BOTTOM OF EMBANKMENT

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0,

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	19.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 1 LEVEL(s)

Y=	25.00	NO. OF FORCL POINTS=	4
X		FORCE	
42.0		.0	
39.0		5000.0	
3.0		5000.0	
.0		.0	

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	135.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER TANGENT RADIUS (X) CENTER (Y) CENTER STABILIZER FS(ONS) GFS(ES)

1	28.0	6.0	37.0	22.0	CENTER BELOW INTERPOLATED OF			
2	28.0	6.0	35.0	22.0	CENTER BELOW INTERPOLATED OF			
3	28.0	10.0	37.0	19.0	1.707	1.627	.984	
4	29.0	6.0	41.0	22.0	CENTER BELOW INTERPOLATED OF			
5	28.0	2.0	37.0	26.0	CIRCLE OUTSIDE SLOPE			
6	28.0	10.0	35.0	18.0	1.811	1.718	1.039	
7	28.0	12.0	37.0	16.0	1.680	1.622	.940	
8	28.0	10.0	39.0	18.0	1.811	1.718	1.039	
9	28.0	8.0	37.0	20.0	CENTER BELOW INTERPOLATED OF			
10	28.0	12.0	35.0	16.0	1.758	1.688	.979	
11	28.0	14.0	37.0	14.0	1.685	1.641	.914	
12	28.0	12.0	39.0	16.0	1.758	1.688	.979	
13	28.0	14.0	35.0	14.0	1.747	1.695	.944	
14	28.0	14.0	39.0	14.0	1.747	1.695	.944	
15	28.0	10.0	39.0	18.0	1.811	1.718	1.039	
16	28.0	10.0	35.0	18.0	1.811	1.718	1.039	

F.S. MINIMUM= 1.680 FOR THE CIRCLE OF CENTER (37.0, 16.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	δFS(BS)
1	30.0	12.0	35.0	18.0	1.305	1.243	.643
2	30.0	12.0	31.0	18.0	1.842	1.699NEG. RESIST.	
3	30.0	16.0	35.0	14.0	1.317	1.277	.635
4	30.0	12.0	39.0	18.0	1.305	1.243	.643
5	30.0	8.0	35.0	22.0	CENTER BELOW INTERPOLATED OF		
6	30.0	12.0	33.0	18.0	1.474	1.387	.717
7	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		
8	30.0	14.0	33.0	16.0	1.433	1.364	.694
9	30.0	14.0	37.0	16.0	1.268	1.222	.621
10	30.0	10.0	37.0	20.0	CENTER BELOW INTERPOLATED OF		
11	30.0	10.0	33.0	20.0	CENTER BELOW INTERPOLATED OF		
12	30.0	16.0	37.0	14.0	1.086	1.049	.621
13	30.0	14.0	39.0	16.0	1.306	1.254	.638
14	30.0	16.0	35.0	14.0	1.317	1.277	.635
15	30.0	16.0	39.0	14.0	1.317	1.277	.635
16	30.0	12.0	39.0	18.0	1.305	1.243	.643
17	30.0	12.0	35.0	18.0	1.305	1.243	.643

F.S. MINIMUM= 1.268 FOR THE CIRCLE OF CENTER (37.0, 16.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 5 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	δFS(BS)
1	35.0	17.0	35.0	16.0	.859	.852	.056
2	35.0	17.0	31.0	16.0	1.027	.971	.056
3	35.0	21.0	35.0	14.0	1.307	1.288	.018
4	35.0	17.0	39.0	16.0	.859	.852	.056
5	35.0	17.0	35.0	22.0	CENTER BELOW INTERPOLATED OF		
6	35.0	17.0	37.0	2.0	.875	.877	.056
7	35.0	19.0	39.0	18.0	1.010	1.009	.056
8	35.0	17.0	41.0	16.0	1.002	.997	.056
9	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
10	35.0	17.0	35.0	18.0	.859	.852	.056
11	35.0	19.0	37.0	16.0	.892	.892	.056
12	35.0	15.0	37.0	20.0	CENTER BELOW INTERPOLATED OF		
13	35.0	19.0	35.0	16.0	.892	.889	.056

14	35.0	19.0	39.0	16.0	.904	.869	.307
15	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
16	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		

F.S. MINIMUM= .875 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 1
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 35.0,

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 1 LEVEL

Y=	X	NO. OF FORCE POINTS=	FORCE
25.00	42.0	4	.0
19.0	3.0		7000.0
1.0	1.0		7000.0

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	135.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER TANGENT RADIUS (X) CENTER (Y) CENTER FS BISHOP FS (ONS) dFB (BS)

1	35.0	13.0	37.0	22.0	CENTER BELOW INTERPOLATED OF
2	35.0	13.0	33.0	22.0	CENTER BELOW INTERPOLATED OF
3	35.0	17.0	37.0	18.0	.987 .951 .395
4	35.0	13.0	41.0	22.0	CENTER BELOW INTERPOLATED OF
5	35.0	9.0	37.0	26.0	CENTER BELOW RIGHT INTERSECT
6	35.0	17.0	33.0	18.0	1.003 .966 .395
7	35.0	19.0	37.0	16.0	1.012 .978 .417
8	35.0	17.0	39.0	18.0	1.003 .966 .395
9	35.0	15.0	37.0	20.0	CENTER BELOW INTERPOLATED OF
10	35.0	19.0	33.0	16.0	1.026 .990 .402
11	35.0	19.0	39.0	16.0	1.026 .990 .402
12	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF
13	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF

F.S. MINIMUM= .987 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 1
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 35.0,

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACK'S	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 1 LEVEL (a)

Y=	X	NO. OF FORCE POINTS=	FORCE
25.00		4	
40.0		.0	
39.0		9000.0	
2.0		9000.0	
.0		.0	

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER TANGENT RADIUS (X) CENTER (Y) CENTER FS (BISHOP) FR (OMS) BFS (BS)

1	35.0	12.0	37.0	22.0	CENTER BELOW INTERPOLATED OF
2	35.0	13.0	33.0	22.0	CENTER BELOW INTERPOLATED OF
3	35.0	17.0	37.0	18.0	1.099 1.063 .505
4	35.0	13.0	41.0	22.0	CENTER BELOW INTERPOLATED OF
5	35.0	9.0	37.0	26.0	CENTER BELOW RIGHT INTERSECT
6	35.0	17.0	35.0	18.0	1.117 1.080 .517
7	35.0	19.0	37.0	18.0	1.132 1.097 .539
8	35.0	17.0	39.0	18.0	1.117 1.080 .517
9	35.0	15.0	37.0	20.0	CENTER BELOW INTERPOLATED OF
10	35.0	19.0	35.0	16.0	1.147 1.111 .542
11	35.0	19.0	39.0	16.0	1.147 1.111 .542
12	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF
13	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF

F.S. MINIMUM= 1.099 FOR THE CIRCLE OF CENTER (37.0, 12.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 1
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 35.0,

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 1 LEVEL(s)

Y=	X	NO. OF FORCE POINTS=	FORCE
25.00		4	
42.0		.0	
39.0		8000.0	
3.0		8000.0	
.0		.0	

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	137.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER TANGENT RADIUS (X) CENTER (Y) CENTER FS (BISHOP) FS (OMS) dFS (B)

1	35.0	13.0	37.0	22.0	CENTER BELOW INTERPOLATED OF
2	35.0	13.0	33.0	22.0	CENTER BELOW INTERPOLATED OF
3	35.0	17.0	37.0	18.0	1.043 1.007 .449
4	35.0	13.0	41.0	22.0	CENTER BELOW INTERPOLATED OF
5	35.0	9.0	37.0	28.0	CENTER BELOW RIGHT INTERSECT
6	35.0	17.0	35.0	18.0	1.060 1.023 .456
7	35.0	19.0	37.0	16.0	1.072 1.038 .479
8	35.0	17.0	39.0	18.0	1.060 1.023 .456
9	35.0	15.0	37.0	20.0	CENTER BELOW INTERPOLATED OF
10	35.0	19.0	35.0	16.0	1.086 1.051 .485
11	35.0	19.0	39.0	16.0	1.086 1.051 .485
12	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF
13	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF

F.S. MINIMUM= 1.043 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0,

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	18.0	18.0	18.0	18.0	18.0	16.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 2 LEVEL(S)

Y= 22.00 NO. OF FORCE POINTS= 4
 X FORCE
 37.7 .0
 34.0 5000.0
 3.0 5000.0
 .0 .0

Y= 25.00 NO. OF FORCE POINTS= 4
 X FORCE
 42.0 .0
 39.0 5000.0
 7.0 5000.0
 .0 .0

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	130.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	28.0	6.0	37.0	22.0	CENTER BELOW INTERPOLATED OF		
2	28.0	6.0	33.0	22.0	CENTER BELOW INTERPOLATED OF		
3	28.0	10.0	37.0	19.0	2.265	2.189	1.341
4	28.0	6.0	41.0	22.0	CENTER BELOW INTERPOLATED OF		
5	28.0	2.0	37.0	26.0	CIRCLE OUTSIDE SLOPE		
6	28.0	10.0	35.0	18.0	2.404	2.312	1.632
7	28.0	12.0	37.0	16.0	2.307	2.249	1.567
8	28.0	10.0	39.0	18.0	2.404	2.312	1.632
9	28.0	8.0	37.0	20.0	CENTER BELOW INTERPOLATED OF		
10	28.0	12.0	35.0	16.0	2.410	2.341	1.631
11	28.0	12.0	39.0	16.0	2.410	2.341	1.631
12	28.0	8.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
13	28.0	8.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		

F.S. MINIMUM= 2.265 FOR THE CIRCLE OF CENTER (37.0, 19.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		
2	30.0	10.0	31.0	20.0	CENTER BELOW INTERPOLATED OF		
3	30.0	14.0	35.0	16.0	1.731	1.680	1.063
4	30.0	10.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
5	30.0	6.0	35.0	24.0	CENTER BELOW INTERPOLATED OF		
6	30.0	14.0	33.0	16.0	1.895	1.826	1.156
7	30.0	16.0	35.0	14.0	1.779	1.739	1.097
8	30.0	14.0	37.0	16.0	1.682	1.636	1.033
9	30.0	12.0	35.0	18.0	1.672	1.610	1.010
10	30.0	12.0	33.0	18.0	1.884	1.797	1.127
11	30.0	12.0	37.0	18.0	1.612	1.556	.976
12	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		
13	30.0	14.0	37.0	16.0	1.682	1.636	1.033
14	30.0	12.0	39.0	18.0	1.672	1.610	1.010
15	30.0	10.0	37.0	20.0	CENTER BELOW INTERPOLATED OF		
16	30.0	14.0	35.0	16.0	1.731	1.680	1.063
17	30.0	14.0	39.0	16.0	1.731	1.680	1.063
18	30.0	10.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
19	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		

F.S. MINIMUM= 1.512 FOR THE CIRCLE OF CENTER (37.0, 16.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		
2	35.0	15.0	31.0	20.0	CENTER BELOW INTERPOLATED OF		
3	35.0	19.0	35.0	16.0	1.105	1.071	.501
4	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
5	35.0	11.0	35.0	24.0	CENTER BELOW INTERPOLATED OF		
6	35.0	19.0	33.0	16.0	1.153	1.110	.527

7	35.0	21.0	35.0	14.0	1.149	1.118	.546
8	35.0	19.0	37.0	16.0	1.092	1.057	.499
9	35.0	17.0	35.0	18.0	1.052	1.015	.447
10	35.0	17.0	33.0	18.0	1.106	1.064	.464
11	35.0	17.0	37.0	18.0	1.015	.999	.44
12	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		
13	35.0	19.0	37.0	16.0	1.092	1.057	.499
14	35.0	17.0	39.0	18.0	1.052	1.015	.447
15	35.0	15.0	37.0	20.0	CENTER BELOW INTERPOLATED OF		
16	35.0	19.0	35.0	16.0	1.106	1.071	.507
17	35.0	19.0	39.0	16.0	1.106	1.071	.505
18	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
19	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		

F.S. MINIMUM= 1.035 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 3
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES TANGENT TO DEPTH, 28.0, 30.0, 35.0,

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 3 LEVEL(S)

Y= 20.00 NO. OF FORCE POINTS= 4
 X FORCE
 34.8 .0
 31.0 1666.0
 3.0 1666.0
 .0 .0

Y= 22.00 NO. OF FORCE POINTS= 4
 X FORCE
 37.7 .0
 34.0 5000.0
 3.0 5000.0
 .0 .0

Y= 25.00 NO. OF FORCE POINTS= 4
 X FORCE
 42.0 .0
 39.0 5000.0
 3.0 5000.0
 .0 .0

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	133.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	28.0	6.0	37.0	22.0	CENTER BELOW INTERPOLATED CR		
2	28.0	6.0	33.0	22.0	CENTER BELOW INTERPOLATED CR		
3	28.0	10.0	37.0	18.0	2.358	2.287	1.639
4	28.0	6.0	41.0	22.0	CENTER BELOW INTERPOLATED CR		
5	28.0	2.0	37.0	26.0	CIRCLE OUTSIDE SLOPE		
6	28.0	10.0	35.0	18.0	2.503	2.411	1.731
7	28.0	12.0	37.0	16.0	2.446	2.388	1.706
8	28.0	10.0	39.0	18.0	2.503	2.411	1.731
9	28.0	8.0	37.0	20.0	CENTER BELOW INTERPOLATED CR		
10	28.0	12.0	35.0	16.0	2.555	2.486	1.776
11	28.0	12.0	39.0	16.0	2.555	2.486	1.776
12	28.0	8.0	39.0	20.0	CENTER BELOW INTERPOLATED CR		
13	28.0	8.0	35.0	20.0	CENTER BELOW INTERPOLATED CR		

F.S. MINIMUM= 2.358 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BS)
1	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED CR		
2	30.0	10.0	31.0	20.0	CENTER BELOW INTERPOLATED CR		
3	30.0	14.0	35.0	16.0	1.826	1.774	1.158
4	30.0	10.0	39.0	20.0	CENTER BELOW INTERPOLATED CR		
5	30.0	6.0	35.0	24.0	CENTER BELOW INTERPOLATED CR		
6	30.0	14.0	33.0	16.0	1.998	1.929	1.259
7	30.0	16.0	35.0	14.0	1.895	1.854	1.212
8	30.0	14.0	37.0	16.0	1.774	1.728	1.127
9	30.0	12.0	35.0	18.0	1.734	1.671	1.071
10	30.0	12.0	33.0	18.0	1.952	1.865	1.195
11	30.0	12.0	37.0	18.0	1.671	1.615	1.035
12	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED CR		
13	30.0	14.0	37.0	16.0	1.774	1.728	1.127
14	30.0	12.0	39.0	18.0	1.734	1.671	1.071
15	30.0	10.0	37.0	20.0	CENTER BELOW INTERPOLATED CR		
16	30.0	14.0	35.0	16.0	1.806	1.774	1.158
17	30.0	14.0	39.0	16.0	1.906	1.774	1.158
18	30.0	10.0	39.0	20.0	CENTER BELOW INTERPOLATED CR		
19	30.0	10.0	35.0	20.0	CENTER BELOW INTERPOLATED CR		

F.S. MINIMUM= 1.671 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS (BISHOP)	FS (OMS)	dFS (BS)
1	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		
2	35.0	15.0	31.0	20.0	CENTER BELOW INTERPOLATED OF		
3	35.0	19.0	35.0	16.0	1.151	1.116	.550
4	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
5	35.0	11.0	35.0	24.0	CENTER BELOW INTERPOLATED OF		
6	35.0	19.0	33.0	16.0	1.200	1.160	.572
7	35.0	21.0	35.0	14.0	1.207	1.175	.600
8	35.0	19.0	37.0	16.0	1.136	1.102	.547
9	35.0	17.0	35.0	18.0	1.079	1.042	.473
10	35.0	17.0	33.0	18.0	1.134	1.092	.497
11	35.0	17.0	37.0	18.0	1.062	1.026	.467
12	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		
13	35.0	19.0	37.0	16.0	1.136	1.102	.547
14	35.0	17.0	39.0	18.0	1.079	1.042	.473
15	35.0	15.0	37.0	20.0	CENTER BELOW INTERPOLATED OF		
16	35.0	19.0	35.0	16.0	1.151	1.116	.550
17	35.0	19.0	39.0	16.0	1.151	1.116	.550
18	35.0	15.0	39.0	20.0	CENTER BELOW INTERPOLATED OF		
19	35.0	15.0	35.0	20.0	CENTER BELOW INTERPOLATED OF		

F.S. MINIMUM= 1.062 FOR THE CIRCLE OF CENTER (37.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 0
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES PASS THROUGH THE POINT (42.0, 35.0)

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACK S	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 0 LEVEL(S)

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS-BISHOP	FS(CMS)	DFS(S)
1	35.9	15.9	37.0	22.0	CENTER BELOW	INTERPOLATED	CS
2	35.9	15.9	37.0	22.0	CENTER BELOW	INTERPOLATED	CS
3	35.9	17.7	37.0	19.0	.457	.731	.000
4	35.0	17.0	41.0	22.0	CENTER BELOW	INTERPOLATED	CS
5	36.3	19.7	37.0	26.0	CENTER BELOW	RIGHT INTERSECT	
6	36.4	18.1	35.0	18.0	.405	.787	.000
7	35.6	16.2	37.0	14.0	.411	.840	.000
8	35.3	17.3	39.0	18.0	.510	.489	.000

9	35.8	15.8	37.0	20.0	CENTER BELOW INTERPOLATED OF		
10	37.2	19.2	33.0	18.0	.373	.358	.000
11	36.2	20.2	35.0	16.0	.411	.395	.000
12	36.6	16.6	35.0	20.0	CENTER BELOW INTERPOLATED OF		
13	38.2	20.2	31.0	18.0	2.397	2.238	.000
14	37.0	21.0	33.0	16.0	.378	.365	.000
15	37.5	17.5	33.0	20.0	CENTER BELOW INTERPOLATED OF		
16	38.0	22.0	31.0	16.0	2.841	2.572	.000
17	36.2	20.2	35.0	16.0	.411	.395	.000
18	36.6	16.6	35.0	20.0	CENTER BELOW INTERPOLATED OF		
19	38.6	18.6	31.0	20.0	CENTER BELOW INTERPOLATED OF		

F.S. MINIMUM= .373 FOR THE CIRCLE OF CENTER (33.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 0
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES PASS THROUGH THE POINT (42.0, 35.0)

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	35.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 1 LEVEL (s)

V= 25.00 NO. OF FORCE POINTS= 4
 X FORCE
 42.0 .0
 39.0 5000.0
 3.0 5000.0
 .0 .0

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	75.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER TANGENT RADII (X) CENTER (Y) CENTER (Z) CENTER (W) CENTER (V) CENTER (U) CENTER (T) CENTER (S) CENTER (R) CENTER (Q) CENTER (P) CENTER (O) CENTER (N) CENTER (M) CENTER (L) CENTER (K) CENTER (J) CENTER (I) CENTER (H) CENTER (G) CENTER (F) CENTER (E) CENTER (D) CENTER (C) CENTER (B) CENTER (A) CENTER

1	35.9	17.9	37.0	22.0	CENTER BELOW INTERPOLATED OF
2	37.8	15.8	33.0	22.0	CENTER BELOW INTERPOLATED OF
3	35.7	17.7	37.0	18.0	.714 .691 .200
4	35.0	17.0	41.0	22.0	CENTER BELOW INTERPOLATED OF
5	36.3	10.3	37.0	26.0	CENTER BELOW RIGHT INTERSECT
6	36.4	18.4	35.0	18.0	.444 .627 .240
7	35.6	19.6	37.0	16.0	.747 .722 .287
8	35.7	17.7	39.0	1.0	.797 .769 .287
9	35.8	15.8	37.0	20.0	CENTER BELOW INTERPOLATED OF
10	37.2	19.2	32.0	15.0	.592 .597 .225
11	36.2	20.2	35.0	16.0	.673 .658 .262
12	36.6	16.6	33.0	20.0	CENTER BELOW INTERPOLATED OF
13	38.2	20.2	31.0	18.0	2.609 2.449 .211
14	37.0	21.0	33.0	16.0	.627 .614 .249
15	37.5	17.5	33.0	20.0	CENTER BELOW INTERPOLATED OF
16	38.0	22.0	31.0	16.0	3.075 2.806 .234
17	36.2	20.2	35.0	16.0	.673 .658 .262
18	36.6	16.6	35.0	20.0	CENTER BELOW INTERPOLATED OF
19	38.6	18.6	31.0	20.0	CENTER BELOW INTERPOLATED OF

F.S. MINIMUM= .598 FOR THE CIRCLE OF CENTER (33.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 0
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES PASS THROUGH THE POINT (42.0, 35.0)

GEOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACKS	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	35.0
BOUNDARY 4	35.0	35.0	28.0	30.0	35.0	35.0	35.0	35.0	70.0	38.0	35.0
BOUNDARY 5	75.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	75.0	35.0

REINFORCING FORCE DATA AT 2 LEVEL (s)

Y= 22.00 NO. OF FORCE POINTS= 4
 X FORCE
 37.7 .0
 34.0 5000.0
 3.0 5000.0
 .0 .0

Y= 25.00 NO. OF FORCE POINTS= 4
 X FORCE
 42.0 .0
 39.0 5000.0
 3.0 5000.0
 .0 .0

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	100.0	40.0	120.0
3	100.0	.0	105.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(OMS)	dFS(BE)
1	35.9	13.9	37.0	22.0	CENTER BELOW	INTERPOLATED CR	
2	37.9	15.9	33.0	22.0	CENTER BELOW	INTERPOLATED CR	
3	35.7	17.7	37.0	18.0	.862	.840	.407
4	35.0	13.0	41.0	22.0	CENTER BELOW	INTERPOLATED CR	
5	36.3	10.3	37.0	26.0	CENTER BELOW	RIGHT INTERSECT	
6	36.4	18.4	35.0	18.0	.781	.764	.377
7	35.6	19.6	37.0	16.0	.932	.911	.471
8	35.5	17.3	39.0	18.0	.959	.929	.440
9	35.8	15.8	37.0	20.0	CENTER BELOW	INTERPOLATED CR	
10	37.2	19.2	33.0	18.0	.726	.711	.354
11	36.2	20.2	35.0	16.0	.848	.832	.437
12	36.6	16.6	35.0	20.0	CENTER BELOW	INTERPOLATED CR	
13	38.2	20.2	31.0	18.0	2.729	2.570	.332
14	37.0	21.0	33.0	16.0	.795	.780	.414
15	37.5	17.5	33.0	20.0	CENTER BELOW	INTERPOLATED CR	
16	38.0	22.0	31.0	16.0	3.231	2.962	.390
17	36.2	20.2	35.0	16.0	.848	.832	.437
18	36.6	16.6	35.0	20.0	CENTER BELOW	INTERPOLATED CR	
19	38.6	18.6	31.0	20.0	CENTER BELOW	INTERPOLATED CR	

F.S. MINIMUM= .726 FOR THE CIRCLE OF CENTER (33.0, 18.0)

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

CONTROL DATA

NUMBER OF SPECIFIED CENTERS 0
 NUMBER OF DEPTH LIMITING TANGENTS 0
 NUMBER OF VERTICAL SECTIONS 12
 NUMBER OF SOIL LAYER BOUNDARIES 5
 NUMBER OF PORE PRESSURE LINES 0
 NUMBER OF POINTS DEFINING COHESION PROFILE 0

SEISMIC COEFFICIENT S1,S2 = .00, .00

UNIT WEIGHT OF WATER = 62.40

SEARCH IS BASED ON BISHOP MODIFIED METHOD

SEARCH STARTS AT CENTER (37.0, 22.0) WITH FINAL GRID OF 2.0

ALL CIRCLES PASS THROUGH THE POINT (42.0, 35.0)

GOMETRY

SECTIONS	.0	10.0	13.0	15.0	20.0	32.0	42.0	64.0	69.0	71.0	74.0
T. CRACK'S	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
W IN CRACK	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 1	18.0	18.0	18.0	18.0	18.0	18.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 2	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 3	35.0	35.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0	25.0
BOUNDARY 4	35.0	35.0	38.0	30.0	35.0	35.0	35.0	35.0	30.0	28.0	25.0
BOUNDARY 5	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0	35.0

REINFORCING FORCE DATA AT 3 LEVEL(S)

Y= 20.00 NO. OF FORCE POINTS= 4
 Y FORCE
 34.8 .0
 31.0 1666.0
 3.0 1666.0
 .0 .0

Y= 22.00 NO. OF FORCE POINTS= 4
 Y FORCE
 37.7 .0
 34.0 5000.0
 3.0 5000.0
 .0 .0

Y= 25.00 NO. OF FORCE POINTS= 4
 Y FORCE
 42.0 .0
 39.0 5000.0
 3.0 5000.0
 .0 .0

SOIL PROPERTIES

LAYER	COHESION	FRICTION ANGLE	DENSITY
1	.0	35.0	135.0
2	1000.0	40.0	120.0
3	100.0	.0	105.0
4	1000.0	40.0	120.0

BISHOP MODIFIED AND/OR ORDINARY METHOD OF SLICES

permafrost depth 7 feet, 10 feet from centerline

NUMBER	TANGENT	RADIUS	(X) CENTER	(Y) CENTER	FS(BISHOP)	FS(QMS)	dFS(BS)
1	35.9	13.9	37.0	22.0	CENTER BELOW INTERPOLATED CR		
2	37.8	15.8	33.0	22.0	CENTER BELOW INTERPOLATED CR		
3	35.7	17.7	37.0	18.0	.887	.865	.434
4	35.0	13.0	41.0	22.0	CENTER BELOW INTERPOLATED CR		
5	36.3	10.3	37.0	26.0	CENTER BELOW RIGHT INTERSECT		
6	36.4	18.4	35.0	18.0	.804	.786	.399
7	35.6	19.6	37.0	16.0	.974	.953	.513
8	35.3	17.3	39.0	18.0	.986	.956	.467
9	35.8	15.8	37.0	20.0	CENTER BELOW INTERPOLATED CR		
10	37.2	19.2	33.0	18.0	.748	.733	.375
11	36.2	20.2	35.0	16.0	.887	.871	.476
12	36.6	16.6	35.0	20.0	CENTER BELOW INTERPOLATED CR		
13	38.2	20.2	31.0	18.0	2.749	2.590	.352
14	37.0	21.0	33.0	16.0	.829	.816	.451
15	37.5	17.5	33.0	20.0	CENTER BELOW INTERPOLATED CR		
16	38.0	22.0	31.0	16.0	3.266	2.997	.425
17	36.2	20.2	35.0	16.0	.887	.871	.476
18	36.6	16.6	35.0	20.0	CENTER BELOW INTERPOLATED CR		
19	38.6	18.6	31.0	20.0	CENTER BELOW INTERPOLATED CR		

F.S. MINIMUM= .748 FOR THE CIRCLE OF CENTER (33.0, 18.0)

APPENDIX C

PROCUREMENT OF SSTIPN PROGRAM

The procurement of computer software at Georgia Tech must follow Electronic Data Processing Equipment Request Procedures (EDP) under an executive order from the Governor. The procurement of SSTIPN is summarized as follows:

1. EPD request number 11-EC-86 was submitted for approval (enclosed).
2. Approval letter for EDP request number 11-EC-86 (enclosed).
3. Georgia Institute of Technology requisition form for SSTIPN (enclosed).
4. Letter of request for purchase of SSTIPN to Professor Duncan (enclosed).
5. Acceptance letter Professor Duncan (enclosed).

TOP SECRET

PROJECT NUMBER:

11-EC-86

ORIGINATOR:

Georgia Institute of Technology
College of Engineering
Department of Civil Engineering

MAILING ADDRESS:

Atlanta, GA 30332

LOCATION OF EQUIPMENT:

Geotechnical Building

PERSON RESPONSIBLE:

Neil G. Williams, Ph.D.

TELEPHONE:

404/874 1221

TYPE OF REQUEST:

_____ New System _____

EQUIPMENT REQUESTED:

Software: Soil Structure Interactive _____
Language: IPM

COST OF EQUIPMENT:

The anticipated cost for the software is \$10,000. The source of funds for this equipment will be for research provided by Hincley, Inc.

WORK TO BE PERFORMED AND WORKLOAD:

The two year program will be used to analyze the geotechnical load structure combination on a pile in Fairbanks, Alaska. The analysis will yield design data for the geotechnical material which will be used in the design of the pile.

The program will be utilized by one graduate student working approximately 20 hours per week. Future applications of the program to other problems will be considered.

ALTERNATIVES CONSIDERED:

The other alternative considered was to purchase a program to produce the same results that the current program produces.

RECOMMENDED ALTERNATIVE:

Acquiring the current program is recommended. The cost of the program is \$10,000. The program will be used for two years. The program will be used by one graduate student working approximately 20 hours per week. Future applications of the program to other problems will be considered.

Best Available Copy

1. PURPOSE AND SCOPE

2. REFERENCES

3. DEFINITIONS

4. PROCEDURES

5. RESULTS

Best Available Copy

BOARD OF REGENTS OF THE UNIVERSITY SYSTEM OF GEORGIA

PRESIDENT'S OFFICE BUILDING
ATHENS, GEORGIA 30602

OFFICE OF ASSISTANT VICE CHANCELLOR
FOR COMPUTING SYSTEMS

January 31, 1986

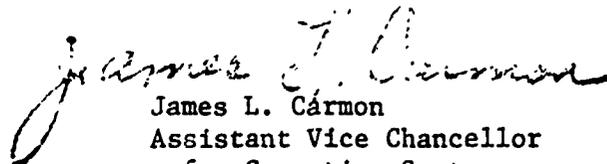
Dr. Joseph M. Pettit
President
Georgia Institute of Technology
225 North Avenue, N.W.
Atlanta, GA 30332

Dear President Pettit:

Approval is given to Georgia Institute of Technology to acquire the
data processing equipment referenced in request number(s):

 12-AT-86

Yours very truly,


James L. Carmon
Assistant Vice Chancellor
for Computing Systems

JLC/jw

cc: Mr. John Gehl

GEORGIA INSTITUTE OF TECHNOLOGY

SCHOOL OF CIVIL ENGINEERING

DATE 1/22/56

SUPPLIES

Signature of person requesting [Signature]

Signature of faculty member supervising [Signature]

Signature of cognizant staff that these items are not in stock [Signature]

Estimated Cost 200.⁰⁰

On Campus _____ Off Campus _____ State Funds _____ Project Funds

Vendor Virginia Tech Foundation

Items Computer Program SSTIPN (on tape), source listing, and user manuals

E20-693

Requisition Number 200-6-25147

Approved [Signature]

J. E. Fitzgerald, Director
School of Civil Engineering

Professor J. M. Duncan
Department of Civil Engineering
104 Patton Hall
Virginia Tech
Blacksburg, VA 24061

Re: Professor Duncan.

The finite element analysis program, Soil Structure Interaction Program Non-linear (SSTIFN), we discussed in our phone conversation, on January 17 will be very useful for our analysis of a geotextile reinforced road embankment. I am requesting that you send a copy of SSTIFN with source listing and user manual to the following address: Neil D. Williams, Ph.D.
c/o George Papaioannu
Georgia Institute of Technology
Department of Civil Engineering
Atlanta, GA 30332

It is my understanding from our phone conversation that the cost of the above items will be \$200.00. The requisition number applicable to this purchase is #300-3-25147.

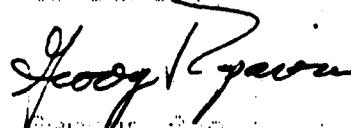
The following tape format information is provided for your transfer of the program:

80 character records
16 records per block
7 blocks
Coded in ASCII or EBCDIC
Density - 1600 or 3250 CPI

In addition, please specify the program tape format of the tape you send.

If further information is required please contact Neil D. Williams or me at (404)374-6025.

Thank you for your effort and time in response.

George Papaioannu

George Papaioannu



COLLEGE OF ENGINEERING

VIRGINIA POLYTECHNIC INSTITUTE AND STATE UNIVERSITY

Blacksburg, Virginia 24061

DEPARTMENT OF CIVIL ENGINEERING (703) 961-6635

February 13, 1986

INVOICE

Mr. George Papaioanou
c/o Dr. Neil D. Williams
Georgia Institute of Technology
Department of Civil Engineering
Atlanta, GA 30332

REF: Requisition No. 200-6-25147

For computer program SSTIPN (Soil-Structure Interaction Program)
sent via BITNET on February 13, 1986.

SSTIPN \$200.00

Payable upon receipt.

Mail payment to:

Professor J. M. Duncan
Department of Civil Engineering
104 Patton Hall
Virginia Tech
Blacksburg, VA 24061

Please make check payable to: The Virginia Tech Foundation, Inc.

APPENDIX D

BASIC PLOTTING PROGRAM

The BASIC program for plotting FEADAM84 and SSTIPN reduced output was written by Robert L. Roglin (21). The program was written for use on an IBM PC with enhanced computer graphics. Output data from FEADAM84 and SSTIPN was reduced for use in the plotting program. Four data files were reduced from the output: Nodal point input data, four node solid element data, Nodal point displacement data, and four node element stress data. Three dimensional plots of stresses were not useful in evaluating stress distributions within the embankment and are not included. Program source listing is enclosed.

```

10 REM      PROGRAM:  GRAPH1
20 REM
30 REM      USER:    R.L. ROGLIN  RESEARCH ENG II STRI
40 REM      GEORGE PAPAIOANNOJ  SPECIAL PROBLEMS
50 REM      ABSTRACT: THIS PROGRAM IS USED TO PLOT A TWO DIMENSIONAL FINITE
60 REM      ELEMENT GRID IN THE UNDEFORMED AND DEFORMED STATES
70 REM
80 REM -----
90 DIM JT(70),X(70),Y(70),DX(3,70),DY(2,70)
100 DIM EL(50),I1(50),I2(50),I3(50),I4(50),EX(50),EY(50)
110 DIM SX(3,50),SY(3,50),SX1(3,50),XS(50),YS(50)
120 DIM ELS(10),ELF(10)
130 REM
140 REM
150 REM  Open Files For Input "JOINTS" "ELEMENTS" "DISPL" "STRESSES"
160 REM
170 REM -----
180 REM
190 OPEN "JOINTS.DAT" FOR INPUT AS #1
200 INPUT #1, NJT
210 FOR I = 1 TO NJT
220   INPUT #1,JT(I),X(JT(I)),Y(JT(I))
230 NEXT I
240 CLOSE #1
250 OPEN "ELEMENTS.DAT" FOR INPUT AS #1
260 INPUT #1, NEL
270 FOR I = 1 TO NEL
280   INPUT #1,EL(I),I1(EL(I)),I2(EL(I)),I3(EL(I)),I4(EL(I)),EX(EL(I)),EY(EL(I))
290 NEXT I
300 INPUT #1,NLR
310 FOR I = 1 TO NLR
320   INPUT #1,LAY,ELS(LAY),ELF(LAY)
330 NEXT I
340 CLOSE #1
350 OPEN "DISPL.DAT" FOR INPUT AS #1
360 INPUT #1, NLC
370 FOR J = 1 TO NLC
380   FOR I = 1 TO NJT
390     INPUT #1, JT(I),DX(J,JT(I)),DY(J,JT(I))
400   NEXT I
410 NEXT J
420 CLOSE #1
430 OPEN "STRESSES.DAT" FOR INPUT AS #1
440 FOR J = 1 TO NLC
450   FOR I = 1 TO NEL
460     INPUT #1,EL(I),SX(J,EL(I)),SY(J,EL(I)),SX1(J,EL(I))
470   NEXT I
480 NEXT J
490 CLOSE #1
500 REM
510 REM
520 REM      Displaced Shape Plotting
530 REM
540 REM -----
550 REM
560 INPUT "HORIZONTAL SCALE FACTOR: " ; HSCALE
570 INPUT "VERTICAL SCALE FACTOR: " ; VSCALE
580 INPUT "HORIZONTAL DISPLACEMENT SCALE FACTOR: " ; DHSCALE
590 INPUT "VERTICAL DISPLACEMENT SCALE FACTOR: " ; DVSCALE
600 INPUT "LOAD CASE NUMBER: " ; LNO
610 SCREEN 1,0
620 COLOR ,0
630 WINDOW (-10,-7) : (-50,15)
640 REM
650 REM

```

```

670 REM
680 REM -----
690 REM
700 LINE (0,0) - (400,0): LINE (0,0) - (0,100)
710 REM
720 REM
730 REM   Plot Undeformed Geometry
740 REM
750 REM -----
760 REM
770 FOR I = 1 TO NEL
780   X1 = HSCALE*X(I1(EL(I))): Y1 = VSCALE*Y(I1(EL(I)))
790   X2 = HSCALE*X(I2(EL(I))): Y2 = VSCALE*Y(I2(EL(I))): LINE (X1,Y1) - (X2,Y2)
800   X3 = HSCALE*X(I3(EL(I))): Y3 = VSCALE*Y(I3(EL(I))): LINE (X3,Y3)
810   X4 = HSCALE*X(I4(EL(I))): Y4 = VSCALE*Y(I4(EL(I))): LINE (X4,Y4)
820   LINE (X1,Y1)
830 NEXT I
840 REM
850 REM
860 REM   Plot Deformed Geometry
870 REM
880 REM -----
890 REM
900 FOR I = 1 TO NJT
910   X1 = HSCALE*X(I1(EL(I))) + DHSCALE*DX(LC,I1(EL(I)))
920   X2 = HSCALE*X(I2(EL(I))) + DHSCALE*DX(LC,I2(EL(I)))
930   X3 = HSCALE*X(I3(EL(I))) + DHSCALE*DX(LC,I3(EL(I)))
940   X4 = HSCALE*X(I4(EL(I))) + DHSCALE*DX(LC,I4(EL(I)))
950   Y1 = VSCALE*Y(I1(EL(I))) + DVSCALE*DY(LC,I1(EL(I)))
960   Y2 = VSCALE*Y(I2(EL(I))) + DVSCALE*DY(LC,I2(EL(I)))
970   Y3 = VSCALE*Y(I3(EL(I))) + DVSCALE*DY(LC,I3(EL(I)))
980   Y4 = VSCALE*Y(I4(EL(I))) + DVSCALE*DY(LC,I4(EL(I)))
990   LINE (X1,Y1) - (X2,Y2),2
1000  LINE (X3,Y3),2
1010  LINE (X4,Y4),2
1020  LINE (X1,Y1),2
1030 NEXT I
1040 REM
1050 REM
1060 REM   Stress Contour Plotting
1070 REM
1080 REM -----
1090 REM
1100 REM
1110 REM   Develop Coordinate Transformation Equations
1120 REM
1130 REM -----
1140 REM
1150 INPUT "SPIN ANGLE, TIP ANGLE: ": SPIN,TIP
1160 CSPIN = COS(SPIN*%PI/180): SSPIN = SIN(SPIN*%PI/180)
1170 CTIP = COS(TIP*%PI/180): STIP = SIN(TIP*%PI/180)
1180 PRINT "cspin = ",CSPIN," sspin = ",SSPIN
1190 PRINT "ctip = ",CTIP," stip = ",STIP
1200 INPUT "STRESS, LOAD: ": STRESS,LD
1210 CLS
1220 FOR I = 1 TO NEL
1230   IF STRESS = 1 THEN SIG = ST*(LD/EL(I))
1240   IF STRESS = 2 THEN SIG = SY*(LD/EL(I))
1250   IF STRESS = 3 THEN SIG = ST*(LD/EL(I))
1260   XS(EL(I)) = C*(EL(I)*COS(SPIN) + SSPIN*%PI)
1270   YS(EL(I)) = C*(EL(I)*SIN(SPIN) + STIP*%PI)
1280 NEXT I
1290 REM
1300 REM
1310 REM   Plot Stress Contours

```

```

1330 REM
1340 REM
1350 REM
1360 REM
1370 REM   Plot Global Axis
1380 REM
1390 REM -----
1400 REM
1410 XSX = 100*CSPIN: YSX = -100*SSPIN*STIP: LINE (0,0) - (XSX,YSX),1
1420 XSY = 0: YSY = 100*CTIP: LINE (0,0) - (XSY,YSY),2
1430 XSZ = 100*SSPIN: YSZ = 100*CSPIN*STIP: LINE (0,0) - (XSZ,YSZ),3
1440 FOR I = 1 TO NEL
1450 X1 = HSCALE*X(I1(EL(I))): Y1 = VSCALE*Y(I1(EL(I)))
1460 Y1 = Y1*CTIP + X1*STIP*(-1)*SSPIN: X1 = X1*CSPIN
1470 X2 = HSCALE*X(I2(EL(I))): Y2 = VSCALE*Y(I2(EL(I)))
1480 Y2 = Y2*CTIP + X2*STIP*(-1)*SSPIN: X2 = X2*CSPIN: LINE (X1,Y1) - (X2,Y2)
1490 X3 = HSCALE*X(I3(EL(I))): Y3 = VSCALE*Y(I3(EL(I)))
1500 Y3 = Y3*CTIP + X3*STIP*(-1)*SSPIN: X3 = X3*CSPIN: LINE - (X2,Y2)
1510 X4 = HSCALE*X(I4(EL(I))): Y4 = VSCALE*Y(I4(EL(I)))
1520 Y4 = Y4*CTIP + X4*STIP*(-1)*SSPIN: X4 = X4*CSPIN: LINE - (X4,Y4)
1530 LINE - (X1,Y1)
1540 NEXT I
1550 FOR K = 1 TO NLR
1560 K1 = (NLR + 1) - K
1570 J = 0
1580 FOR I = ELS(K1) TO (ELF(K1) - 1)
1590   IB = ELF(K1) - J: IE = ELF(K1) - J - 1
1600   LINE (XS(IB),YS(IB)) - (XS(IE),YS(IE))
1610   IF K1 = 1 THEN GOTO 1640
1620   IE = ELF(K1 - 1) - J
1630   LINE (XS(IB),YS(IB)) - (XS(IE),YS(IE))
1640   J = J + 1
1650 NEXT I
1660 IF K1 = 1 THEN GOTO 1700
1670 IB = ELF(K1) - J: IE = ELF(K1 - 1) - J
1680 LINE (XS(IB),YS(IB)) - (XS(IE),YS(IE))
1690 NEXT K
1700 REM
1710 STOP

```

APPENDIX E

SSTIPN OUTPUT DATA

Detailed results of the SSTIPN analyses are contained herein. Output data includes nodal point displacements, soil element stress and strain data, and internal forces of structural elements. Analyses include output for all five different geotextile strengths. Each of the five geotextile strengths include three different conditions as stated in Section 3.2.

SSTIPN: 1 LAYER OF GEOTEXTILE, P=1000 PPI, T=0

TOTAL NUMBER OF NODES-----	95
NUMBER OF BAR ELEMENTS-----	9
NUMBER OF DIFF. BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF. BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO. OF INTERFACE ELE. IN PREEXIST. PART-----	0
NUMBER OF INTERFACE ELE. IN FOUNDATION-----	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	75
NUMBER OF DIFF. SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	35
NUMBER OF NODES IN FOUNDATION-----	52
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	6
NUMBER OF LOAD CASES-----	1

CALINE FACTOR ----- 1.00000

ATMOSPHERIC PRESSURE --- 1.05800

UNIT WEIGHT OF WATER --- 0.2120

DISPLACEMENT SEQUENCE FOR A TOTAL OF 7 INCREMENTS

INCREMENT NO. 1	PUT ON LAYER NO. 1
INCREMENT NO. 2	PUT ON LAYER NO. 2
INCREMENT NO. 3	PUT ON LAYER NO. 3
INCREMENT NO. 4	PUT ON LAYER NO. 4
INCREMENT NO. 5	PUT ON LAYER NO. 5
INCREMENT NO. 6	PUT ON LAYER NO. 6
INCREMENT NO. 7	APPLY LOAD CASE 1

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		S.C. CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	000	000	1	1	1
2	10.000	0.000	1	1	1
3	20.000	0.000	1	1	1
4	30.000	0.000	1	1	1
5	38.000	0.000	1	1	1
6	42.000	0.000	1	1	1
7	50.000	0.000	1	1	1
8	54.000	0.000	1	1	1
9	58.000	0.000	1	1	1
10	68.000	0.000	1	1	1
11	74.000	0.000	1	1	1
12	82.000	0.000	1	1	1
13	90.000	0.000	1	1	1
14	0.000	4.000	0	0	0
15	10.000	4.000	0	0	0
16	20.000	4.000	0	0	0
17	30.000	4.000	0	0	0
18	38.000	4.000	0	0	0
19	42.000	4.000	0	0	0
20	50.000	4.000	0	0	0
21	54.000	4.000	0	0	0
22	58.000	4.000	0	0	0
23	68.000	4.000	0	0	0
24	74.000	4.000	0	0	0
25	82.000	4.000	0	0	0
26	90.000	4.000	0	0	0
27	0.000	7.000	1	0	1
28	10.000	7.000	0	0	0
29	20.000	7.000	0	0	0
30	30.000	7.000	0	0	0
31	38.000	7.000	0	0	0
32	42.000	7.000	0	0	0
33	50.000	7.000	0	0	0
34	54.000	7.000	0	0	0
35	58.000	7.000	0	0	0
36	68.000	7.000	0	0	0
37	74.000	7.000	0	0	0
38	82.000	7.000	0	0	0
39	90.000	7.000	1	0	1
40	0.000	10.000	1	0	1
41	10.000	10.000	0	0	0
42	20.000	10.000	0	0	0
43	30.000	10.000	0	0	0
44	38.000	10.000	0	0	0
45	42.000	10.000	0	0	0
46	50.000	10.000	0	0	0
47	54.000	10.000	0	0	0
48	58.000	10.000	0	0	0
49	68.000	10.000	0	0	0
50	74.000	10.000	0	0	0
51	82.000	10.000	1	0	1
52	90.000	10.000	0	0	0
53	38.000	11.500	0	0	0
54	42.000	11.500	0	0	0
55	50.000	11.500	0	0	0
56	54.000	11.500	0	0	0
57	58.000	11.500	0	0	0
58	68.000	11.500	0	0	0
59	74.000	11.500	0	0	0
60	82.000	11.500	0	0	0
61	90.000	11.500	1	0	1
62	42.000	13.000	0	0	0
63	50.000	13.000	0	0	0
64	54.000	13.000	0	0	0
65	58.000	13.000	0	0	0
66	68.000	13.000	0	0	0
67	74.000	13.000	0	0	0
68	82.000	13.000	0	0	0
69	90.000	13.000	1	0	1
70	48.000	14.000	0	0	0
71	50.000	14.000	0	0	0
72	54.000	14.000	0	0	0
73	58.000	14.000	0	0	0
74	68.000	14.000	0	0	0
75	74.000	14.000	0	0	0
76	82.000	14.000	0	0	0
77	90.000	14.000	1	0	1
78	50.000	15.000	0	0	0
79	54.000	15.000	0	0	0
80	58.000	15.000	0	0	0
81	68.000	15.000	0	0	0
82	74.000	15.000	0	0	0
83	82.000	15.000	0	0	0
84	90.000	15.000	1	0	1
85	54.000	16.000	0	0	0
86	58.000	16.000	0	0	0
87	68.000	16.000	0	0	0
88	74.000	16.000	0	0	0
89	82.000	16.000	0	0	0
90	90.000	16.000	1	0	1
91	58.000	17.000	0	0	0
92	68.000	17.000	0	0	0
93	74.000	17.000	0	0	0
94	82.000	17.000	0	0	0
95	90.000	17.000	1	0	1

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	30	1.00	0

ELMY NO.	CONNECTED NODES I J	MATL NO.
1	43 44	1
2	44 45	1
3	45 46	1
4	46 47	1
5	47 48	1
6	48 49	1
7	49 50	1
8	50 51	1
9	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S MODULUS		RATIO	BULK MODULUS		STRENGTH PARAMETERS			
		CONSTANT	EXPONENT		CONSTANT	EXPONENT	C	PHI	DPHI	K0
1	.0880	800.00	.500	.500	1800.00	.800	.00	35.00	.00	.50
2	.0830	40.00	.300	.500	20.00	.200	.08	0.00	.00	.50
3	.0800	1000.00	.400	.700	500.00	.500	.50	40.00	.00	.50

FOUR NODES SOLID ELEMENT DATA

ELET NO.	CONNECTED NODES I J K L	MATL NO.	ELEMENT CENTER X-ORD	Y-ORD
1	1 2 15 14	3	5.000	2.000
2	2 3 16 15	3	15.000	2.000
3	3 4 17 16	3	25.000	2.000
4	4 5 18 17	2	33.000	2.000
5	5 6 19 18	2	39.000	2.000
6	6 7 20 19	2	46.000	2.000
7	7 8 21 20	2	52.000	2.000
8	8 9 22 21	2	58.000	2.000
9	9 10 23 22	3	62.000	2.000
10	10 11 24 23	3	70.000	2.000
11	11 12 25 24	3	78.000	2.000
12	12 13 26 25	3	88.000	2.000
13	13 14 27 26	3	5.000	5.500
14	14 15 28 27	3	15.000	5.500
15	15 16 29 28	2	25.000	5.500
16	16 17 30 29	2	33.000	5.500
17	17 18 31 30	2	39.000	5.500
18	18 19 32 31	2	46.000	5.500

19	20 21 34 33	2	82.000	5.500
20	21 22 35 34	2	88.000	5.500
21	22 23 36 35	2	92.000	5.500
22	23 24 37 36	3	70.000	5.500
23	24 25 38 37	3	78.000	5.500
24	25 26 39 38	3	88.000	5.500
25	26 27 40 39	3	5.000	8.500
26	27 28 41 40	3	15.000	8.500
27	28 29 42 41	2	25.000	8.500
28	29 30 43 42	2	33.000	8.500
29	30 31 44 43	2	39.000	8.500
30	31 32 45 44	2	46.000	8.500
31	32 33 46 45	2	52.000	8.500
32	33 34 47 46	2	58.000	8.500
33	34 35 48 47	2	62.000	8.500
34	35 36 49 48	2	70.000	8.500
35	36 37 50 49	3	78.000	8.500
36	37 38 51 50	3	88.000	8.500
37	38 39 52 51	3	34.000	10.750
38	39 40 53 52	1	39.000	10.750
39	40 41 54 53	1	46.000	10.750
40	41 42 55 54	1	52.000	10.750
41	42 43 56 55	1	58.000	10.750
42	43 44 57 56	1	62.000	10.750
43	44 45 58 57	1	70.000	10.750
44	45 46 59 58	1	78.000	10.750
45	46 47 60 59	1	88.000	10.750
46	47 48 61 60	1	40.000	12.250
47	48 49 62 61	1	46.000	12.250
48	49 50 63 62	1	52.000	12.250
49	50 51 64 63	1	58.000	12.250
50	51 52 65 64	1	62.000	12.250
51	52 53 66 65	1	70.000	12.250
52	53 54 67 66	1	78.000	12.250
53	54 55 68 67	1	88.000	12.250
54	55 56 69 68	1	47.000	13.500
55	56 57 70 69	1	52.000	13.500
56	57 58 71 70	1	58.000	13.500
57	58 59 72 71	1	62.000	13.500
58	59 60 73 72	1	70.000	13.500
59	60 61 74 73	1	78.000	13.500
60	61 62 75 74	1	88.000	13.500
61	62 63 76 75	1	78.000	13.500
62	63 64 77 76	1	88.000	13.500
63	64 65 78 77	1	40.000	14.500
64	65 66 79 78	1	46.000	14.500
65	66 67 80 79	1	52.000	14.500
66	67 68 81 80	1	58.000	14.500
67	68 69 82 81	1	62.000	14.500
68	69 70 83 82	1	70.000	14.500
69	70 71 84 83	1	78.000	14.500
70	71 72 85 84	1	88.000	14.500
71	72 73 86 85	1	40.000	15.500
72	73 74 87 86	1	46.000	15.500
73	74 75 88 87	1	52.000	15.500
74	75 76 89 88	1	58.000	15.500
75	76 77 90 89	1	62.000	15.500
76	77 78 91 90	1	70.000	15.500
77	78 79 92 91	1	78.000	15.500
78	79 80 93 92	1	88.000	15.500
79	80 81 94 93	1	40.000	16.500
80	81 82 95 94	1	46.000	16.500
81	82 83 96 95	1	52.000	16.500
82	83 84 97 96	1	58.000	16.500
83	84 85 98 97	1	62.000	16.500
84	85 86 99 98	1	70.000	16.500
85	86 87 100 99	1	78.000	16.500
86	87 88 101 100	1	88.000	16.500

SETUP: 1 LAYER OF GEOTEXTILE, P=1000 PPI, T=0

```
*****  
*****  
* LOAD CASE ----- 1 *  
*****  
*****
```

LARGST ELE. NO. IN THIS INCREMENT 78
LARGST N P NO IN THIS INCREMENT 88

BAND WIDTH----- 40
TOTAL NUMBER OF EQUATIONS----- 222
NUMBER OF EQUATIONS IN BLOCK----- 88
NUMBER OF BLOCKS----- 3
NUMBER OF W.P. FORCE CARDS----- 3
NUMBER OF PRESSURE CARDS----- 0

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP	X-FORCE	Y-FORCE
1	0.	0.
2	0.	0.
3	0.	0.
4	0.	0.
5	0.	0.
6	0.	0.
7	0.	0.
8	0.	0.
9	0.	0.
10	0.	0.
11	0.	0.
12	0.	0.
13	0.	0.
14	0.	0.
15	0.	0.
16	0.	0.
17	0.	0.
18	0.	0.
19	0.	0.
20	0.	0.
21	0.	0.
22	0.	0.
23	0.	0.
24	0.	0.
25	0.	0.
26	0.	0.
27	0.	0.
28	0.	0.
29	0.	0.
30	0.	0.
31	0.	0.
32	0.	0.
33	0.	0.
34	0.	0.
35	0.	0.
36	0.	0.
37	0.	0.
38	0.	0.
39	0.	0.
40	0.	0.
41	0.	0.
42	0.	0.
43	0.	0.
44	0.	0.
45	0.	0.
46	0.	0.
47	0.	0.
48	0.	0.
49	0.	0.
50	0.	0.
51	0.	0.
52	0.	0.
53	0.	0.
54	0.	0.
55	0.	0.
56	0.	0.
57	0.	0.
58	0.	0.
59	0.	0.
60	0.	0.
61	0.	0.
62	0.	0.
63	0.	0.
64	0.	0.
65	0.	0.
66	0.	0.
67	0.	0.
68	0.	0.
69	0.	0.
70	0.	0.
71	0.	0.
72	0.	0.
73	0.	0.
74	0.	0.
75	0.	0.
76	0.	0.
77	0.	0.
78	0.	0.
79	0.	0.
80	0.	0.
81	0.	0.
82	0.	0.
83	0.	0.
84	0.	0.
85	0.	0.
86	0.	0.
87	0.	0.
88	0.	0.
89	0.	0.
90	0.	0.
91	0.	0.
92	-4.50	0.
93	-4.50	0.
94	-2.00	0.
95	0.	0.

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0000	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0002	.0000	.0000	.0002	15
16	.0001	.0001	.0000	.0005	.0005	.0000	.0007	16
17	.0004	.0002	.0000	.0021	.0021	.0000	.0028	17
18	.0217	.0102	.0000	.2388	.0588	.0000	.2473	18
19	.0408	.0027	.0000	.3884	.0841	.0000	.3940	19
20	.0378	.0089	.0000	.2860	.1378	.0000	.2950	20
21	.0230	.0183	.0000	.0822	.1843	.0000	.1833	21
22	.0098	.0001	.0000	.0007	.0082	.0000	.0083	22
23	.0013	.0018	.0000	.0008	.0081	.0000	.0082	23
24	.0008	.0088	.0000	.0018	.0113	.0000	.0114	24
25	.0024	.0018	.0000	.0028	.0083	.0000	.0088	25
26	.0000	.0008	.0000	.0000	.0022	.0000	.0022	26
27	.0000	.0000	.0000	.0000	.0001	.0000	.0001	27
28	.0000	.0000	.0000	.0000	.0003	.0000	.0003	28
29	.0002	.0000	.0000	.0020	.0008	.0000	.0020	29
30	.0317	.0070	.0000	.2823	.0730	.0000	.3013	30
31	.0538	.0224	.0000	.4278	.1187	.0000	.4439	31
32	.0788	.0088	.0000	.5118	.1111	.0000	.5228	32
33	.0838	.0108	.0000	.4272	.2780	.0000	.5102	33
34	.0880	.0223	.0000	.3488	.2804	.0000	.4483	34
35	.0818	.0293	.0000	.2428	.1881	.0000	.3078	35
36	.0014	.0043	.0000	.0001	.0144	.0000	.0144	36
37	.0018	.0130	.0000	.0028	.0212	.0000	.0213	37
38	.0028	.0033	.0000	.0028	.0100	.0000	.0103	38
39	.0000	.0028	.0000	.0000	.0038	.0000	.0038	39
40	.0000	.0000	.0000	.0000	.0001	.0000	.0001	40
41	.0000	.0000	.0000	.0002	.0002	.0000	.0004	41
42	.0278	.0123	.0000	.1892	.0688	.0000	.2008	42
43	.0813	.0178	.0000	.4128	.1120	.0000	.4278	43
44	.0942	.0287	.0000	.4334	.1288	.0000	.4623	44
45	.0888	.0138	.0000	.4220	.1888	.0000	.4801	45
46	.0887	.0111	.0000	.3888	.3884	.0000	.5327	46
47	.0882	.0287	.0000	.3174	.3883	.0000	.5918	47
48	.0848	.0363	.0000	.2888	.3177	.0000	.4988	48
49	.0834	.0883	.0000	.1887	.2433	.0000	.2844	49
50	.0028	.0328	.0000	.0080	.0804	.0000	.0808	50
51	.0002	.0021	.0000	.0008	.0108	.0000	.0108	51
52	.0000	.0038	.0000	.0000	.0082	.0000	.0082	52
53	.0002	.0387	.0000	.0480	.1883	.0000	.2047	53
54	.0808	.0138	.0000	.8888	.1424	.0000	.6124	54
55	.0908	.0111	.0000	.3788	.3711	.0000	.5310	55
56	.0818	.0281	.0000	.3208	.4081	.0000	.5180	56
57	.0808	.0377	.0000	.2488	.3483	.0000	.4288	57
58	.0822	.1083	.0000	.1110	.2880	.0000	.3087	58
59	.0081	.0388	.0000	.0388	.1048	.0000	.1111	59
60	.0031	.0013	.0000	.0043	.0080	.0000	.0100	60

61	.0000	.0037	.0000	.0000	.0048	.0000	.0048	61
62	.0888	.0138	.0000	.1880	.0087	.0000	.1882	62
63	.1888	.0817	.0000	.2384	.1481	.0000	.2778	63
64	.1888	.0870	.0000	.2344	.2804	.0000	.3508	64
65	.1381	.0008	.0000	.2107	.1888	.0000	.2802	65
66	.0183	.2873	.0000	.0882	.3808	.0000	.3880	66
67	.0148	.0448	.0000	.0343	.1388	.0000	.1431	67
68	.0077	.0004	.0000	.0082	.0088	.0000	.0114	68
69	.0000	.0028	.0000	.0000	.0038	.0000	.0038	69
70	.1282	.0181	.0000	.1808	.0288	.0000	.1828	70
71	.1431	.0018	.0000	.2088	.0881	.0000	.2211	71
72	.1427	.0878	.0000	.2088	.2033	.0000	.2882	72
73	.1338	.0170	.0000	.1887	.1848	.0000	.2886	73
74	.0801	.2887	.0000	.1033	.3714	.0000	.3888	74
75	.0372	.0481	.0000	.0804	.1804	.0000	.1820	75
76	.0084	.0012	.0000	.0078	.0084	.0000	.0102	76
77	.0000	.0027	.0000	.0000	.0023	.0000	.0023	77
78	.1848	.0088	.0000	.2018	.0848	.0000	.2088	78
79	.1848	.0888	.0000	.2328	.1488	.0000	.2787	79
80	.2288	.0118	.0000	.2888	.1062	.0000	.2883	80
81	.2802	.4791	.0000	.2887	.5340	.0000	.6174	81
82	.8327	.2808	.0000	.4847	.3488	.0000	.8084	82
83	1.1878	.0808	.0000	1.1884	.0848	.0000	1.1877	83
84	.0000	.1317	.0000	.0000	.1282	.0000	.1282	84
85	.2018	.0888	.0000	.2181	.0842	.0000	.2318	85
86	.2004	.0088	.0000	.2882	.0388	.0000	.2888	86
87	.2017	.0848	.0000	.2177	.8188	.0000	.8280	87
88	.8341	.8188	.0000	.8888	.8727	.0000	1.1148	88
89	1.8878	.0828	.0000	1.8888	.0738	.0000	1.8808	89
90	.0000	.3822	.0000	.0000	.3738	.0000	.3738	90
91	.8018	.0821	.0000	.8018	.0821	.0000	.8031	91
92	.4288	.7788	.0000	.4288	.7788	.0000	.8878	92
93	1.2408	.8420	.0000	1.2408	.8420	.0000	1.8878	93
94	1.7100	.0418	.0000	1.7100	.0418	.0000	1.7108	94
95	.0000	.8888	.0000	.0000	.8888	.0000	.8888	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS - INTERNAL MEMBER FORCES

ELEMENT NO. AXIAL FORCE

INCREMENTAL VALUES

1	.0883
2	.0030
3	.0038
4	.0043
5	.0122
6	.1222
7	.1807
8	.0108
9	.0012

GLOBAL VALUES

1	.1038
2	.0888
3	.2114
4	.3814
5	.4388
6	.8747
7	.3737

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	525.7	330.8	227.9	.156	.001	.001	.003	.302	-.001	.003	1
2	524.8	333.5	223.8	.152	.001	-.007	.011	.011	-.002	-.013	2
3	517.6	327.0	223.5	.158	.023	-.020	.057	.037	-.034	.071	3
4	.7	17.2	.2	.484	1.887	-.775	2.583	2.470	-1.288	3.759	4
5	.8	17.0	.2	.484	.987	-.053	8.511	4.801	-3.781	8.562	5
6	.7	17.9	.2	.484	-.760	2.828	7.740	5.083	-3.397	8.440	6
7	.7	18.4	.2	.484	-1.847	3.703	4.208	4.432	-2.378	8.808	7
8	.8	18.8	.2	.484	-1.144	2.048	-.752	2.088	-1.188	3.278	8
9	488.0	338.0	188.2	.228	-.001	.130	.017	.130	-.001	.131	9
10	488.1	338.1	188.1	.228	-.013	-.208	.028	.208	-.014	.220	10
11	478.8	370.7	181.8	.242	-.007	.207	-.088	.218	-.018	.232	11
12	511.4	363.8	211.6	.208	.018	.094	-.052	.102	.008	.093	12
13	437.4	257.8	184.9	.122	.001	.003	-.001	.003	.001	.002	13
14	488.0	272.8	208.4	.176	.011	-.011	.021	.018	-.018	.031	14
15	2.2	15.1	.8	.473	1.478	-1.188	4.434	2.727	-2.480	5.177	15
16	2.5	16.4	.8	.473	3.083	-2.180	7.041	4.847	-3.844	8.781	16
17	1.2	16.7	.4	.488	1.880	-.214	8.840	5.181	-3.888	8.847	17
18	.8	17.0	.2	.484	-1.388	3.138	7.488	5.238	-3.472	8.710	18
19	.8	17.0	.2	.484	-2.814	4.388	7.982	5.182	-4.411	10.893	19
20	.7	18.1	.2	.484	-2.484	5.088	5.288	5.823	-3.308	9.232	20
21	.8	17.1	.2	.484	-1.518	3.218	2.930	3.834	-1.932	5.887	21
22	388.3	330.7	188.4	.281	-.028	.318	.088	.320	-.031	.382	22
23	408.2	308.3	184.7	.230	-.008	.244	-.123	.288	-.022	.280	23
24	487.8	310.0	182.2	.190	.032	-.100	-.088	.111	.021	-.089	24
25	408.8	228.8	188.8	.093	-.001	.000	.002	.001	-.001	.002	25
26	.0	.1	.0	.488	.887	-1.101	2.778	1.887	-1.801	3.488	26
27	1.2	13.8	.4	.488	2.889	-1.780	4.838	3.541	-2.722	6.282	27
28	1.8	13.8	.8	.480	1.300	-.812	1.888	1.888	-1.071	2.831	28
29	1.3	14.1	.4	.488	.808	-.888	2.910	2.087	-.883	2.910	29
30	.8	18.0	.2	.484	-.882	2.888	-.031	2.888	-.882	3.248	30
31	.8	18.7	.2	.484	-1.870	3.804	-1.828	2.718	-1.881	8.387	31
32	.6	18.4	.2	.484	-2.071	3.842	-2.280	4.182	-2.282	8.438	32
33	.7	17.7	.2	.484	-2.474	5.843	.488	5.870	-2.480	8.430	33
34	.7	17.4	.2	.484	-.838	4.287	.748	4.718	-.837	4.882	34
35	282.9	288.8	114.7	.277	-.033	.801	-.118	.808	-.038	.847	35
36	348.0	217.2	148.7	.188	.010	-.088	-.012	.080	.010	.080	36
37	.1	4.0	.0	.488	.378	.212	17.083	8.828	-8.248	17.084	37
38	183.0	847.4	888.3	.078	.888	2.088	20.241	10.848	-8.602	20.481	38
39	808.8	814.8	348.8	.181	-1.884	4.083	10.408	7.127	-4.788	11.888	39
40	348.1	441.2	127.7	.388	-1.328	4.880	2.273	5.187	-1.843	6.710	40
41	407.8	448.7	183.3	.330	-1.888	5.381	-.820	5.391	-1.894	6.888	41
42	11.8	388.8	4.0	.488	-1.770	4.818	-.188	4.818	-1.771	6.380	42
43	11.8	388.8	4.0	.488	-1.812	4.027	-3.302	4.482	-1.888	6.448	43
44	708.8	830.0	277.3	.280	.224	1.828	-2.138	2.442	-.282	2.733	44
45	1087.2	888.4	488.3	.144	-.032	.048	.088	.088	-.042	.087	45
46	278.2	432.3	160.8	.343	.018	-.082	-.238	.188	-.184	.382	46
47	.1	4.0	.0	.488	.380	-.384	2.737	1.437	-1.400	2.837	47
48	.1	4.0	.0	.488	.088	1.420	5.080	3.388	-1.870	5.228	48
49	.1	4.0	.0	.488	-.724	1.284	2.887	1.480	-1.370	3.288	49
50	.1	4.0	.0	.488	-1.888	6.707	-2.378	8.801	-1.878	7.478	50
51	.1	4.0	.0	.488	-.888	9.124	-2.488	9.278	-.744	10.022	51
52	828.8	848.0	248.0	.274	-.228	2.888	-.328	2.877	-.281	3.128	52
53	888.3	882.3	381.2	.178	-.088	.083	.122	.078	-.108	.188	53
54	.1	4.0	.0	.488	-.084	-.024	.048	.048	-.048	.147	54
55	178.8	418.7	83.1	.428	-.038	-.147	.802	.488	-.387	.823	55
56	.1	4.0	.0	.488	-.442	1.008	-2.187	1.887	-1.020	2.807	56
57	11.8	388.8	4.0	.488	-1.480	1.220	5.182	2.778	-3.048	5.821	57
58	1787.2	887.3	843.4	.088	-.417	2.888	1.144	3.081	-.812	3.882	58
59	888.8	888.8	388.3	.204	-.448	2.789	.103	2.770	-.447	3.218	59
60	821.3	488.4	202.3	.282	-.108	.108	-.088	.117	-.118	.233	60
61	.1	4.0	.0	.488	.372	-.388	2.702	1.387	-1.408	2.807	61

62	.1	4.0	.0	.488	.362	-.243	8.828	2.838	-2.720	8.880	82
63	11.8	388.8	4.0	.488	-.288	-.284	8.088	13.043	-3.088	8.111	83
64	.1	4.0	.0	.488	-.288	8.283	18.830	18.482	-8.888	22.087	84
65	.1	4.0	.0	.488	-8.184	22.880	-18.817	28.884	-8.038	33.803	85
66	.1	4.0	.0	.488	-4.487	18.873	-88.180	81.008	-38.480	80.487	86
67	.1	4.0	.0	.488	7.242	-3.818	-88.878	32.118	-28.781	80.808	87
68	.1	4.0	.0	.488	-.788	-.708	2.487	1.488	-1.418	2.888	88
69	11.8	388.8	4.0	.488	-.834	-.872	-1.247	1.888	-.838	1.738	89
70	11.8	388.8	4.0	.488	-.088	-.038	-2.847	1.388	-1.282	2.848	90
71	.1	4.0	.0	.488	-11.788	18.283	-22.332	22.881	-18.388	38.288	91
72	.1	4.0	.0	.488	-8.848	20.841	-48.878	34.078	-22.808	88.878	92
73	.1	4.0	.0	.488	17.888	-11.183	-28.830	23.830	-17.020	40.880	93
74	.1	4.0	.0	.488	4.884	-8.884	88.838	30.824	-38.324	87.188	94
75	.1	4.0	.0	.488	-3.081	11.780	83.428	32.080	-23.324	88.482	95
76	.1	4.0	.0	.488	-17.814	30.882	-2.274	30.888	-17.881	48.430	96
77	.1	4.0	.0	.488	-7.817	18.887	-28.821	21.843	-13.883	38.238	97
78	.1	4.0	.0	.488	21.111	-18.787	-8.808	21.713	-18.388	38.103	98

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THEYA	STG1/SIG3	LEVEL	ELE
1	.248	484	.008	.484	.248	.118	1.484	1.887	.078	1
2	.288	824	.028	.828	.288	.138	5.387	2.078	.088	2
3	.384	.380	.134	.807	.237	.138	41.188	2.144	.080	3
4	.377	.848	.008	.848	.377	.088	1.774	1.448	1.880	4
5	.348	.828	.017	.827	.347	.080	8.810	1.818	1.782	5
6	.482	.848	.084	.847	.481	.084	4.881	1.437	1.887	6
7	.818	.718	.028	.718	.817	.102	2.881	1.387	2.040	7
8	.817	.808	-.001	.808	.817	-.088	-.424	1.810	1.928	8
9	.232	1.022	.028	1.023	.232	.348	-2.130	3.082	.207	9
10	.401	1.437	.081	1.440	.398	.820	2.828	3.811	.281	10
11	.444	1.488	-.072	1.487	.443	-.821	-8.282	3.803	.287	11
12	.378	.878	-.108	.888	.388	-.271	-11.888	2.821	.188	12
13	.141	.243	-.002	.283	-.141	-.071	-.648	2.002	-.083	13
14	.181	.224	.042	.280	.188	.047	31.788	1.812	.038	14
15	.140	-.208	.020	.210	-.134	-.038	18.823	1.871	-.788	15
16	.288	-.278	.042	.310	-.228	-.048	38.884	1.378	-.882	16
17	.318	-.388	.023	.402	.212	-.048	18.443	1.288	-.883	17
18	.382	.488	.018	.488	.380	.088	7.887	1.318	1.108	18
19	.347	.484	.078	.488	.348	.080	7.774	1.380	1.208	19
20	.484	.818	.012	.817	.483	.082	8.818	1.282	1.241	20
21	.340	.448	.008	.480	.338	.088	3.188	1.328	1.104	21
22	.388	1.838	.084	1.841	.381	.840	3.788	4.847	.372	22
23	.323	1.287	-.204	1.308	.281	-.814	-11.878	4.881	.328	23
24	.318	.820	-.113	.888	.281	-.188	-18.380	2.340	.118	24
25	.081	-.081	-.003	.081	-.081	-.028	3.442	2.210	.022	25
26	.081	-.081	-.003	.081	-.081	-.028	88.878	2.218	-.000	26
27	.887	.818	.812	.888	.088	.088	88.483	12.218	.887	27
28	.111	.041	.028	.121	.030	.048	70.228	3.878	.808	28
29	.131	-.212	-.014	.214	-.128	-.043	8.711	1.870	-.880	29
30	.187	.281	-.003	.281	.188	.082	-1.828	1.861	1.046	30
31	.240	.347	-.008	.348	-.240	-.084	-2.814	1.448	1.078	31
32	.212	.321	-.012	.323	-.211	-.088	-8.341	1.823	1.123	32
33	.481	.802	.002	.802	.481	.080	1.043	1.281	1.207	33
34	.828	.828	.008	.828	.828	.081	2.887	1.183	1.011	34
35	.418	1.818	-.130	1.828	.418	-.887	-8.382	4.218	.377	35
36	.088	-.248	-.018	.248	-.084	-.083	-4.888	3.900	-.078	36
37	.138	-.082	-.088	.218	-.014	-.102	-81.317	18.788	8.488	37
38	.188	.108	.027	.170	.088	.038	87.812	1.787	.288	38
39	.120	.284	.031	.281	-.114	-.074	12.421	2.301	.483	39
40	.087	.231	.138	.310	-.012	-.181	28.888	-24.844	-1.000	40
41	.083	.770	-.112	.788	-.078	-.388	-8.188	10.818	3.837	41
42	-.112	.783	.004	.383	-.112	.247	-.483	-3.434	-1.000	42
43	-.288	.370	-.118	.381	-.280	.230	-10.101	-1.328	-1.000	43
44	.287	1.838	-.203	1.882	.242	-.880	-8.813	8.082	2.838	44
45	-.141	-.180	.244	.312	-.273	.282	28.337	-1.143	-1.000	45
46	.040	.048	.021	.084	.022	.021	41.018	2.488	.702	46
47	.088	-.131	.113	.213	-.028	-.118	38.087	-8.808	-1.000	47
48	.088	.288	.084	.302	.071	.118	14.070	4.233	1.202	48
49	.038	.314	-.112	.383	-.004	-.178	-18.441	-88.182	-1.000	49
50	.213	.378	-.082	.380	.188	.088	-18.438	1.872	.381	50
51	.273	.880	-.100	.810	-.243	-.183	-18.884	2.808	.881	51
52	.038	1.488	-.287	1.843	-.080	-.812	-8.818	-18.248	-1.000	52
53	.888	.148	.427	.348	-.781	.883	24.888	-4.441	-1.000	53
54	.022	.081	.124	.171	-.078	.128	41.832	-2.200	-1.000	54
55	-.020	.241	-.028	.244	-.027	-.138	8.880	-7.421	-1.000	55
56	-.044	.288	.002	.288	-.044	.188	40.1	-8.488	-1.000	56
57	-1.241	-.887	-.123	-.821	-1.277	.228	18.287	-.843	-1.000	57
58	-1.838	2.487	-1.808	3.288	-2.288	2.777	-21.717	-1.418	-1.000	58
59	-2.180	2.008	1.877	1.237	-3.211	2.224	28.778	-.388	-1.000	59
60	-.388	.384	-.207	.438	-.412	.428	-14.884	-1.082	-1.000	60
61	.018	.084	.003	.028	.008	.088	12.782	2.788	.884	61
62	.182	-.128	.134	.278	-.008	-.134	47.378	48.881	18.884	62
63	.082	-.123	.421	.384	-.488	.430	80.884	-.847	-1.000	63
64	.408	.882	-.133	.828	.333	.183	-30.078	1.818	.322	64
65	.718	.788	-.008	.787	.718	.021	-10.488	1.088	.022	65
66	.488	.801	-.038	.823	.448	-.038	-32.328	1.178	.088	66
67	.282	.334	-.023	.340	.248	.047	-14.822	1.388	.143	67
68	-.011	.038	.028	.047	-.028	.088	24.102	-2.032	-1.000	68
69	.318	.238	.847	.340	.218	.081	88.282	1.882	.208	69
70	.474	.878	.087	.808	.448	.077	23.812	1.343	.128	70
71	.302	.344	-.088	.348	.300	.023	-11.848	1.181	.088	71
72	.488	.488	-.020	.488	.440	.022	-22.113	1.127	.047	72
73	-.288	.282	-.011	-.287	-.273	.012	-82.388	1.088	.032	73
74	-.208	-.208	.028	-.178	-.238	-.028	43.318	-.788	-1.000	74
75	.387	.388	-.021	.387	.348	.028	27.848	1.188	.088	75
76	.830	.888	-.001	.888	.830	.028	-.834	1.108	.038	76
77	.328	.384	-.011	.387	.328	.021	18.471	1.127	.047	77
78	.244	.232	-.004	.248	.230	.007	-74.301	1.043	.023	78

SSTIPN: 1 LAYER OF GEOTEXTILE, P=1000 PPI, T=1000

TOTAL NUMBER OF NODES-----	88
NUMBER OF BAR ELEMENTS-----	9
NUMBER OF DIFF. BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF. BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO. OF INTERFACE ELE. IN PREEXIST PART	0
NUMBER OF INTERFACE ELE. IN FOUNDATION	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF. SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	38
NUMBER OF NODES IN FOUNDATION-----	82
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	4
NUMBER OF LOAD CASES-----	2

CALING FACTOR ----- 1.00000

ATMOSPHERIC PRESSURE --- 1.05800

UNIT WEIGHT OF WATER --- 0.8120

OUTPUT SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO. 1	APPLY LOAD CASE	1
INCREMENT NO. 2	PUT ON LAYER NO.	1
INCREMENT NO. 3	PUT ON LAYER NO.	2
INCREMENT NO. 4	PUT ON LAYER NO.	3
INCREMENT NO. 5	PUT ON LAYER NO.	4
INCREMENT NO. 6	PUT ON LAYER NO.	5
INCREMENT NO. 7	PUT ON LAYER NO.	6
INCREMENT NO. 8	APPLY LOAD CASE	2

MODAL POINT INPUT DATA

NODE NUMBER	MODAL POINT COORDINATES		B.C. CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	0.000	0.000	1	1	1
2	10.000	0.000	1	1	1
3	20.000	0.000	1	1	1
4	30.000	0.000	1	1	1
5	38.000	0.000	1	1	1
6	42.000	0.000	1	1	1
7	50.000	0.000	1	1	1
8	54.000	0.000	1	1	1
9	58.000	0.000	1	1	1
10	68.000	0.000	1	1	1
11	74.000	0.000	1	1	1
12	82.000	0.000	1	1	1
13	90.000	0.000	1	1	1
14	0.000	4.000	0	0	0
15	10.000	4.000	0	0	0
16	20.000	4.000	0	0	0
17	30.000	4.000	0	0	0
18	38.000	4.000	0	0	0
19	42.000	4.000	0	0	0
20	50.000	4.000	0	0	0
21	54.000	4.000	0	0	0
22	58.000	4.000	0	0	0
23	68.000	4.000	0	0	0
24	74.000	4.000	0	0	0
25	82.000	4.000	0	0	0
26	90.000	4.000	0	0	0
27	0.000	7.000	1	0	1
28	10.000	7.000	0	0	0
29	20.000	7.000	0	0	0
30	30.000	7.000	0	0	0
31	38.000	7.000	0	0	0
32	42.000	7.000	0	0	0
33	50.000	7.000	0	0	0
34	54.000	7.000	0	0	0
35	58.000	7.000	0	0	0
36	68.000	7.000	0	0	0
37	74.000	7.000	0	0	0
38	82.000	7.000	0	0	0
39	90.000	7.000	1	0	1
40	0.000	10.000	1	0	1
41	10.000	10.000	0	0	0
42	20.000	10.000	0	0	0
43	30.000	10.000	0	0	0
44	38.000	10.000	0	0	0
45	42.000	10.000	0	0	0
46	50.000	10.000	0	0	0
47	54.000	10.000	0	0	0
48	58.000	10.000	0	0	0
49	68.000	10.000	0	0	0
50	74.000	10.000	0	0	0
51	82.000	10.000	0	0	0
52	90.000	10.000	1	0	1
53	0.000	11.500	0	0	0
54	10.000	11.500	0	0	0
55	20.000	11.500	0	0	0
56	30.000	11.500	0	0	0
57	38.000	11.500	0	0	0
58	42.000	11.500	0	0	0
59	50.000	11.500	0	0	0
60	54.000	11.500	0	0	0
61	58.000	11.500	1	0	1
62	42.000	13.000	0	0	0
63	50.000	13.000	0	0	0
64	54.000	13.000	0	0	0
65	58.000	13.000	0	0	0
66	68.000	13.000	0	0	0
67	74.000	13.000	0	0	0
68	82.000	13.000	1	0	1
69	90.000	13.000	0	0	0
70	48.000	14.000	0	0	0
71	50.000	14.000	0	0	0
72	58.000	14.000	0	0	0
73	68.000	14.000	0	0	0
74	74.000	14.000	0	0	0
75	82.000	14.000	0	0	0
76	90.000	14.000	1	0	1
77	50.000	15.000	0	0	0
78	60.000	15.000	0	0	0
79	64.000	15.000	0	0	0
80	68.000	15.000	0	0	0
81	74.000	15.000	0	0	0
82	82.000	15.000	0	0	0
83	90.000	15.000	1	0	1
84	54.000	16.000	0	0	0
85	58.000	16.000	0	0	0
86	68.000	16.000	0	0	0
87	74.000	16.000	0	0	0
88	82.000	16.000	0	0	0
89	90.000	16.000	1	0	1
90	58.000	17.000	0	0	0
91	68.000	17.000	0	0	0
92	74.000	17.000	0	0	0
93	82.000	17.000	0	0	0
94	90.000	17.000	1	0	1

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	30.	1.00	0.

ELMT NO.	CONNECTED NODES I	J	MATL NO.
1	43	44	1
2	44	45	1
3	45	46	1
4	46	47	1
5	47	48	1
6	48	49	1
7	49	50	1
8	50	51	1
9	51	52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S MODULUS		RATIO	POISSON'S MODULUS		STRENGTH C	PARAMETERS		KO
		CONSTANT	EXPONENT		CONSTANT	EXPONENT		PMI	DPHI	
1	.0830	800.00	.300	.500	1500.00	.200	.00	35.00	.00	.50
2	.0830	400.00	.300	.500	1000.00	.200	.00	40.00	.00	.50
3	.0800	1000.00	.400	.700	500.00	.500	.50	40.00	.00	.50

FOUR NODES SOLID ELEMENT DATA

ELMT NO.	CONNECTED NODES I	J	K	L	MATL NO.	ELEMENT CENTER X-ORD	Y-ORD
1	1	2	15	14	3	5.000	2.000
2	2	3	15	15	3	15.000	2.000
3	3	4	17	15	3	25.000	2.000
4	4	5	18	17	2	33.000	2.000
5	5	6	19	18	2	39.000	2.000
6	6	7	20	19	2	46.000	2.000
7	7	8	21	20	2	52.000	2.000
8	8	9	22	21	2	58.000	2.000
9	9	10	23	22	3	62.000	2.000
10	10	11	24	23	3	70.000	2.000
11	11	12	25	24	3	78.000	2.000
12	12	13	26	25	3	86.000	2.000
13	14	15	26	27	3	5.000	5.500
14	15	16	28	28	3	15.000	5.500
15	16	17	30	29	2	25.000	5.500
16	17	18	31	30	2	32.000	5.500
17	18	19	32	31	2	39.000	5.500
18	19	20	33	32	2	46.000	5.500
19	20	21	34	33	2	52.000	5.500
20	21	22	35	34	2	58.000	5.500
21	22	23	36	35	2	62.000	5.500
22	23	24	37	36	3	70.000	5.500
23	24	25	38	37	3	78.000	5.500
24	25	26	39	38	3	86.000	5.500
25	27	28	41	40	3	5.000	8.500
26	28	29	42	41	2	15.000	8.500
27	29	30	43	42	2	25.000	8.500
28	30	31	44	43	2	32.000	8.500
29	31	32	45	44	2	39.000	8.500
30	32	33	46	45	2	46.000	8.500
31	32	34	47	46	2	52.000	8.500
32	34	35	48	47	2	58.000	8.500
33	35	36	49	48	2	62.000	8.500
34	36	37	50	49	2	70.000	8.500
35	37	38	51	50	3	78.000	8.500
36	38	39	52	51	3	86.000	8.500
37	43	44	53	53	1	24.000	10.750
38	44	45	54	53	1	38.000	10.750
39	45	46	55	54	1	46.000	10.750
40	46	47	56	55	1	52.000	10.750
41	47	48	57	56	1	58.000	10.750
42	48	49	58	57	1	62.000	10.750
43	49	50	59	58	1	70.000	10.750
44	50	51	60	59	1	78.000	10.750
45	51	52	61	60	1	86.000	10.750
46	53	54	62	62	1	40.000	12.250
47	54	55	63	62	1	48.000	12.250
48	55	56	64	63	1	52.000	12.250
49	56	57	65	64	1	58.000	12.250
50	57	58	66	65	1	62.000	12.250
51	58	59	67	66	1	70.000	12.250
52	59	60	68	67	1	78.000	12.250
53	60	61	69	68	1	86.000	12.250
54	62	63	71	70	1	47.000	13.500
55	63	64	72	71	1	52.000	13.500
56	64	65	73	72	1	58.000	13.500
57	65	66	74	73	1	62.000	13.500
58	66	67	75	74	1	70.000	13.500
59	67	68	76	75	1	78.000	13.500
60	68	69	77	76	1	86.000	13.500
61	70	71	78	78	1	48.000	14.500
62	71	72	79	78	1	52.000	14.500
63	72	73	80	79	1	58.000	14.500
64	73	74	81	80	1	62.000	14.500
65	74	75	82	81	1	70.000	14.500
66	75	76	83	82	1	78.000	14.500
67	76	77	84	83	1	86.000	14.500
68	78	79	85	85	1	52.000	15.500
69	79	80	86	85	1	58.000	15.500
70	80	81	87	86	1	62.000	15.500
71	81	82	88	87	1	70.000	15.500
72	82	83	89	88	1	78.000	15.500
73	83	84	90	89	1	86.000	15.500
74	85	86	91	91	1	57.000	16.500
75	86	87	92	91	1	62.000	16.500
76	87	88	93	92	1	70.000	16.500
77	88	89	94	93	1	78.000	16.500
78	89	90	95	94	1	86.000	16.500

SS TIPN: 1 LAYER OF GEOTEXTILE, P=1000 PPI, T=1000

```
.....  
.....  
* LOAD CASE ----- 2 *  
.....  
.....
```

LARGEST ELE. NO. IN THIS INCREMENT 78
LARGEST N P NO IN THIS INCREMENT 88

BAND WIDTH----- 40
TOTAL NUMBER OF EQUATIONS----- 222
NUMBER OF EQUATIONS IN BLOCK----- 64
NUMBER OF BLOCKS----- 3
NUMBER OF W.P. PUNCH CARDS----- 3
NUMBER OF PRESSURE CARDS----- 0

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP X-FORCE Y-FORCE

1	0.	0.
2	0.	0.
3	0.	0.
4	0.	0.
5	0.	0.
6	0.	0.
7	0.	0.
8	0.	0.
9	0.	0.
10	0.	0.
11	0.	0.
12	0.	0.
13	0.	0.
14	0.	0.
15	0.	0.
16	0.	0.
17	0.	0.
18	0.	0.
19	0.	0.
20	0.	0.
21	0.	0.
22	0.	0.
23	0.	0.
24	0.	0.
25	0.	0.
26	0.	0.
27	0.	0.
28	0.	0.
29	0.	0.
30	0.	0.
31	0.	0.
32	0.	0.
33	0.	0.
34	0.	0.
35	0.	0.
36	0.	0.
37	0.	0.
38	0.	0.
39	0.	0.
40	0.	0.
41	0.	0.
42	0.	0.
43	0.	0.
44	0.	0.
45	0.	0.
46	0.	0.
47	0.	0.
48	0.	0.
49	0.	0.
50	0.	0.
51	0.	0.
52	0.	0.
53	0.	0.
54	0.	0.
55	0.	0.
56	0.	0.
57	0.	0.
58	0.	0.
59	0.	0.
60	0.	0.
61	0.	0.
62	0.	0.
63	0.	0.
64	0.	0.
65	0.	0.
66	0.	0.
67	0.	0.
68	0.	0.
69	0.	0.
70	0.	0.
71	0.	0.
72	0.	0.
73	0.	0.
74	0.	0.
75	0.	0.
76	0.	0.
77	0.	0.
78	0.	0.
79	0.	0.
80	0.	0.
81	0.	0.
82	0.	0.
83	0.	0.
84	0.	0.
85	0.	0.
86	0.	0.
87	0.	0.
88	0.	0.
89	0.	0.
90	0.	0.
91	0.	0.
92	0.	0.
93	0.	-4.80
94	0.	-2.00
95	0.	0.

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	530.2	333.5	228.5	158	.002	.001	.005	.004	-.001	.005	1
2	525.1	338.9	228.8	164	.002	.010	.017	.018	-.003	.019	2
3	488.8	308.9	210.7	160	.025	-.025	.085	.041	-.041	.082	3
4	.7	17.3	.2	484	2.033	-.783	2.732	2.589	-1.319	3.908	4
5	.8	17.0	.2	484	.820	.084	8.740	4.825	-3.881	8.788	5
6	.7	17.3	.2	484	-.489	2.833	7.301	8.133	-3.488	8.402	6
7	.7	18.4	.2	483	-1.882	3.885	4.288	4.481	-2.417	8.889	7
8	.8	18.7	.2	484	-1.180	2.041	-.704	2.079	-1.218	3.287	8
9	448.2	334.8	182.8	227	-.001	133	020	133	-.002	138	9
10	488.2	380.8	188.8	237	-.013	254	-.028	.208	-.014	.219	10
11	476.8	370.8	181.8	241	-.007	.205	-.083	.213	-.018	.228	11
12	510.5	383.0	211.1	208	.018	.085	-.048	.102	.008	.084	12
13	484.3	282.2	187.8	123	.002	.007	-.001	.003	.002	.001	13
14	484.7	287.8	202.7	123	.017	-.214	-.032	.044	-.018	.044	14
15	2.0	14.2	.7	478	1.818	-1.582	5.801	3.555	-3.218	8.774	15
16	1.7	16.5	.6	481	3.720	-2.418	10.483	6.418	-5.513	11.822	16
17	.7	18.4	.2	483	1.283	.089	11.160	8.282	-4.811	11.203	17
18	.8	17.0	.2	484	-1.882	3.388	4.874	5.875	-4.111	8.888	18
19	.8	17.0	.2	484	-2.700	4.488	8.878	8.801	-4.748	11.267	19
20	.7	18.2	.2	484	-2.808	5.214	5.987	6.222	-3.813	9.438	20
21	.8	17.1	.2	484	-1.820	3.323	3.177	3.780	-2.087	5.878	21
22	404.8	332.4	180.3	286	-.023	.318	.048	.321	-.031	.382	22
23	404.8	304.8	164.4	230	-.008	.240	-.113	.283	-.021	.274	23
24	482.8	307.8	180.8	180	.030	-.103	-.087	.113	.020	.083	24
25	408.8	228.8	187.0	083	-.001	.000	.002	.001	-.001	.002	25
26	.0	.1	.0	488	1.018	-1.388	2.302	1.828	-2.204	3.432	26
27	.4	13.4	.1	484	3.518	-2.887	4.827	4.328	-3.878	8.002	27
28	1.8	13.6	.5	482	1.302	-1.052	2.788	1.948	-1.888	3.867	28
29	.8	18.1	.2	408	.083	1.180	2.885	2.181	-.808	3.070	29
30	.8	18.8	.2	484	-1.177	2.887	-.808	2.840	-1.188	4.078	30
31	.8	18.8	.2	484	-1.774	3.738	-.873	3.758	-1.794	5.883	31
32	.8	18.8	.2	484	-2.217	4.078	-1.228	4.138	-2.278	6.411	32
33	.7	17.7	.2	484	-2.888	8.117	1.580	8.188	-2.887	8.883	33
34	.7	17.4	.2	484	-.781	4.410	1.778	4.388	-.800	5.488	34
35	288.8	283.4	113.1	.277	-.110	.870	.488	.840	-.180	.871	35
36	348.8	218.8	148.8	187	-.080	-.131	.187	.183	-.081	.284	36
37	11.8	388.8	4.0	888	2.18	-.188	17.388	8.888	-8.483	17.388	37
38	484.7	428.7	381.7	137	-.028	2.068	18.488	10.688	-8.448	20.084	38
39	432.1	483.2	183.8	218	-1.848	2.883	10.380	7.088	-4.738	11.822	39
40	223.2	428.1	78.4	407	-1.838	5.378	2.288	5.581	-1.721	7.282	40
41	348.1	441.8	128.8	384	-1.547	5.888	-1.028	5.800	-1.583	7.183	41
42	204.1	422.7	72.1	.418	-1.888	4.888	-.841	4.833	-1.881	6.284	42
43	11.8	388.8	4.0	483	-1.612	3.888	-3.772	4.843	-2.100	6.843	43
44	878.7	388.8	288.8	278	-.218	1.842	-2.183	2.483	-.302	2.788	44
45	1077.1	887.8	471.8	141	-.033	.048	.055	.055	-.041	.088	45
46	288.8	420.8	88.8	.384	-.084	.884	.083	.074	-.088	.138	46
47	.1	4.0	.0	488	-.010	-.182	.714	.437	-.288	.732	47
48	88.8	408.4	29.3	484	-.214	-.188	.811	.408	-.478	.887	48
49	.1	4.0	.0	488	.884	-.043	.700	.122	-.742	.874	49
50	.1	4.0	.0	488	-1.088	5.384	1.388	6.447	-1.180	7.687	50
51	.1	4.0	.0	488	-.878	8.317	-2.238	9.441	-.700	10.141	51
52	838.8	584.8	248.8	278	-.282	2.877	-.320	2.888	-.280	3.148	52
53	880.3	875.7	388.0	188	-.088	.057	.124	.080	-.110	.180	53
54	11.8	388.8	4.0	488	-.137	-.710	.842	.472	-.434	.818	54
55	11.8	388.8	4.0	488	-.072	.084	1.488	.788	-.738	1.484	55
56	11.8	388.8	4.0	488	-.843	.848	-1.118	.581	-.848	1.087	56
57	1.8	388.8	4.0	488	-.833	.708	4.888	2.388	-2.482	4.841	57
58	1482.8	848.2	861.7	.114	-.481	1.001	1.282	3.112	-.573	3.841	58
59	878.1	813.1	388.2	202	-.402	2.808	-.088	2.807	-.482	3.288	59
60	408.8	484.1	188.3	288	-.104	.104	-.088	.113	-.117	.220	60
61	.1	.0	.0	488	.287	-.018	-.038	.288	-.021	.278	61

82	279.8	432.4	101.2	.383	.141	-.072	.327	.230	-.181	.390	82
83	.1	4.0	.0	.485	5.847	-4.878	24.855	13.812	-13.141	28.882	83
84	.1	4.0	.0	.485	-1.180	8.318	57.816	32.208	-28.079	58.260	84
85	.1	4.0	.0	.485	-8.828	24.484	7.832	24.053	-8.383	31.338	85
86	.1	4.0	.0	.485	-4.880	16.182	-48.244	41.813	-30.411	72.328	86
87	.1	4.0	.0	.485	8.188	-3.010	-81.417	27.704	-24.828	82.233	87
88	11.8	388.8	4.0	.485	-2.201	-.288	4.488	2.384	-2.488	4.888	88
89	.1	4.0	.0	.485	10.421	-.303	-3.881	10.728	-.810	11.338	89
90	88.8	410.0	34.1	.485	-.013	.114	.030	.118	-.018	.131	90
91	.1	4.0	.0	.485	-10.824	19.288	-3.891	19.401	-10.737	30.138	91
92	.1	4.0	.0	.485	-11.718	21.884	-22.788	24.888	-14.824	38.488	92
93	.1	4.0	.0	.485	14.781	-8.272	-22.042	18.178	-12.887	31.873	93
94	.1	4.0	.0	.485	8.878	-.288	-8.842	11.087	-2.480	13.808	94
95	138.8	414.3	47.1	.443	-.031	.118	.047	.120	-.034	.184	95
96	.1	4.0	.0	.485	-10.182	18.838	-2.803	18.882	-10.218	24.808	96
97	.1	4.0	.0	.485	-12.788	20.228	-10.883	21.100	-13.883	34.783	97
98	.1	4.0	.0	.485	17.148	-11.880	-7.883	17.837	-12.108	28.808	98

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-KY	SIG-1	SIG-3	TAU-MAX	THRYA	SIG1/SIG3	LEVEL	ELE
1	.281	.488	.011	.488	.280	.118	2.877	1.882	-.078	1
2	.282	.538	.038	.548	.288	.144	7.832	2.114	.083	2
3	.388	.584	.144	.608	.211	.148	24.723	2.408	-.102	3
4	.383	.681	.008	.681	.383	.084	1.888	1.430	1.888	4
5	.383	.828	.018	.831	.381	.080	8.887	1.811	1.788	5
6	.448	.842	.017	.844	.447	.088	4.884	1.441	1.888	6
7	.810	.713	.010	.714	.810	.102	2.708	1.400	2.030	7
8	.310	.303	-.001	.803	.310	.087	-.384	1.623	1.830	8
9	.332	1.038	.031	1.038	.331	.353	2.817	2.130	.211	9
10	.402	1.432	.083	1.438	.388	.518	2.837	3.887	.288	10
11	.440	.488	-.162	1.481	.414	.832	-8.837	3.874	.283	11
12	.378	.881	-.104	.882	.388	.273	-11.171	2.841	-.180	12
13	.148	.288	-.002	.288	.148	.070	-.722	1.888	-.082	13
14	.208	.222	.088	.228	.148	.088	41.888	1.874	.048	14
15	.184	.207	.030	.221	.141	.650	24.414	1.871	-.853	15
16	.248	.284	.043	.313	.218	.047	33.808	1.428	-.832	16
17	.288	.388	.022	.380	.284	.048	13.832	1.328	-.888	17
18	.380	.488	.018	.481	.347	.087	8.873	1.328	1.138	18
19	.344	.487	.017	.482	.341	.081	8.321	1.384	1.222	19
20	.483	.818	.014	.818	.481	.083	8.323	1.288	1.283	20
21	.338	.880	.007	.880	.338	.088	3.423	1.328	1.112	21
22	.388	1.841	.087	1.847	.380	.843	3.888	4.878	-.374	22
23	.314	1.288	-.188	1.288	.277	.804	-10.884	4.832	-.321	23
24	.310	.832	-.108	.888	.278	.184	-17.088	2.408	-.124	24
25	.042	.081	.003	.082	.042	.028	4.008	2.182	.022	25
26	.008	-.004	.007	.012	-.007	.010	88.848	-1.882	-1.000	26
27	.188	.024	.048	.178	.024	.082	73.484	12.882	1.842	27
28	.108	.024	.012	.108	.022	.043	82.082	4.808	.884	28
29	.188	.187	.033	.208	.137	.038	33.781	1.818	.713	29
30	.178	.302	-.003	.302	.178	.083	-1.473	1.722	1.287	30
31	.228	.338	-.004	.338	.228	.084	-2.088	1.478	1.087	31
32	.208	.322	-.011	.323	.204	.088	-8.828	1.882	1.180	32
33	.488	.811	.002	.811	.488	.082	3.808	1.284	1.237	33
34	.828	.827	.008	.827	.828	.081	3.180	1.182	1.017	34
35	.414	1.788	-.142	1.788	.414	.831	-4.321	4.423	-.887	35
36	.088	.288	-.024	.288	.082	.104	-6.728	4.232	.087	36
37	.080	.087	-.083	.128	.018	.084	-37.888	8.720	2.128	37
38	.084	.102	.021	.118	.078	.021	38.283	1.861	.208	38
39	.083	.281	-.071	.304	.081	.121	17.777	4.888	1.488	39
40	-.018	.270	-.124	.318	-.082	.188	-20.448	-8.048	-1.000	40
41	-.188	.328	.028	.328	-.188	.242	3.302	-2.088	-1.000	41
42	-.484	.778	.088	.781	-.487	.818	2.883	-1.711	-1.000	42
43	-.878	.088	-.121	.107	-.887	.382	-10.080	-.178	-1.000	43
44	-.287	2.018	-.223	2.038	-.238	.808	-7.142	8.813	2.841	44
45	-.118	.178	-.224	.287	-.238	.388	28.278	-1.238	-1.000	45
46	-.028	.028	-.028	.088	-.002	.028	44.184	-31.108	-1.000	46
47	-.014	.182	-.107	.210	-.044	.127	28.813	-4.780	-1.000	47
48	-.048	.238	.108	.278	-.083	.178	18.880	-3.328	-1.000	48
49	-.048	.280	-.077	.287	-.087	.182	-12.483	-4.470	-1.000	49
50	-.237	.410	-.043	.420	-.228	.087	-13.230	1.888	.318	50
51	-.278	.884	-.088	.811	-.248	.181	-18.828	2.488	.841	51
52	-.021	1.470	-.308	1.831	-.083	.807	-11.284	-18.474	-1.000	52
53	-.840	.188	.410	.383	-.728	.841	28.848	-.488	1.000	53
54	-.128	.148	.128	.288	.007	.130	43.018	38.102	13.048	54
55	-.088	.287	.088	.274	.082	.111	18.888	8.300	1.888	55
56	-.183	.281	-.001	.281	-.183	.207	-.184	-1.708	-1.000	56
57	-.824	-.308	.118	-.271	-.882	.188	18.188	.410	-1.000	57
58	-1.017	1.738	-1.840	2.588	-1.838	2.288	-28.888	-1.372	-1.000	58
59	-2.008	.388	1.780	1.341	-2.988	2.148	27.882	-.484	-1.000	59
60	-.384	.347	-.183	.392	-.408	.400	-13.823	.881	-1.000	60
61	-.010	.088	-.057	.048	-.028	.037	-48.447	-.848	-1.000	61
62	-.828	-.118	.307	.888	-.048	.388	81.781	-18.888	-1.000	62
63	-.124	-.084	-.142	.247	-.038	.143	82.888	-8.388	-1.000	63
64	-.218	.842	-.081	.880	-.288	.111	-27.888	1.823	.308	64
65	-.748	.788	-.053	.787	-.748	.021	4.141	1.088	.021	65
66	-.487	.801	-.028	.818	-.482	.032	-28.318	1.143	-.083	66
67	-.247	.328	-.020	.333	-.242	.048	-12.837	1.377	-.140	67
68	-.348	.378	-.278	-.083	-.842	.280	48.408	.128	-1.000	68
69	-.484	.488	-.074	.488	-.404	.078	80.317	1.372	1.38	69
70	.380	.848	-.041	.883	.342	.108	-11.388	1.818	.230	70
71	.380	.381	-.001	.381	.380	.020	-2.080	1.117	.043	71
72	.420	.483	-.008	.488	.418	.023	-11.188	1.111	.041	72
73	.284	.283	-.008	.283	.278	.078	-47.208	1.084	-.024	73
74	.388	.388	-.002	.388	.384	.002	-38.188	1.011	.004	74
75	.303	.488	.022	.482	.300	.081	7.888	1.838	.200	75
76	.342	.383	-.001	.383	.342	.020	-1.430	1.117	.043	76
77	.300	.344	-.004	.344	.300	.022	-6.883	1.147	.088	77
78	.247	.241	-.003	.248	.239	.004	-87.382	1.038	.013	78

SSTIPN. 1 LAYER OF GEOTEXTILE, P=1000 PPI, EXTENDED, T=1000

TOTAL NUMBER OF NODES----- 98
 NUMBER OF BAR ELEMENTS----- 11
 NUMBER OF DIFF. BAR MATERIALS----- 1
 NUMBER OF BEAM ELEMENTS----- 0
 NUMBER OF DIFF. BEAM MATERIALS----- 0
 NUMBER OF NODAL LINKS----- 0
 NUMBER OF INTERFACE ELEMENTS----- 0
 NO OF INTERFACE ELE IN PREEXIST PART----- 0
 NUMBER OF INTERFACE ELE IN FOUNDATION----- 0
 NUMBER OF INTERFACE MATERIALS----- 0
 TOTAL NUMBER OF SOIL ELEMENTS----- 78
 NUMBER OF DIFF SOIL MATERIALS----- 3
 NUMBER OF ELEMENTS IN FOUNDATION----- 18
 NUMBER OF NODES IN FOUNDATION----- 52
 NUMBER OF PREEXISTING ELEMENTS----- 0
 NUMBER OF PREEXISTING NODES----- 0
 NUMBER OF CONSTRUCTION LAYERS----- 4
 NUMBER OF LOAD CASES----- 2

CALING FACTOR ----- 1.00000

ATMOSPHERIC PRESSURE --- 1.06800

UNIT WEIGHT OF WATER --- 0.3120

COMPUTATION SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO. 1 APPLY LOAD CASE 1
 INCREMENT NO. 2 PUT ON LAYER NO. 1
 INCREMENT NO. 3 PUT ON LAYER NO. 2
 INCREMENT NO. 4 PUT ON LAYER NO. 3
 INCREMENT NO. 5 PUT ON LAYER NO. 4
 INCREMENT NO. 6 PUT ON LAYER NO. 5
 INCREMENT NO. 7 PUT ON LAYER NO. 6
 INCREMENT NO. 8 APPLY LOAD CASE 2

NOODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		S.C. CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	.000	.000	1	1	1
2	10.000	.000	1	1	1
3	20.000	.000	1	1	1
4	30.000	.000	1	1	1
5	38.000	.000	1	1	1
6	42.000	.000	1	1	1
7	50.000	.000	1	1	1
8	54.000	.000	1	1	1
9	58.000	.000	1	1	1
10	58.000	.000	1	1	1
11	74.000	.000	1	1	1
12	82.000	.000	1	1	1
13	90.000	.000	1	1	1
14	.000	4.000	0	0	0
15	10.000	4.000	0	0	0
16	20.000	4.000	0	0	0
17	30.000	4.000	0	0	0
18	38.000	4.000	0	0	0
19	42.000	4.000	0	0	0
20	50.000	4.000	0	0	0
21	54.000	4.000	0	0	0
22	58.000	4.000	0	0	0
23	58.000	4.000	0	0	0
24	74.000	4.000	0	0	0
25	82.000	4.000	0	0	0
26	90.000	4.000	1	0	1
27	.000	7.000	1	0	1
28	10.000	7.000	0	0	0
29	20.000	7.000	0	0	0
30	30.000	7.000	0	0	0
31	38.000	7.000	0	0	0
32	42.000	7.000	0	0	0
33	50.000	7.000	0	0	0
34	54.000	7.000	0	0	0
35	58.000	7.000	0	0	0
36	58.000	7.000	0	0	0
37	74.000	7.000	0	0	0
38	82.000	7.000	0	0	0
39	90.000	7.000	1	0	1
40	.000	10.000	1	0	1
41	10.000	10.000	0	0	0
42	20.000	10.000	0	0	0
43	30.000	10.000	0	0	0
44	38.000	10.000	0	0	0
45	42.000	10.000	0	0	0
46	50.000	10.000	0	0	0
47	54.000	10.000	0	0	0
48	58.000	10.000	0	0	0
49	58.000	10.000	0	0	0
50	74.000	10.000	0	0	0
51	82.000	10.000	0	0	0
52	90.000	10.000	1	0	1
53	38.000	11.500	0	0	0
54	42.000	11.500	0	0	0
55	50.000	11.500	0	0	0
56	54.000	11.500	0	0	0
57	58.000	11.500	0	0	0
58	58.000	11.500	0	0	0
59	74.000	11.500	0	0	0
60	82.000	11.500	0	0	0
61	90.000	11.500	1	0	1
62	42.000	13.000	0	0	0
63	50.000	13.000	0	0	0
64	54.000	13.000	0	0	0
65	58.000	13.000	0	0	0
66	58.000	13.000	0	0	0
67	74.000	13.000	0	0	0
68	82.000	13.000	0	0	0
69	90.000	13.000	1	0	1
70	48.000	14.000	0	0	0
71	50.000	14.000	0	0	0
72	54.000	14.000	0	0	0
73	58.000	14.000	0	0	0
74	58.000	14.000	0	0	0
75	74.000	14.000	0	0	0
76	82.000	14.000	0	0	0
77	90.000	14.000	1	0	1
78	50.000	15.000	0	0	0
79	54.000	15.000	0	0	0
80	58.000	15.000	0	0	0
81	58.000	15.000	0	0	0
82	74.000	15.000	0	0	0
83	82.000	15.000	0	0	0
84	90.000	15.000	1	0	1
85	54.000	16.000	0	0	0
86	58.000	16.000	0	0	0
87	58.000	16.000	0	0	0
88	74.000	16.000	0	0	0
89	82.000	16.000	0	0	0
90	90.000	16.000	1	0	1
91	58.000	17.000	0	0	0
92	58.000	17.000	0	0	0
93	74.000	17.000	0	0	0
94	82.000	17.000	0	0	0
95	90.000	17.000	1	0	1

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LRNGTH
	30.	1.50	0.

ELMT NO.	CONNECTED NODES	MATL NO.
	I J	
1	41 42	
2	42 43	
3	43 44	
4	44 45	
5	45 46	
6	46 47	
7	47 48	
8	48 49	
9	49 50	
10	50 51	
11	51 52	

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S CONSTANT	MODULUS EXPONENT	RATIO	BULK MODULUS CONSTANT	MODULUS EXPONENT	STRENGTH C	PARAMETERS PHI	DPHI	KD
1	.0880	6000.00	.500	.500	1500.00	.500	.00	35.00	.00	.50
2	.0830	4000.00	.300	.500	20.00	.200	.08	30.00	.00	.50
3	.0800	1000.00	.400	.700	500.00	.500	.50	40.00	.00	.50

FOUR NODES SOLID ELEMENT DATA

ELMT NO.	CONNECTED NODES	MATL NO.	ELEMENT CENTER COORDINATES
	I J K L		X-ORD Y-ORD
1	1 2 3 4	3	5.000 2.000
2	2 3 4 5	3	15.000 2.000
3	3 4 5 6	3	25.000 2.000
4	4 5 6 7	2	33.000 2.000
5	5 6 7 8	2	39.000 2.000
6	6 7 8 9	2	46.000 2.000
7	7 8 9 10	2	50.000 2.000
8	8 9 10 11	2	56.000 2.000
9	9 10 11 12	2	62.000 2.000
10	10 11 12 13	3	70.000 2.000
11	11 12 13 14	3	78.000 2.000
12	12 13 14 15	3	86.000 2.000
13	13 14 15 16	3	9.000 5.500
14	14 15 16 17	3	15.000 5.500
15	15 16 17 18	2	25.000 5.500
16	16 17 18 19	2	33.000 5.500
17	17 18 19 20	2	39.000 5.500
18	18 19 20 21	2	46.000 5.500
19	19 20 21 22	2	50.000 5.500
20	20 21 22 23	2	56.000 5.500
21	21 22 23 24	2	62.000 5.500
22	22 23 24 25	2	70.000 5.500
23	23 24 25 26	3	78.000 5.500
24	24 25 26 27	3	86.000 5.500
25	25 26 27 28	3	9.000 8.500
26	26 27 28 29	3	15.000 8.500
27	27 28 29 30	2	25.000 8.500
28	28 29 30 31	2	33.000 8.500
29	29 30 31 32	2	39.000 8.500
30	30 31 32 33	2	46.000 8.500
31	31 32 33 34	2	50.000 8.500
32	32 33 34 35	2	56.000 8.500
33	33 34 35 36	2	62.000 8.500
34	34 35 36 37	2	70.000 8.500
35	35 36 37 38	3	78.000 8.500
36	36 37 38 39	3	86.000 8.500
37	37 38 39 40	3	9.000 10.750
38	38 39 40 41	3	15.000 10.750
39	39 40 41 42	3	25.000 10.750
40	40 41 42 43	3	33.000 10.750
41	41 42 43 44	1	39.000 10.750
42	42 43 44 45	1	46.000 10.750
43	43 44 45 46	1	50.000 10.750
44	44 45 46 47	1	56.000 10.750
45	45 46 47 48	1	62.000 10.750
46	46 47 48 49	1	70.000 10.750
47	47 48 49 50	1	78.000 10.750
48	48 49 50 51	1	86.000 10.750
49	49 50 51 52	1	9.000 12.250
50	50 51 52 53	1	15.000 12.250
51	51 52 53 54	1	25.000 12.250
52	52 53 54 55	1	33.000 12.250
53	53 54 55 56	1	39.000 12.250
54	54 55 56 57	1	46.000 12.250
55	55 56 57 58	1	50.000 12.250
56	56 57 58 59	1	56.000 12.250
57	57 58 59 60	1	62.000 12.250
58	58 59 60 61	1	70.000 12.250
59	59 60 61 62	1	78.000 12.250
60	60 61 62 63	1	86.000 12.250
61	61 62 63 64	1	9.000 14.500
62	62 63 64 65	1	15.000 14.500
63	63 64 65 66	1	25.000 14.500
64	64 65 66 67	1	33.000 14.500
65	65 66 67 68	1	39.000 14.500
66	66 67 68 69	1	46.000 14.500
67	67 68 69 70	1	50.000 14.500
68	68 69 70 71	1	56.000 14.500
69	69 70 71 72	1	62.000 14.500
70	70 71 72 73	1	70.000 14.500
71	71 72 73 74	1	78.000 14.500
72	72 73 74 75	1	86.000 14.500
73	73 74 75 76	1	9.000 16.500
74	74 75 76 77	1	15.000 16.500
75	75 76 77 78	1	25.000 16.500
76	76 77 78 79	1	33.000 16.500
77	77 78 79 80	1	39.000 16.500
78	78 79 80 81	1	46.000 16.500
79	79 80 81 82	1	50.000 16.500
80	80 81 82 83	1	56.000 16.500
81	81 82 83 84	1	62.000 16.500
82	82 83 84 85	1	70.000 16.500
83	83 84 85 86	1	78.000 16.500
84	84 85 86 87	1	86.000 16.500
85	85 86 87 88	1	9.000 18.500
86	86 87 88 89	1	15.000 18.500
87	87 88 89 90	1	25.000 18.500
88	88 89 90 91	1	33.000 18.500
89	89 90 91 92	1	39.000 18.500
90	90 91 92 93	1	46.000 18.500
91	91 92 93 94	1	50.000 18.500
92	92 93 94 95	1	56.000 18.500
93	93 94 95 96	1	62.000 18.500
94	94 95 96 97	1	70.000 18.500
95	95 96 97 98	1	78.000 18.500
96	96 97 98 99	1	86.000 18.500
97	97 98 99 100	1	9.000 20.500
98	98 99 100 101	1	15.000 20.500
99	99 100 101 102	1	25.000 20.500
100	100 101 102 103	1	33.000 20.500
101	101 102 103 104	1	39.000 20.500
102	102 103 104 105	1	46.000 20.500
103	103 104 105 106	1	50.000 20.500
104	104 105 106 107	1	56.000 20.500
105	105 106 107 108	1	62.000 20.500
106	106 107 108 109	1	70.000 20.500
107	107 108 109 110	1	78.000 20.500
108	108 109 110 111	1	86.000 20.500
109	109 110 111 112	1	9.000 22.500
110	110 111 112 113	1	15.000 22.500
111	111 112 113 114	1	25.000 22.500
112	112 113 114 115	1	33.000 22.500
113	113 114 115 116	1	39.000 22.500
114	114 115 116 117	1	46.000 22.500
115	115 116 117 118	1	50.000 22.500
116	116 117 118 119	1	56.000 22.500
117	117 118 119 120	1	62.000 22.500
118	118 119 120 121	1	70.000 22.500
119	119 120 121 122	1	78.000 22.500
120	120 121 122 123	1	86.000 22.500
121	121 122 123 124	1	9.000 24.500
122	122 123 124 125	1	15.000 24.500
123	123 124 125 126	1	25.000 24.500
124	124 125 126 127	1	33.000 24.500
125	125 126 127 128	1	39.000 24.500
126	126 127 128 129	1	46.000 24.500
127	127 128 129 130	1	50.000 24.500
128	128 129 130 131	1	56.000 24.500
129	129 130 131 132	1	62.000 24.500
130	130 131 132 133	1	70.000 24.500
131	131 132 133 134	1	78.000 24.500
132	132 133 134 135	1	86.000 24.500
133	133 134 135 136	1	9.000 26.500
134	134 135 136 137	1	15.000 26.500
135	135 136 137 138	1	25.000 26.500
136	136 137 138 139	1	33.000 26.500
137	137 138 139 140	1	39.000 26.500
138	138 139 140 141	1	46.000 26.500
139	139 140 141 142	1	50.000 26.500
140	140 141 142 143	1	56.000 26.500
141	141 142 143 144	1	62.000 26.500
142	142 143 144 145	1	70.000 26.500
143	143 144 145 146	1	78.000 26.500
144	144 145 146 147	1	86.000 26.500
145	145 146 147 148	1	9.000 28.500
146	146 147 148 149	1	15.000 28.500
147	147 148 149 150	1	25.000 28.500
148	148 149 150 151	1	33.000 28.500
149	149 150 151 152	1	39.000 28.500
150	150 151 152 153	1	46.000 28.500
151	151 152 153 154	1	50.000 28.500
152	152 153 154 155	1	56.000 28.500
153	153 154 155 156	1	62.000 28.500
154	154 155 156 157	1	70.000 28.500
155	155 156 157 158	1	78.000 28.500
156	156 157 158 159	1	86.000 28.500
157	157 158 159 160	1	9.000 30.500
158	158 159 160 161	1	15.000 30.500
159	159 160 161 162	1	25.000 30.500
160	160 161 162 163	1	33.000 30.500
161	161 162 163 164	1	39.000 30.500
162	162 163 164 165	1	46.000 30.500
163	163 164 165 166	1	50.000 30.500
164	164 165 166 167	1	56.000 30.500
165	165 166 167 168	1	62.000 30.500
166	166 167 168 169	1	70.000 30.500
167	167 168 169 170	1	78.000 30.500
168	168 169 170 171	1	86.000 30.500
169	169 170 171 172	1	9.000 32.500
170	170 171 172 173	1	15.000 32.500
171	171 172 173 174	1	25.000 32.500
172	172 173 174 175	1	33.000 32.500
173	173 174 175 176	1	39.000 32.500
174	174 175 176 177	1	46.000 32.500
175	175 176 177 178	1	50.000 32.500
176	176 177 178 179	1	56.000 32.500
177	177 178 179 180	1	62.000 32.500
178	178 179 180 181	1	70.000 32.500
179	179 180 181 182	1	78.000 32.500
180	180 181 182 183	1	86.000 32.500
181	181 182 183 184	1	9.000 34.500
182	182 183 184 185	1	15.000 34.500
183	183 184 185 186	1	25.000 34.500
184	184 185 186 187	1	33.000 34.500
185	185 186 187 188	1	39.000 34.500
186	186 187 188 189	1	46.000 34.500
187	187 188 189 190	1	50.000 34.500
188	188 189 190 191	1	56.000 34.500
189	189 190 191 192	1	62.000 34.500
190	190 191 192 193	1	70.000 34.500
191	191 192 193 194	1	78.000 34.500
192	192 193 194 195	1	86.000 34.500
193	193 194 195 196	1	9.000 36.500
194	194 195 196 197	1	15.000 36.500
195	195 196 197 198	1	25.000 36.500
196	196 197 198 199	1	33.000 36.500
197	197 198 199 200	1	39.000 36.500
198	198 199 200 201	1	46.000 36.500
199	199 200 201 202	1	50.000 36.500
200	200 201 202 203	1	56.000 36.500
201	201 202 203 204	1	62.000 36.500
202	202 203 204 205	1	70.000 36.500
203	203 204 205 206	1	78.000 36.500
204	204 205 206 207	1	86.000 36.500
205	205 206 207 208	1	9.000 38

```

*****
*****
*                               *
* LOAD CASE ----- 2         *
*                               *
*****
*****
    
```

LARGEST ELE. NO. IN THIS INCREMENT 78
 LARGEST NP NO IN THIS INCREMENT 88

BAND WIDTH----- 40
 TOTAL NUMBER OF EQUATIONS----- 222
 NUMBER OF EQUATIONS IN BLOCK----- 84
 NUMBER OF BLOCKS----- 3
 NUMBER OF N.P. FORCE CARDS----- 3
 NUMBER OF PRESSURE CARDS----- 0

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP	X-FORCE	Y-FORCE
1	0.	0.
2	0.	0.
3	0.	0.
4	0.	0.
5	0.	0.
6	0.	0.
7	0.	0.
8	0.	0.
9	0.	0.
10	0.	0.
11	0.	0.
12	0.	0.
13	0.	0.
14	0.	0.
15	0.	0.
16	0.	0.
17	0.	0.
18	0.	0.
19	0.	0.
20	0.	0.
21	0.	0.
22	0.	0.
23	0.	0.
24	0.	0.
25	0.	0.
26	0.	0.
27	0.	0.
28	0.	0.
29	0.	0.
30	0.	0.
31	0.	0.
32	0.	0.
33	0.	0.
34	0.	0.
35	0.	0.
36	0.	0.
37	0.	0.
38	0.	0.
39	0.	0.
40	0.	0.
41	0.	0.
42	0.	0.
43	0.	0.
44	0.	0.
45	0.	0.
46	0.	0.
47	0.	0.
48	0.	0.
49	0.	0.
50	0.	0.
51	0.	0.
52	0.	0.
53	0.	0.
54	0.	0.
55	0.	0.
56	0.	0.
57	0.	0.
58	0.	0.
59	0.	0.
60	0.	0.
61	0.	0.
62	0.	0.

64 0. 0.
 65 0. 0.
 66 0. 0.
 67 0. 0.
 68 0. 0.
 69 0. 0.
 70 0. 0.
 71 0. 0.
 72 0. 0.
 73 0. 0.
 74 0. 0.
 75 0. 0.
 76 0. 0.
 77 0. 0.
 78 0. 0.
 79 0. 0.
 80 0. 0.
 81 0. 0.
 82 0. 0.
 83 0. 0.
 84 0. 0.
 85 0. 0.
 86 0. 0.
 87 0. 0.
 88 0. 0.
 89 0. 0.
 90 0. 0.
 91 0. 0.
 92 0. -4.50
 93 0. -4.50
 94 0. -2.00
 95 0. 0.

LOAD CASE : 2 ITERATION : 2

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0001	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0000	.0000	.0000	.0000	15
16	.0000	.0000	.0000	.0000	.0000	.0000	.0000	16
17	.0000	.0000	.0000	.0000	.0000	.0000	.0000	17
18	-.0135	.0022	.0000	-.2080	.0388	.0000	2121	18
19	-.0171	-.0038	.0000	-.2830	-.0742	.0000	3022	19
20	-.0089	-.0051	.0000	-.1874	-.1278	.0000	2108	20
21	-.0081	.0034	.0000	-.0410	.1380	.0000	1824	21
22	.0003	.0010	.0000	.0005	-.0037	.0000	.0038	22
23	-.0013	.0028	.0000	-.0005	.0080	.0000	.0061	23
24	.0009	-.0057	.0000	.0015	-.0105	.0000	.0108	24
25	.0021	.0020	.0000	.0024	-.0084	.0000	.0089	25
26	.0000	.0006	.0000	.0000	-.0028	.0000	.0028	26
27	.0000	.0001	.0000	.0000	.0010	.0000	.0010	27
28	.0001	.0001	.0000	.0014	.0002	.0000	.0015	28
29	.0001	.0000	.0000	.0018	.0003	.0000	.0021	29
30	-.0183	.0048	.0000	-.2107	.0838	.0000	2201	30
31	-.0302	.0048	.0000	-.3189	.0708	.0000	3277	31
32	-.0377	-.0052	.0000	-.3889	-.1287	.0000	4071	32
33	-.0450	-.0081	.0000	-.3188	-.2848	.0000	4067	33
34	-.0855	.0040	.0000	-.2844	-.2388	.0000	3843	34
35	-.0741	-.0082	.0000	-.1892	-.1881	.0000	2844	35
36	-.0030	-.0074	.0000	-.0014	-.0187	.0000	.0188	36
37	-.0019	.0113	.0000	.0027	.0194	.0000	.0195	37
38	.0028	-.0038	.0000	.0024	-.0103	.0000	.0108	38
39	.0000	.0019	.0000	.0000	-.0042	.0000	.0042	39
40	.0000	-.0002	.0000	.0000	.0018	.0000	.0018	40
41	-.0008	.0002	.0000	-.0081	.0010	.0000	.0082	41
42	-.0287	.0088	.0000	-.1019	.0207	.0000	1040	42
43	-.0888	.0107	.0000	-.2728	.1078	.0000	2841	43
44	-.0890	.0041	.0000	-.3011	.0672	.0000	3088	44
45	-.0804	.0080	.0000	-.3067	.1788	.0000	3840	45
46	-.0881	.0088	.0000	-.2884	-.3821	.0000	4427	46
47	-.0853	.0088	.0000	-.2274	-.3247	.0000	3887	47
48	-.0884	.0129	.0000	-.1888	-.2482	.0000	3089	48
49	-.0404	-.1110	.0000	-.0787	-.2838	.0000	2887	49
50	-.0003	-.0278	.0000	-.0032	-.0487	.0000	.0488	50
51	.0000	-.0030	.0000	.0012	-.0118	.0000	.0118	51
52	.0000	.0028	.0000	.0000	.0073	.0000	.0073	52
53	.0070	.0004	.0000	.0087	.1080	.0000	.0088	53
54	-.0881	-.0088	.0000	-.4880	-.1808	.0000	5087	54
55	-.0814	-.0101	.0000	-.2837	-.3241	.0000	4374	55
56	-.0818	.0088	.0000	-.2853	-.3480	.0000	4208	56
57	-.0813	.0104	.0000	-.1781	-.2718	.0000	2231	57
58	-.0078	-.1177	.0000	-.0801	-.2828	.0000	2888	58
59	-.0047	.0318	.0000	.0432	-.0887	.0000	.1089	59
60	-.0028	-.0027	.0000	-.0040	-.0108	.0000	.0112	60

81	.0000	.0027	.0000	.0000	-.0087	.0000	.0087	81
82	-.0880	-.0088	.0000	-.1282	-.0202	.0000	.1288	82
83	-.0718	-.0082	.0000	-.1388	-.1470	.0000	.2323	83
84	-.0788	.0082	.0000	-.1438	-.1712	.0000	.2334	84
85	-.0888	.0123	.0000	-.1821	-.1718	.0000	.2388	85
86	-.0224	-.1201	.0000	-.0884	-.2204	.0000	.2307	86
87	-.0180	-.0338	.0000	-.0338	-.1274	.0000	.1318	87
88	-.0088	-.0030	.0000	-.0078	-.0082	.0000	.0118	88
89	.0000	.0027	.0000	.0000	-.0037	.0000	.0037	89
90	-.0781	-.0044	.0000	-.1283	-.0473	.0000	.1388	90
91	-.0883	.0082	.0000	-.1820	-.0880	.0000	.1783	91
92	-.1188	.0074	.0000	-.1743	-.1133	.0000	.2078	92
93	-.1288	.0188	.0000	-.1848	-.1178	.0000	.2180	93
94	-.0343	-.1208	.0000	-.0838	-.2002	.0000	.2171	94
95	-.0228	-.0243	.0000	-.0448	-.1380	.0000	.1481	95
96	-.0077	-.0038	.0000	-.0082	-.0087	.0000	.0127	96
97	.0000	.0024	.0000	.0000	-.0028	.0000	.0028	97
98	-.1207	-.0010	.0000	-.1801	-.0823	.0000	.1888	98
99	-.1281	.0071	.0000	-.1883	-.0848	.0000	.1882	99
80	-.1338	.0187	.0000	-.1883	-.0830	.0000	.1778	80
81	-.2888	-.3208	.0000	-.3184	-.3732	.0000	.4812	81
82	.7202	-.2481	.0000	.8838	-.3373	.0000	.7828	82
83	1.1187	-.0408	.0000	1.1183	-.0448	.0000	1.1182	83
84	.0000	.1208	.0000	.0000	.1171	.0000	.1171	84
85	-.1302	.0072	.0000	-.1402	-.0121	.0000	.1407	85
86	-.1408	.0238	.0000	-.1818	-.0008	.0000	.1818	86
87	-.1418	-.0418	.0000	-.1834	-.0887	.0000	.2671	87
88	.8811	-.0438	.0000	.8081	-.0888	.0000	1.0040	88
89	1.8708	-.0810	.0000	1.8818	-.0721	.0000	1.8838	89
90	.0000	.3718	.0000	.0000	.3832	.0000	.3832	90
91	-.0703	.0288	.0000	-.0703	-.0288	.0000	.0748	91
92	-.0704	-.0482	.0000	-.0704	-.0452	.0000	.0487	92
93	.7380	-.0087	.0000	.7380	-.0087	.0000	1.1713	93
94	1.8411	-.0838	.0000	1.8411	-.0838	.0000	1.8420	94
95	.0000	.0000	.0000	.0000	.0000	.0000	.0000	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO. AXIAL FORCE

INCREMENTAL VALUES

1	-.0782
2	-.0837
3	-.0103
4	-.0072
5	-.0068
6	-.0238
7	-.0231
8	-.0872
9	.1804
10	.0012
11	.0001

TOTAL VALUES

1	-.2904
2	-.8188
3	-.1389
4	-.0278
5	.1434
6	.3078
7	.3088
8	.4047
9	.2830
10	.0078
11	.0046

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	538.1	340.4	232.1	188	.003	.005	.004	.008	.002	.004	1
2	811.1	323.8	220.4	188	.000	.003	.018	.010	.008	.018	2
3	505.1	318.3	218.3	187	.023	.023	.058	.037	.037	.074	3
4	.7	17.2	.2	484	1.888	-.478	2.388	2.231	-1.007	3.237	4
5	.6	17.1	.2	484	700	480	7.182	4.187	-3.008	7.188	5
6	.7	17.9	.2	484	748	2.828	8.080	4.328	-2.888	8.231	6
7	.7	18.4	.2	483	-1.305	3.335	3.018	3.783	-1.752	5.538	7
8	.8	18.9	.2	484	-784	1.784	3.60	1.882	-.872	2.734	8
9	483.3	340.9	188.8	222	.077	.122	.014	.123	.008	.118	9
10	483.4	340.8	188.2	228	-.013	.207	.018	.207	-.013	.221	10
11	478.2	371.0	191.3	242	-.008	.188	-.081	.207	-.013	.220	11
12	513.8	388.8	212.4	208	.018	.100	-.047	.108	.008	.088	12
13	488.8	288.8	220.1	126	.010	.008	.004	.011	.008	.008	13
14	388.8	224.8	172.8	.118	.062	-.013	.037	.011	.008	.040	14
15	2.0	13.8	.7	478	1.087	-1.040	3.118	1.884	-1.887	3.780	15
16	4.0	18.2	1.4	487	2.710	-1.812	4.838	3.748	-2.748	6.488	16
17	1.5	18.9	.8	488	1.248	3.60	8.882	3.820	-2.231	6.051	17
18	.8	17.1	.2	484	-1.224	3.043	8.138	4.248	-2.430	8.877	18
19	.8	17.1	.2	484	-1.848	3.733	8.730	4.827	-3.138	8.087	19
20	.8	18.2	.2	484	-1.808	4.188	4.024	4.818	-2.238	7.088	20
21	.8	17.1	.2	484	-1.230	2.781	2.474	3.104	-1.883	4.888	21
22	402.8	311.8	188.0	218	.038	.328	.040	.327	.038	.388	22
23	34.8	308.7	184.4	231	-.004	.230	-.108	.242	-.018	.288	23
24	484.1	314.8	188.1	180	.030	.108	-.088	.118	.021	.088	24
25	380.1	223.8	188.8	121	.033	-.008	.042	.042	-.018	.088	25
26	.4	13.4	.1	488	4.88	-.383	1.828	.882	-.880	1.333	26
27	.0	.1	.0	488	1.804	-1.087	1.888	2.200	-1.384	3.884	27
28	1.3	13.8	.4	484	1.138	-.888	1.017	1.272	-.800	2.072	28
29	.8	14.4	.3	481	388	.847	2.071	.788	-.322	2.088	29
30	.1	18.2	.4	488	.874	2.408	-.228	2.410	-.883	3.082	30
31	.8	18.7	.2	484	-1.183	3.188	-1.933	3.372	-1.388	4.732	31
32	.8	18.8	.2	484	-1.324	3.018	-2.831	3.438	-1.748	5.180	32
33	.7	17.7	.2	484	-1.911	8.420	.241	8.422	-1.913	7.388	33
34	.7	17.4	.2	484	-.487	4.380	-.102	4.381	-.488	4.888	34
35	284.7	288.8	118.4	.277	-.011	-.482	-.111	.488	-.018	.488	35
36	348.0	217.8	181.4	.182	.008	-.007	.078	.007	.008	.088	36
37	288.8	438.8	104.8	378	.487	-.107	18.887	8.808	-8.888	18.888	37
38	342.0	438.8	128.0	.381	-.388	1.784	10.434	10.488	-8.418	20.074	38
39	481.8	480.8	188.8	288	-1.384	3.788	10.227	8.833	-4.828	11.480	39
40	387.7	448.3	148.1	334	-1.224	4.784	2.288	4.888	-1.438	6.430	40
41	384.8	447.8	147.8	338	-1.237	8.112	-1.082	8.188	-1.281	8.437	41
42	187.8	418.8	84.8	434	-1.418	4.138	-.804	4.147	-1.427	8.874	42
43	11.8	388.8	4.0	488	-1.030	3.888	-4.787	4.871	-2.038	8.708	43
44	724.4	848.1	282.8	281	1.828	-.288	-2.188	2.388	-.283	2.878	44
45	1388.8	783.1	834.8	.078	-.033	.088	-.038	.088	-.038	.088	45
46	11.8	388.8	4.0	488	-.040	.014	.014	.014	-.041	.087	46
47	.1	4.0	.0	488	.082	.023	.788	.438	-.333	.771	47
48	142.3	418.2	48.7	440	.021	.088	.803	.280	-.214	.808	48
49	11.8	388.8	4.0	488	-.288	.888	1.304	1.822	-.818	1.840	49
50	.1	4.0	.0	488	-1.007	.808	3.387	1.881	-1.883	3.884	50
51	824.8	487.0	208.0	.281	-.327	2.880	.281	2.888	-.333	3.288	51
52	740.8	881.8	288.8	282	-.228	2.788	-.181	2.788	-.228	2.888	52
53	837.8	848.4	408.8	182	-.078	.088	.074	.077	-.077	.181	53
54	.1	4.0	.0	488	-.388	-.327	2.321	1.43	-1.188	2.388	54
55	11.8	388.8	4.0	488	-.310	-.281	2.807	1.881	-1.801	3.882	55
56	11.8	388.8	4.0	488	-.218	-.221	4.433	2.227	-2.228	4.488	56
57	.1	4.0	.0	488	-1.187	.083	4.887	1.880	-2.883	4.814	57
58	1804.3	840.7	744.8	.077	-.418	2.871	1.187	2.878	-.822	3.487	58
59	1283.2	788.4	887.4	133	-.382	2.783	-.348	2.783	-.38	3.124	59
60	481.3	481.8	188.8	288	-.102	.084	.027	.088	-.101	.188	60
61	.1	4.0	.0	488	-.878	-.828	2.474	.380	-1.330	2.710	61
62	.1	4.0	.0	488	.344	-.238	1.207	.723	-.817	1.340	62
63	281.7	430.0	84.1	.381	-.228	-1.171	1.318	.283	-.228	3.088	63
64	.1	4.0	.0	488	-.328	8.828	18.888	14.281	-4.087	18.348	64
65	.1	4.0	.0	488	8.444	23.410	23.888	27.888	-10.822	38.208	65
66	.1	4.0	.0	488	2.880	18.288	84.312	84.240	-41.801	98.040	66
67	.1	4.0	.0	488	8.814	-3.880	-87.288	30.808	-27.838	88.284	67
68	.1	4.0	.0	488	1.138	-.012	.221	.187	-.071	.288	68
69	212.0	423.8	78.4	411	-.234	-.218	.213	.288	-.243	.801	69
70	.1	4.0	.0	488	.888	10.837	-1.78	10.872	-.823	8.848	70
71	.1	4.0	.0	488	12.207	28.837	-13.081	27.800	-13.271	41.171	71
72	.1	4.0	.0	488	7.344	17.238	32.714	28.468	-18.812	40.820	72
73	.1	4.0	.0	488	18.738	-10.288	-28.887	21.888	-18.488	37.481	73
74	.1	4.0	.0	488	-.280	-.183	-7.481	3.788	-3.888	7.488	74
75	338.1	438.8	123.8	388	.008	.080	.018	.081	.008	.082	75
76	.1	4.0	.0	488	-11.288	18.443	4.321	18.888	-11.421	30.020	76
77	.1	4.0	.0	488	10.138	18.288	-8.344	18.747	-10.483	28.230	77
78	.1	4.0	.0	488	20.073	-14.878	-.0.882	20.878	-18.877	38.383	78

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-MX	SIG-1	SIG-3	TAU-MAX	THEYA	SIG1/SIG3	LEVEL	ELE
1	.283	.814	.008	.814	-.283	.126	1.828	1.888	.081	1
2	.242	.487	.040	.803	.237	.133	8.841	2.121	.088	2
3	.348	.372	-.134	.484	-.274	.138	42.288	2.208	.082	3
4	.383	.884	.008	.884	-.383	.088	1.821	1.447	1.717	4
5	.371	.880	.018	.881	-.370	.081	4.888	1.488	1.812	5
6	.482	.888	.013	.888	-.481	.088	3.882	1.424	1.888	6
7	.812	.712	.007	.712	-.811	.100	1.824	1.382	2.003	7
8	.333	.823	-.002	.823	-.333	.088	8.828	1.872	1.808	8
9	.380	.887	.023	.888	-.388	.318	2.037	2.778	1.88	9
10	.408	1.447	.028	1.448	-.408	.822	1.882	3.878	.280	10
11	.440	1.428	.188	1.484	-.418	.818	8.885	3.488	.288	11
12	.381	.810	-.100	.828	-.382	.283	-10.388	2.880	.184	12
13	.188	.308	.008	.308	-.188	.088	3.808	1.820	.082	13
14	.134	.218	.087	.283	-.088	.078	28.883	2.828	.083	14
15	.103	.174	-.013	.177	-.103	.038	10.175	1.811	.784	15
16	.278	.288	-.038	.221	-.242	.038	40.401	1.318	.784	16
17	.284	.487	.017	.420	-.231	.048	11.180	1.270	.884	17
18	.282	.488	.010	.488	-.281	.084	8.807	1.301	1.087	18
19	.282	.488	.012	.488	-.281	.087	8.800	1.328	1.182	19
20	.482	.808	.008	.808	-.481	.084	4.727	1.238	1.183	20
21	.310	.417	.008	.417	-.310	.084	2.788	1.348	1.071	21
22	.338	.888	.080	1.870	-.333	.888	2.878	8.008	.400	22
23	.323	1.211	-.178	1.248	-.288	.478	-10.877	4.328	.301	23
24	.318	.888	-.108	.887	-.287	.208	-18.108	2.428	.129	24
25	.178	.083	.078	.218	.040	.088	80.781	8.408	.078	25
26	.031	.007	.023	.048	-.007	.028	83.808	8.087	-.000	26
27	.088	.080	.061	.088	-.080	.028	88.888	8.880	.080	27
28	.117	.034	.000	.117	-.034	.042	-88.880	3.472	.834	28
29	.188	.283	.008	.284	-.181	.048	3.828	1.870	.920	29
30	.188	.288	-.002	.288	-.188	.044	-1.318	1.488	.888	30
31	.238	.381	-.008	.381	-.238	.088	-2.823	1.488	1.188	31
32	.181	.280	-.013	.281	-.180	.081	-7.213	1.824	1.014	32
33	.484	.808	.002	.808	-.484	.087	.781	1.231	1.138	33
34	.888	.888	.008	.888	-.888	.080	.038	1.178	1.000	34
35	.473	.788	-.124	.727	-.488	.811	-8.838	3.828	.328	35
36	.087	.188	-.008	.204	-.087	.118	-2.072	4.844	.088	36
37	.377	.178	.088	.488						

43	.214	.788	-.177	.847	-.188	.341	-18.884	5.140	1.538	43
44	.383	1.507	-.118	1.518	-.370	.578	-5.937	4.104	1.184	44
45	-.170	-.287	-.181	-.385	-.238	.302	18.687	-1.535	-1.000	45
46	.000	.024	.011	.028	-.008	.017	21.422	-8.780	-1.000	46
47	.087	141	.110	.217	-.019	118	34.468	-11.338	-1.000	47
48	.284	487	.287	841	.071	288	34.574	8.088	3.008	48
49	-.133	-.114	-.012	-.118	-.134	.124	-2.772	-.858	-1.000	49
50	-.008	.288	-.070	.273	-.022	.148	-14.188	-12.228	-1.000	50
51	-.043	1.146	-.814	1.407	-.303	.888	-22.988	-4.638	-1.000	51
52	-.312	.887	.288	.924	-.378	.851	15.102	-2.437	-1.000	52
53	.484	.388	.188	.380	-.478	.434	11.380	1.818	-1.000	53
54	.042	.084	.080	.134	-.028	.081	81.018	-4.782	-1.000	54
55	-.200	.308	.278	.534	-.028	.280	38.400	-20.800	-1.000	55
56	-.188	.188	.127	.238	-.208	.222	17.513	-1.188	-.000	56
57	-.038	.180	-.078	.188	-.082	.134	-18.804	-3.007	-1.000	57
58	-1.881	1.181	-.780	1.342	-2.032	1.687	-13.384	-.861	-1.000	58
59	-1.883	.818	.880	.743	-2.111	1.427	12.218	-.882	-1.000	59
60	-.388	.302	.047	.308	-.388	.332	4.080	-.880	-1.000	60
61	.012	.016	-.003	.018	.010	.004	-31.364	1.727	.270	61
62	.188	-.127	-.118	.284	-.031	.118	30.178	8.438	2.788	62
63	.788	-.180	.484	.874	-.387	.870	88.112	-2.887	-1.000	63
64	.471	.807	-.140	.888	.383	.188	-32.073	1.813	.302	64
65	.881	.722	-.010	.724	.873	.023	-12.482	1.088	.028	65
66	.808	.837	-.038	.882	.481	.041	-33.848	1.170	.083	66
67	.232	.318	-.023	.323	.228	.048	-13.943	1.428	.180	67
68	.027	.031	.033	.088	.001	.034	87.871	73.738	27.028	68
69	.441	.208	-.204	.488	-.310	.403	74.822	-1.800	.000	69
70	.803	.888	-.038	.801	-.890	.088	-18.812	1.228	.084	70
71	.888	.833	-.008	.833	.884	.028	-8.104	1.084	.031	71
72	.402	.438	-.013	.442	.388	.022	-17.738	1.113	.082	72
73	.288	.281	-.010	.284	.273	.011	-81.188	1.078	.028	73
74	.011	-.018	-.001	.020	.011	.004	-8.284	1.741	-.278	74
75	.381	.888	.018	.870	.348	.110	4.888	1.831	.234	75
76	.281	.332	.002	.332	.281	.020	2.420	1.140	.082	76
77	.388	.378	-.003	.378	.388	.020	-3.424	1.120	.083	77
78	.248	.238	-.004	.248	.234	.007	-70.748	1.088	.022	78

SSTIPN: 1 LAYER OF GEOTEXTILE, P=2000 PPI, T=0

TOTAL NUMBER OF NODES----- 98
NUMBER OF BAR ELEMENTS----- 9
NUMBER OF DIFF. BAR MATERIALS----- 1
NUMBER OF BEAM ELEMENTS----- 0
NUMBER OF DIFF. BEAM MATERIALS----- 0
NUMBER OF MODAL LINKS----- 0
NUMBER OF INTERFACE ELEMENTS----- 0
NO OF INTERFACE ELE. IN PREEXIST PART 0
NUMBER OF INTERPACK ELEM IN FOUNDATION 0
NUMBER OF INTERFACE MATERIALS----- 0
TOTAL NUMBER OF SOIL ELEMENTS----- 78
NUMBER OF DIFF. SOIL MATERIALS----- 3
NUMBER OF ELEMENTS IN FOUNDATION----- 38
NUMBER OF NODES IN FOUNDATION----- 52
NUMBER OF PREEXISTING ELEMENTS----- 0
NUMBER OF PREEXISTING NODES----- 0
NUMBER OF CONSTRUCTION LAYERS----- 4
NUMBER OF LOAD CASES----- 1

CALING FACTOR ----- 1.00000
ATMOSPHERIC PRESSURE --- 1.05800
UNIT WEIGHT OF WATER --- 0.0120

COMPUTATION SEQUENCE FOR A TOTAL OF 7 INCREMENTS

INCREMENT NO. 1 PUT ON LAYER NO. 1
INCREMENT NO. 2 PUT ON LAYER NO. 2
INCREMENT NO. 3 PUT ON LAYER NO. 3
INCREMENT NO. 4 PUT ON LAYER NO. 4
INCREMENT NO. 5 PUT ON LAYER NO. 5
INCREMENT NO. 6 PUT ON LAYER NO. 6
INCREMENT NO. 7 APPLY LOAD CASE 1

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		S.C. CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	0.000	0.000	1	1	1
2	10.000	0.000	1	1	1
3	20.000	0.000	1	1	1
4	30.000	0.000	1	1	1
5	38.000	0.000	1	1	1
6	42.000	0.000	1	1	1
7	50.000	0.000	1	1	1
8	54.000	0.000	1	1	1
9	58.000	0.000	1	1	1
10	68.000	0.000	1	1	1
11	74.000	0.000	1	1	1
12	82.000	0.000	1	1	1
13	90.000	0.000	1	1	1
14	10.000	4.000	0	0	0
15	20.000	4.000	0	0	0
16	30.000	4.000	0	0	0
17	40.000	4.000	0	0	0
18	48.000	4.000	0	0	0
19	52.000	4.000	0	0	0
20	56.000	4.000	0	0	0
21	64.000	4.000	0	0	0
22	68.000	4.000	0	0	0
23	74.000	4.000	0	0	0
24	82.000	4.000	0	0	0
25	90.000	4.000	0	0	0
26	10.000	7.000	0	0	0
27	20.000	7.000	0	0	0
28	30.000	7.000	0	0	0
29	38.000	7.000	0	0	0
30	42.000	7.000	0	0	0
31	50.000	7.000	0	0	0
32	54.000	7.000	0	0	0
33	58.000	7.000	0	0	0
34	68.000	7.000	0	0	0
35	74.000	7.000	0	0	0
36	82.000	7.000	0	0	0
37	90.000	7.000	0	0	0
38	10.000	10.000	1	0	1
39	20.000	10.000	1	0	1
40	30.000	10.000	0	0	0
41	40.000	10.000	0	0	0
42	48.000	10.000	0	0	0
43	52.000	10.000	0	0	0
44	56.000	10.000	0	0	0
45	64.000	10.000	0	0	0
46	68.000	10.000	0	0	0
47	74.000	10.000	0	0	0
48	82.000	10.000	0	0	0
49	90.000	10.000	0	0	0
50	10.000	13.000	0	0	0
51	20.000	13.000	0	0	0
52	30.000	13.000	1	0	1
53	38.000	13.000	0	0	0
54	42.000	13.000	0	0	0
55	50.000	13.000	0	0	0
56	54.000	13.000	0	0	0
57	58.000	13.000	0	0	0
58	68.000	13.000	0	0	0
59	74.000	13.000	0	0	0
60	82.000	13.000	0	0	0
61	90.000	13.000	1	0	1
62	10.000	16.000	0	0	0
63	20.000	16.000	0	0	0
64	30.000	16.000	0	0	0
65	38.000	16.000	0	0	0
66	42.000	16.000	0	0	0
67	50.000	16.000	0	0	0
68	54.000	16.000	0	0	0
69	58.000	16.000	0	0	0
70	68.000	16.000	0	0	0
71	74.000	16.000	0	0	0
72	82.000	16.000	0	0	0
73	90.000	16.000	0	0	0
74	10.000	19.000	0	0	0
75	20.000	19.000	0	0	0
76	30.000	19.000	1	0	1
77	38.000	19.000	0	0	0
78	42.000	19.000	0	0	0
79	50.000	19.000	0	0	0
80	54.000	19.000	0	0	0
81	58.000	19.000	0	0	0
82	68.000	19.000	0	0	0
83	74.000	19.000	0	0	0
84	82.000	19.000	0	0	0
85	90.000	19.000	1	0	1
86	10.000	22.000	0	0	0
87	20.000	22.000	0	0	0
88	30.000	22.000	0	0	0
89	38.000	22.000	0	0	0
90	42.000	22.000	0	0	0
91	50.000	22.000	0	0	0
92	54.000	22.000	0	0	0
93	58.000	22.000	0	0	0
94	68.000	22.000	0	0	0
95	74.000	22.000	0	0	0
96	82.000	22.000	0	0	0
97	90.000	22.000	1	0	1

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	80.	1.00	0.

ELMY NO.	CONNECTED NODES I J	MATL NO.
1	43 44	1
2	44 45	1
3	45 46	1
4	46 47	1
5	47 48	1
6	48 49	1
7	49 50	1
8	50 51	1
9	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL UNIT	WY	YOUNG'S MODULUS CONSTANT	EXPOONENT	ATIG	BULK MODULUS CONSTANT	EXPOONENT	STRENGTH C	PARAMETERS PH	DPHT	KB
1	.0830	8000.00	.500	.500	1500.00	.500	.00	35.00	00	.50
2	.0830	10.00	.300	.300	20.00	.200	.05	00	00	.50
3	.0800	1000.00	.400	.700	500.00	.500	.50	40.00	.00	.50

POOR NODES SOLID ELEMENT DATA

ELET NO.	CONNECTED NODES I J K L	MATL NO.	ELEMENT CENTER COORDINATES X-ORD Y-ORD
1	1 2 15 14	3	5.000 2.000
2	2 3 15 15	3	15.000 2.000
3	3 4 17 15	3	25.000 2.000
4	4 5 17 16	2	35.000 2.000
5	5 6 18 16	2	45.000 2.000
6	6 7 20 16	2	55.000 2.000
7	7 8 21 20	2	65.000 2.000
8	8 9 22 21	2	75.000 2.000
9	9 10 23 22	3	85.000 2.000
10	10 11 24 23	3	95.000 2.000
11	11 12 25 24	3	105.000 2.000
12	12 13 24 25	3	115.000 2.000
13	14 15 26 27	3	125.000 5.500
14	15 16 28 28	3	135.000 5.500
15	16 17 30 29	2	145.000 5.500
16	17 18 31 30	2	155.000 5.500
17	18 19 32 31	2	165.000 5.500
18	19 20 33 32	2	175.000 5.500
19	20 21 34 33	2	185.000 5.500
20	21 22 35 34	2	195.000 5.500
21	22 23 36 35	2	205.000 5.500
22	23 24 37 36	3	215.000 5.500
23	24 25 38 37	3	225.000 5.500
24	25 26 39 38	3	235.000 5.500
25	26 27 41 40	3	245.000 5.500
26	28 29 42 41	2	255.000 8.500
27	29 30 43 42	2	265.000 8.500
28	30 31 44 43	2	275.000 8.500
29	31 32 45 44	2	285.000 8.500
30	32 33 46 45	2	295.000 8.500
31	33 34 47 46	2	305.000 8.500
32	34 35 48 47	2	315.000 8.500
33	35 36 49 48	2	325.000 8.500
34	36 37 50 49	2	335.000 8.500
35	37 38 51 50	2	345.000 8.500
36	38 39 52 51	3	355.000 8.500
37	43 44 53 53	1	365.000 10.750
38	44 45 54 54	1	375.000 10.750
39	45 46 55 54	1	385.000 10.750
40	46 47 56 55	1	395.000 10.750
41	47 48 57 56	1	405.000 10.750
42	48 49 58 57	1	415.000 10.750
43	49 50 59 58	1	425.000 10.750
44	50 51 60 59	1	435.000 10.750
45	51 52 61 60	1	445.000 10.750
46	52 53 62 62	1	455.000 12.250
47	54 55 63 62	1	465.000 12.250
48	55 56 64 63	1	475.000 12.250
49	56 57 65 64	1	485.000 12.250
50	57 58 66 65	1	495.000 12.250
51	58 59 67 66	1	505.000 12.250
52	59 60 68 67	1	515.000 12.250
53	60 61 69 68	1	525.000 12.250
54	62 63 71 70	1	535.000 13.500
55	63 64 72 71	1	545.000 13.500
56	64 65 73 72	1	555.000 13.500
57	65 66 74 73	1	565.000 13.500
58	66 67 75 74	1	575.000 13.500
59	67 68 76 75	1	585.000 13.500
60	68 69 77 76	1	595.000 13.500
61	70 71 78 76	1	605.000 14.500
62	71 72 79 76	1	615.000 14.500
63	72 73 80 79	1	625.000 14.500
64	73 74 81 80	1	635.000 14.500
65	74 75 82 81	1	645.000 14.500
66	75 76 83 82	1	655.000 14.500
67	76 77 84 83	1	665.000 14.500
68	77 78 85 85	1	675.000 15.500
69	78 79 86 85	1	685.000 15.500
70	79 80 87 85	1	695.000 15.500
71	80 81 88 85	1	705.000 15.500
72	82 83 89 88	1	715.000 15.500
73	83 84 90 89	1	725.000 15.500
74	85 86 91 91	1	735.000 16.500
75	86 87 92 91	1	745.000 16.500
76	87 88 93 91	1	755.000 16.500
77	88 89 94 92	1	765.000 16.500
78	89 90 95 94	1	775.000 16.500

STTIPN: 1 LAYER OF GEOTEXTILF, P=2000 PPI, T=0

```
*****
*****
*****
LOAD CASE ----- 1
*****
*****
```

LARGEST ELE NO. IN THIS INCREMENT 78
LARGEST N P NO IN THIS INCREMENT 86

SAND WIDTH----- 40
TOTAL NUMBER OF EQUATIONS----- 222
NUMBER OF EQUATIONS IN BLOCK----- 28
NUMBER OF BLOCKS----- 3
NUMBER OF N P FORCE CARDS----- 3
NUMBER OF PRESSURE CARDS----- 0

NOODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP X-FORCE Y-FORCE

1 0. 0.
 2 0. 0.
 3 0. 0.
 4 0. 0.
 5 0. 0.
 6 0. 0.
 7 0. 0.
 8 0. 0.
 9 0. 0.
 10 0. 0.
 11 0. 0.
 12 0. 0.
 13 0. 0.
 14 0. 0.
 15 0. 0.
 16 0. 0.
 17 0. 0.
 18 0. 0.
 19 0. 0.
 20 0. 0.
 21 0. 0.
 22 0. 0.
 23 0. 0.
 24 0. 0.
 25 0. 0.
 26 0. 0.
 27 0. 0.
 28 0. 0.
 29 0. 0.
 30 0. 0.
 31 0. 0.
 32 0. 0.
 33 0. 0.
 34 0. 0.
 35 0. 0.
 36 0. 0.
 37 0. 0.
 38 0. 0.
 39 0. 0.
 40 0. 0.
 41 0. 0.
 42 0. 0.
 43 0. 0.
 44 0. 0.
 45 0. 0.
 46 0. 0.
 47 0. 0.
 48 0. 0.
 49 0. 0.
 50 0. 0.
 51 0. 0.
 52 0. 0.
 53 0. 0.
 54 0. 0.
 55 0. 0.
 56 0. 0.
 57 0. 0.
 58 0. 0.
 59 0. 0.
 60 0. 0.
 61 0. 0.
 62 0. 0.

63 0. 0.
 64 0. 0.
 65 0. 0.
 66 0. 0.
 67 0. 0.
 68 0. 0.
 69 0. 0.
 70 0. 0.
 71 0. 0.
 72 0. 0.
 73 0. 0.
 74 0. 0.
 75 0. 0.
 76 0. 0.
 77 0. 0.
 78 0. 0.
 79 0. 0.
 80 0. 0.
 81 0. 0.
 82 0. 0.
 83 0. 0.
 84 0. 0.
 85 0. 0.
 86 0. 0.
 87 0. 0.
 88 0. 0.
 89 0. 0.
 90 0. 0.
 91 0. 0.
 92 0. -4.50
 93 0. -4.50
 94 0. -2.00
 95 0. 0.

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0000	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0000	.0000	.0000	.0000	15
16	.0000	.0000	.0000	.0000	.0000	.0000	.0000	16
17	.0004	.0022	.0000	.0048	.0071	.0000	.0082	17
18	.0222	.0128	.0000	.2274	.0571	.0000	.2344	18
19	.0483	.0039	.0000	.3418	.0661	.0000	.3479	19
20	.0494	.0078	.0000	.2138	.1340	.0000	.2564	20
21	.0318	.0237	.0000	.0838	.1809	.0000	.1882	21
22	.0011	.0012	.0000	.0011	.0081	.0000	.0082	22
23	.0010	.0018	.0000	.0003	.0080	.0000	.0080	23
24	.0007	.0088	.0000	.0012	.0104	.0000	.0108	24
25	.0020	.0021	.0000	.0021	.0088	.0000	.0089	25
26	.0000	.0000	.0000	.0000	.0000	.0000	.0000	26
27	.0000	.0000	.0000	.0000	.0000	.0000	.0000	27
28	.0000	.0000	.0000	.0000	.0000	.0000	.0000	28
29	.0000	.0000	.0000	.0000	.0000	.0000	.0000	29
30	.0131	.0048	.0000	.2378	.0838	.0000	.2486	30
31	.0378	.0284	.0000	.3829	.1128	.0000	.3706	31
32	.0678	.0148	.0000	.4888	.1100	.0000	.4882	32
33	.1024	.0090	.0000	.3940	.2848	.0000	.4748	33
34	.1149	.0318	.0000	.3377	.2787	.0000	.4386	34
35	.1011	.0488	.0000	.2380	.2028	.0000	.3103	35
36	.0003	.0048	.0000	.0000	.0138	.0000	.0139	36
37	.0018	.0108	.0000	.0000	.0182	.0000	.0182	37
38	.0022	.0039	.0000	.0018	.0108	.0000	.0108	38
39	.0007	.0018	.0000	.0000	.0048	.0000	.0048	39
40	.0000	.0000	.0000	.0000	.0000	.0000	.0000	40
41	.0000	.0000	.0000	.0000	.0000	.0000	.0000	41
42	.0138	.0088	.0000	.1400	.0808	.0000	.1833	42
43	.0421	.0102	.0000	.3120	.0817	.0000	.3282	43
44	.0488	.0247	.0000	.3187	.1222	.0000	.3474	44
45	.0488	.0071	.0000	.2814	.1828	.0000	.3487	45
46	.0800	.0380	.0000	.2283	.3742	.0000	.4272	46
47	.0488	.0702	.0000	.1828	.3378	.0000	.3840	47
48	.0438	.0484	.0000	.0731	.2284	.0000	.2408	48
49	.0014	.0071	.0000	.0087	.0488	.0000	.0480	49
50	.0001	.0038	.0000	.0014	.0128	.0000	.0127	50
51	.0000	.0028	.0000	.0000	.0078	.0000	.0078	51
52	.0438	.0347	.0000	.3767	.1880	.0000	.3822	52
53	.0432	.0212	.0000	.4442	.1374	.0000	.5034	53
54	.0413	.0071	.0000	.2778	.3438	.0000	.4420	54
55	.0383	.0388	.0000	.2108	.3810	.0000	.4442	55
56	.0318	.0731	.0000	.1841	.3871	.0000	.4021	56
57	.0148	.0812	.0000	.0871	.2872	.0000	.2788	57
58	.0007	.0307	.0000	.0388	.0874	.0000	.1048	58
59	.0031	.0308	.0000	.0044	.0117	.0000	.0128	59

60	.0000	.0028	.0000	.0000	.0088	.0000	.0088	60
61	.0400	.0212	.0000	.0888	.0082	.0000	.0882	61
62	.0207	.0071	.0000	.1487	.1487	.0000	.1715	62
63	.0288	.0078	.0000	.2074	.2074	.0000	.2282	63
64	.0233	.0708	.0000	.0788	.2488	.0000	.2823	64
65	.0220	.0828	.0000	.0880	.1811	.0000	.2619	65
66	.0143	.0328	.0000	.0321	.1270	.0000	.1304	66
67	.0084	.0044	.0000	.0082	.0107	.0000	.0135	67
68	.0030	.0028	.0000	.0000	.0040	.0000	.0040	68
69	.4884	.0178	.0000	.5248	.784	.0000	.8288	69
70	.4118	.0081	.0000	.4840	.0882	.0000	.6666	70
71	.8148	.0488	.0000	.8887	.1838	.0000	.8787	71
72	.5000	.1283	.0000	.3838	.2802	.0000	.7088	72
73	.0308	.0488	.0000	.0708	.1703	.0000	.1841	73
74	.0181	.0338	.0000	.0374	.1382	.0000	.1733	74
75	.0081	.0048	.0000	.0102	.0180	.0000	.0188	75
76	.0000	.0020	.0000	.0000	.0030	.0000	.0030	76
77	.8881	.0087	.0000	.8208	.0471	.0000	.8228	77
78	.8833	.0488	.0000	.8214	.1182	.0000	.8221	78
79	.8008	.1282	.0000	.8248	.2008	.0000	.8864	79
80	.8333	.2878	.0000	.8680	.3074	.0000	1.0082	80
81	.8031	.2007	.0000	.8184	.3438	.0000	.9438	81
82	.8888	.0587	.0000	.8880	.0404	.0000	.8921	82
83	.0000	.0823	.0000	.0000	.0730	.0000	.0780	83
84	.8338	.0840	.0000	.8120	.0731	.0000	.8482	84
85	.8910	.0818	.0000	.7008	.1088	.0000	.7088	85
86	.8873	.3843	.0000	.8871	.4187	.0000	.8731	86
87	.8812	.3708	.0000	.8887	.8028	.0000	.8888	87
88	.2731	.1378	.0000	.2720	.1400	.0000	.3080	88
89	.0000	.2038	.0000	.0000	.2011	.0000	.2017	89
90	.8800	.0816	.0000	.8800	.0818	.0000	.8881	90
91	.8818	.3881	.0000	.8818	.3881	.0000	.7822	91
92	.8888	.8784	.0000	.8888	.8784	.0000	.8888	92
93	.1127	.3188	.0000	.1127	.3188	.0000	.3383	93
94	.0000	.3078	.0000	.0000	.3078	.0000	.3078	94

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO	AXIAL FORCE
INCREMENTAL VALUES	
1	.0438
2	.0031
3	.0121
4	.0222
5	.0488
6	.1728
7	.1848
8	.0083
9	.0010
TOTAL VALUES	
1	.0878
2	.1372
3	.3275
4	.5258
5	.8884
6	.8208
7	.5084

FOUR HOLE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHRER MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	528.3	330.5	227.7	155	.001	.000	.002	.002	-.001	.002	1
2	523.8	333.0	225.4	181	.001	.007	.010	.010	-.002	.011	2
3	524.2	330.1	225.8	187	.022	-.008	.052	.035	-.032	.087	3
4	.8	17.2	.2	.484	1.855	-.739	2.444	2.340	-1.224	3.564	4
5	.8	17.1	.2	.484	.882	.114	8.138	4.824	-3.558	8.182	5
6	.7	17.8	.2	.484	-.766	2.501	7.427	4.824	-3.181	8.118	6
7	.7	18.4	.2	.483	-1.542	3.888	4.240	4.438	-2.312	8.747	7
8	.8	18.8	.2	.484	-1.287	2.088	-.781	2.131	-1.198	3.330	8
9	487.3	338.8	188.8	224	-.008	.139	.011	.140	-.008	.146	9
10	471.8	362.2	180.8	237	-.010	.183	.022	.184	-.010	.204	10
11	475.8	370.8	191.4	242	-.009	.197	-.073	.208	-.012	.217	11
12	510.2	362.8	210.8	208	.013	.173	-.044	.108	.008	.089	12
13	438.7	287.2	184.7	122	.001	.002	-.001	.002	.001	.002	13
14	488.2	273.3	203.7	114	.010	-.008	.018	.014	-.013	.028	14
15	2.1	13.8	.7	.475	1.271	-1.038	3.971	2.180	-2.025	4.214	15
16	4.4	18.0	1.8	.483	2.818	-1.884	8.101	3.922	-3.081	8.883	16
17	1.8	15.8	.5	.484	1.727	-.188	8.713	4.258	-2.725	8.983	17
18	.8	17.1	.2	.484	-1.082	2.814	8.081	4.840	-2.718	7.252	18
19	.8	17.1	.2	.484	-2.265	4.112	7.474	8.828	-3.889	8.828	19
20	.7	18.2	.2	.484	-2.441	8.204	8.108	8.978	-3.218	8.183	20
21	.8	17.2	.2	.484	-1.478	3.423	2.885	3.788	-1.825	5.584	21
22	420.1	341.3	187.4	238	.017	.264	.037	.288	-.018	.313	22
23	402.0	307.5	182.3	231	-.008	.231	-.083	.240	-.018	.284	23
24	483.0	307.4	192.2	191	.025	-.115	-.082	.122	.018	.103	24
25	408.8	228.8	188.8	083	-.001	.030	.001	.001	-.001	.002	25
26	.0	.1	.0	.488	.733	-.839	2.124	1.288	-1.374	2.643	26
27	1.5	13.5	.8	.481	2.018	-1.310	3.103	2.827	-1.822	4.549	27
28	2.8	13.8	.9	.488	1.017	-.825	.007	1.017	-.825	1.843	28
29	1.3	14.1	.4	.485	.880	.888	1.234	1.228	-.010	1.238	29
30	.8	18.0	.2	.484	-.887	2.538	-2.278	2.728	-.888	3.713	30
31	.8	18.8	.2	.484	-1.162	3.248	-3.773	3.847	-1.841	5.788	31
32	.8	18.8	.2	.484	-1.831	3.878	-4.114	4.843	-2.485	7.038	32
33	.7	17.8	.2	.484	-2.187	8.845	-1.503	8.918	-2.227	8.162	33
34	.7	17.7	.2	.484	-.425	4.032	-.282	4.038	-.432	4.488	34
35	278.8	246.0	108.2	280	-.027	-.475	-.078	.477	-.028	.507	35
36	344.8	218.9	108.8	187	.003	.083	-.013	.088	-.003	.083	36
37	.1	0.0	.0	.485	.138	.254	18.788	8.588	-.8.203	18.788	37
38	1218.7	819.3	818.3	.000	.431	2.067	20.124	10.748	-9.580	20.124	38
39	408.2	448.5	182.7	330	-1.548	3.930	10.272	7.012	-4.830	11.641	39
40	280.8	428.8	48.8	288	-1.288	4.744	1.870	4.808	-1.414	8.318	40
41	118.0	412.2	40.7	481	-1.101	8.200	-.878	8.218	-1.119	8.338	41
42	11.8	388.8	4.0	.488	-1.244	4.282	-1.348	4.282	-1.328	8.887	42
43	11.8	388.8	4.0	.488	-1.080	3.877	-3.803	4.282	-1.888	8.841	43
44	884.8	883.4	280.2	.277	.244	1.888	-2.102	2.382	-.273	2.688	44
45	1174.8	883.7	823.1	123	-.037	.083	.031	.088	-.039	.104	45
46	188.3	420.8	88.4	.421	-.038	.043	.187	.108	-.104	.213	46
47	.1	4.0	.0	.488	-.110	.049	.463	.218	-.278	.480	47
48	231.0	428.1	82.3	.403	-.128	.184	.188	.182	-.184	.338	48
49	11.8	388.8	4.0	.488	-.323	.288	.331	.338	-.384	.700	49
50	388.8	443.1	133.3	.380	-.388	.374	-.147	.381	-.376	.788	50
51	817.7	838.8	242.8	.274	-.388	2.918	.091	2.913	-.388	3.288	51
52	748.0	887.5	280.9	.282	-.218	2.789	-.281	2.775	-.222	2.987	52
53	843.8	888.7	280.0	.288	-.078	.077	.082	.082	-.083	.188	53
54	.1	4.0	.0	.488	.843	-1.170	48.883	24.840	-28.087	50.007	54
55	.1	4.0	.0	.488	.020	.008	88.487	28.743	-28.713	88.487	55
56	.1	4.0	.0	.488	-.088	3.208	80.814	31.847	-28.787	80.704	56
57	.1	4.0	.0	.488	-3.801	2.808	30.113	18.817	-18.912	30.824	57
58	808.8	834.1	328.2	.274	-.388	2.883	.880	2.808	-.424	3.332	58
59	1410.7	811.3	838.4	110	-.280	2.734	-.813	2.788	-.302	3.088	59
60	438.0	483.8	188.0	.318	-.117	.084	.082	.088	-.121	.220	60
61	.1	4.0	.0	.488	3.208	-.084	-.877	3.230	.088	3.318	61

62	438.4	462.8	181.4	.318	.061	-.022	-.028	.064	-.025	.078	62
63	289.7	429.7	93.3	.381	.076	-.028	-.272	1.172	-.121	.293	63
64	.1	4.0	.0	.488	-1.820	8.188	44.807	28.202	-18.882	48.884	64
65	.1	4.0	.0	.488	-6.032	21.834	44.878	34.283	-18.361	52.824	65
66	.1	4.0	.0	.488	-3.819	16.365	33.019	25.589	-13.153	38.752	66
67	.1	4.0	.0	.488	3.882	-1.370	-31.184	12.841	-14.888	31.589	67
68	.1	4.0	.0	.488	.001	.438	4.884	2.311	-2.223	4.884	68
69	.1	4.0	.0	.488	.748	-1.945	7.842	3.483	-4.880	8.103	69
70	.1	4.0	.0	.488	2.053	4.886	-5.017	6.201	.836	5.086	70
71	.1	4.0	.0	.488	6.038	22.882	21.902	26.384	-9.738	28.083	71
72	.1	4.0	.0	.488	8.314	19.881	40.886	31.972	-21.726	33.884	72
73	.1	4.0	.0	.488	5.424	-1.874	13.184	9.284	-5.834	15.118	73
74	.1	4.0	.0	.488	1.429	-.038	-3.387	2.347	-1.183	3.688	74
75	1032.5	888.2	472.4	.087	-.012	.024	.082	.088	-.0-1	.088	75
76	380.7	443.2	133.7	.348	-.201	.188	-.323	.318	-.281	.571	76
77	.1	4.0	.0	.488	-8.170	9.129	14.182	11.855	-11.588	23.182	77
78	.1	4.0	.0	.488	1.002	3.702	13.281	9.118	-4.748	13.833	78

FOUP NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	YHEVA	SIG1/SIG3	LEVEL	ELE
1	.248	.483	.006	.483	-.248	.119	1.280	1.870	.078	1
2	.283	.520	.022	.522	-.282	.138	4.888	2.078	.088	2
3	.348	.580	.124	.644	-.343	.188	20.166	-2.031	.083	3
4	.366	.538	.008	.528	-.366	.088	1.888	1.484	1.688	4
5	.383	.528	.017	.531	-.382	.080	5.371	1.810	1.783	5
6	.481	.684	.018	.688	-.488	.088	4.724	1.427	1.881	6
7	.527	.730	.010	.730	-.527	.102	2.708	1.388	2.038	7
8	.322	.518	-.001	.518	-.322	.087	4.422	1.601	1.832	8
9	.324	1.088	.018	1.070	-.324	.373	1.178	3.301	.228	9
10	.400	1.381	.042	1.383	-.400	.482	2.441	3.471	.278	10
11	.438	1.426	.142	1.448	-.438	.514	-7.882	3.481	.282	11
12	.374	.918	-.082	.934	-.378	-.288	-9.283	2.851	1.87	12
13	.140	.280	-.002	.280	-.140	.070	-1.814	2.000	.083	13
14	.178	.238	.038	.253	-.178	.048	28.981	1.820	.038	14
15	.142	.182	-.018	.178	-.142	.048	11.827	1.248	.888	15
16	.280	.282	.033	.284	-.280	.033	44.248	1.287	.888	16
17	.328	.402	.018	.407	-.328	.041	13.702	1.282	.820	17
18	.382	.487	.012	.488	-.381	.084	8.881	1.287	1.073	18
19	.382	.474	.018	.478	-.380	.088	7.438	1.328	1.113	19
20	.521	.644	.012	.648	-.520	.082	8.483	1.240	1.247	20
21	.381	.481	.008	.482	-.381	.088	2.488	1.282	1.111	21
22	.384	1.578	.081	1.578	-.381	.886	2.828	4.143	.341	22
23	.314	1.208	-.184	1.231	-.318	.472	-8.842	4.277	.287	23
24	.300	.883	-.088	.707	-.278	.218	-13.734	2.884	.137	24
25	.042	.081	-.003	.081	-.042	.028	2.948	2.188	.022	25
26	.001	.008	.003	.003	-.007	.008	88.308	-3.78	-1.000	26
27	.082	-.011	-.008	.083	-.010	.041	25.880	8.118	.828	27
28	.100	.028	.008	.100	-.028	.038	88.883	4.042	.782	28
29	.133	.214	-.007	.214	-.132	.041	4.781	1.822	.822	29
30	.198	.288	-.007	.288	-.188	.082	-3.838	1.833	1.040	30
31	.283	.388	-.010	.388	-.282	.083	-8.233	1.420	1.081	31
32	.241	.348	-.018	.351	-.239	.088	-8.472	1.488	1.120	32
33	.823	.840	-.002	.841	-.823	.088	-1.184	1.228	1.180	33
34	.817	.818	.008	.818	-.817	.088	2.878	1.180	.884	34
35	.422	1.181	-.010	1.188	-.422	.834	-3.334	4.038	.348	35
36	.088	.337	-.017	.338	-.088	.142	-3.387	6.188	.121	36
37	.078	.071	-.008	.128	-.010	.088	-48.848	14.828	8.028	37
38	.088	.124	-.008	.128	-.087	.018	-12.811	1.441	1.684	38
39	.017	.283	-.031	.287	-.013	-.122	7.388	-8.787	8.347	39
40	-.024	.310	.108	.341	-.054	-.187	16.083	-6.337	-1.000	40
41	-.111	.488	-.014	.488	-.111	.288	-1.388	-4.177	-1.000	41
42	-.011	.414	-.068	.424	-.022	.228	-8.883	-18.888	-1.000	42
43	.287	1.802	-.122	1.808	-.288	.318	-12.388	4.188	1.188	43
44	.288	1.403	-.088	1.408	-.282	.572	-2.782	5.188	1.818	44
45	-.187	.381	.128	.380	-.188	.288	13.018	-2.092	-1.000	45
46	-.028	.081	.021	.071	-.018	-.028	28.287	3.828	1.081	46
47	-.087	.123	-.114	.208	-.078	-.118	38.883	-7.283	-1.000	47
48	-.080	.418	.133	.481	-.123	.287	13.848	-3.882	-1.000	48
49	-.187	.043	-.108	.048	-.181	-.22.848	-2.418	-1.000	49	
50	-.377	.480	-.181	.810	-.408	.488	10.288	-1.288	-1.000	50
51	-.478	.884	-.847	1.182	-.733	.842	-21.884	-1.871	-1.000	51
52	-.270	.888	.228	.888	-.310	.882	10.073	-3.214	-1.000	52
53	-.332	.344	.120	.384	-.382	.388	8.788	-1.038	-1.000	53
54	.038	.088	-.114	.180	-.070	-.118	42.404	-2.288	-1.000	54
55	.080	.208	-.129	.288	-.001	-.143	31.848	-482.010	-1.000	55
56	.178	.344	.027	.348	-.171	.088	8.711	2.037	.388	56
57	-.008	1.184	-.082	1.188	-.043	.118	-22.481	-4.343	-1.000	57
58	-.871	1.004	-.302	1.081	-.818	.888	-8.838	-1.148	-1.000	58
59	-1.888	.823	-.172	.838	-1.801	1.218	4.088	-1.822	-1.000	59
60	-.418	.242	.108	.288	-.428	.348	8.888	-1.888	-1.000	60
61	.128	.011	.011	.138	-.112	.011	81.408	1.208	.078	61
62	.308	.048	.002	.008	-.048	.128	88.811	8.887	2.088	62
63	.483	-.073	.317	.888	-.100	.377	81.473	-8.881	-1.000	63
64	.381	.480	-.088	.808	.308	.100	-31.788	1.882	.242	64
65	-.701	-.778	-.013	-.778	.688	.038	8.448	1.111	.041	65
66	-.808	.538	-.013	.843	-.800	.021	-18.218	1.088	.208	66
67	-.200	.308	-.008	.308	-.188	.088	-4.078	1.881	.081	67
68	.040	.038	-.002	.041	.038	.003	-84.808	1.140	.082	68
69	-.028	.024	-.084	.078	-.028	.084	88.088	-2.888	-1.000	69
70	.321	.388	-.020	.381	.318	.038	-18.302	1.242	.080	70
71	.888	.708	-.008	.710	.888	-.022	11.828	1.088	.024	71
72	.414	.484	.018	.481	.407	.027	20.713	1.133	.048	72
73	.218	.283	-.008	.284	-.217	.018	8.180	1.188	.083	73
74	.088	.072	.001	.072	.088	.004	8.481	1.118	.043	74
75	-.088	.271	.432	.860	-.378	.488	23.811	-1.483	-1.000	75
76	-.311	.884	.432	1.087	-.443	.770	-17.088	-2.474	-1.000	76
77	.008	.040	.004	.041	-.007	.017	8.847	8.733	1.780	77
78	.203	.223	.008	.224	-.202	.011	14.480	1.108	.040	78

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		S.C. CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	000	000	1	1	1
2	10.000	0.000	1	1	1
3	20.000	0.000	1	1	1
4	30.000	0.000	1	1	1
5	38.000	0.000	1	1	1
6	42.000	0.000	1	1	1
7	50.000	0.000	1	1	1
8	54.000	0.000	1	1	1
9	58.000	0.000	1	1	1
10	68.000	0.000	1	1	1
11	74.000	0.000	1	1	1
12	82.000	0.000	1	1	1
13	80.000	7.000	0	0	0
14	10.000	4.000	0	0	0
15	20.000	4.000	0	0	0
16	30.000	4.000	0	0	0
17	38.000	4.000	0	0	0
18	42.000	4.000	0	0	0
19	50.000	4.000	0	0	0
20	54.000	4.000	0	0	0
21	58.000	4.000	0	0	0
22	68.000	4.000	0	0	0
23	74.000	4.000	0	0	0
24	82.000	4.000	0	0	0
25	80.000	7.000	1	0	1
26	10.000	7.000	0	0	0
27	20.000	7.000	0	0	0
28	30.000	7.000	0	0	0
29	38.000	7.000	0	0	0
30	42.000	7.000	0	0	0
31	50.000	7.000	0	0	0
32	54.000	7.000	0	0	0
33	58.000	7.000	0	0	0
34	68.000	7.000	0	0	0
35	74.000	7.000	0	0	0
36	82.000	7.000	0	0	0
37	80.000	10.000	1	0	1
38	10.000	10.000	0	0	0
39	20.000	10.000	0	0	0
40	30.000	10.000	0	0	0
41	38.000	10.000	0	0	0
42	42.000	10.000	0	0	0
43	50.000	10.000	0	0	0
44	54.000	10.000	0	0	0
45	58.000	10.000	0	0	0
46	68.000	10.000	0	0	0
47	74.000	10.000	0	0	0
48	82.000	10.000	0	0	0
49	80.000	13.000	0	0	0
50	10.000	13.000	0	0	0
51	20.000	13.000	0	0	0
52	30.000	13.000	0	0	0
53	38.000	13.000	0	0	0
54	42.000	13.000	0	0	0
55	50.000	13.000	0	0	0
56	54.000	13.000	0	0	0
57	58.000	13.000	0	0	0
58	68.000	13.000	0	0	0
59	74.000	13.000	0	0	0
60	82.000	13.000	0	0	0
61	80.000	14.000	1	0	1
62	10.000	14.000	0	0	0
63	20.000	14.000	0	0	0
64	30.000	14.000	0	0	0
65	38.000	14.000	0	0	0
66	42.000	14.000	0	0	0
67	50.000	14.000	0	0	0
68	54.000	14.000	0	0	0
69	58.000	14.000	0	0	0
70	68.000	14.000	0	0	0
71	74.000	14.000	0	0	0
72	82.000	14.000	0	0	0
73	80.000	17.000	1	0	1
74	10.000	17.000	0	0	0
75	20.000	17.000	0	0	0
76	30.000	17.000	0	0	0
77	38.000	17.000	0	0	0
78	42.000	17.000	0	0	0
79	50.000	17.000	0	0	0
80	54.000	17.000	0	0	0
81	58.000	17.000	0	0	0
82	68.000	17.000	0	0	0
83	74.000	17.000	0	0	0
84	82.000	17.000	0	0	0
85	80.000	19.000	1	0	1
86	10.000	19.000	0	0	0
87	20.000	19.000	0	0	0
88	30.000	19.000	0	0	0
89	38.000	19.000	0	0	0
90	42.000	19.000	0	0	0
91	50.000	19.000	0	0	0
92	54.000	19.000	0	0	0
93	58.000	19.000	0	0	0
94	68.000	19.000	0	0	0
95	74.000	19.000	0	0	0
96	82.000	19.000	0	0	0
97	80.000	22.000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	60	1.00	0

ELMY NO.	CONNECTED NODES I	J	MATL NO.
1	43	44	1
2	44	45	1
3	45	46	1
4	46	47	1
5	47	48	1
6	48	49	1
7	49	50	1
8	50	51	1
9	51	52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WY	YOUNG'S MODULUS CONSTANT EXPONENT	RATIO	BULK MODULUS CONSTANT EXPONENT	STRENGTH C	PARAMETERS PHI	DPHI	K6
1	.0680	6000.00	.500	1500.00	.600	.00	35.00	.00 .50
2	.0530	40.00	.300	800	20.00	.200	.05	.00 .50
3	.0600	1000.00	.400	.700	500.00	.500	.50	40.00 .00 .50

FOUR NODES SOLYS ELEMENT DATA

ELET NO	CONNECTED NODES I	J	K	L	MATL NO	ELEMENT CENTER COORDINATES X-ORD	Y-ORD
1	1	2	15	14	3	5.000	2.000
2	2	3	15	15	3	15.000	2.000
3	3	4	17	18	3	25.000	2.000
4	4	5	17	19	2	25.000	2.000
5	5	6	18	18	2	35.000	2.000
6	6	7	20	19	2	45.000	2.000
7	7	8	21	20	2	52.000	2.000
8	8	9	22	21	2	58.000	2.000
9	9	10	23	22	3	62.000	2.000
10	10	11	24	23	3	70.000	2.000
11	11	12	25	24	3	78.000	2.000
12	12	13	26	25	3	88.000	2.000
13	14	15	28	27	3	5.000	5.500
14	15	16	29	28	3	15.000	5.500
15	16	17	30	29	2	25.000	5.500
16	17	18	31	30	2	33.000	5.500
17	18	19	32	31	2	39.000	5.500
18	19	20	33	32	2	46.000	5.500
19	20	21	34	33	2	52.000	5.500
20	21	22	35	34	2	58.000	5.500
21	22	23	36	35	2	62.000	5.500
22	23	24	37	36	3	70.000	5.500
23	24	25	38	37	3	78.000	5.500
24	25	26	39	38	3	88.000	5.500
25	27	28	41	40	3	5.000	8.500
26	28	29	42	41	2	15.000	8.500
27	29	30	43	42	2	25.000	8.500
28	30	31	44	43	2	33.000	8.500
29	31	32	45	44	2	39.000	8.500
30	32	33	46	45	2	46.000	8.500
31	33	34	47	46	2	52.000	8.500
32	34	35	48	47	2	58.000	8.500
33	35	36	49	48	2	62.000	8.500
34	36	37	50	49	2	70.000	8.500
35	37	38	51	50	3	78.000	8.500
36	38	39	52	51	3	88.000	8.500
37	40	41	53	52	1	24.500	10.750
38	41	42	54	53	1	34.500	10.750
39	42	43	55	54	1	44.500	10.750
40	43	44	56	55	1	54.500	10.750
41	44	45	57	56	1	64.500	10.750
42	45	46	58	57	1	74.500	10.750
43	46	47	59	58	1	84.500	10.750
44	47	48	60	59	1	94.500	10.750
45	48	49	61	60	1	104.500	10.750
46	50	51	62	61	1	114.500	12.250
47	51	52	63	62	1	124.500	12.250
48	52	53	64	63	1	134.500	12.250
49	53	54	65	64	1	144.500	12.250
50	54	55	66	65	1	154.500	12.250
51	55	56	67	66	1	164.500	12.250
52	56	57	68	67	1	174.500	12.250
53	57	58	69	68	1	184.500	12.250
54	58	59	70	69	1	194.500	12.250
55	59	60	71	70	1	204.500	12.250
56	60	61	72	71	1	214.500	12.250
57	61	62	73	72	1	224.500	12.250
58	62	63	74	73	1	234.500	12.250
59	63	64	75	74	1	244.500	12.250
60	64	65	76	75	1	254.500	12.250
61	65	66	77	76	1	264.500	12.250
62	66	67	78	77	1	274.500	12.250
63	67	68	79	78	1	284.500	12.250
64	68	69	80	79	1	294.500	12.250
65	69	70	81	80	1	304.500	12.250
66	70	71	82	81	1	314.500	12.250
67	71	72	83	82	1	324.500	12.250
68	72	73	84	83	1	334.500	12.250
69	73	74	85	84	1	344.500	12.250
70	74	75	86	85	1	354.500	12.250
71	75	76	87	86	1	364.500	12.250
72	76	77	88	87	1	374.500	12.250
73	77	78	89	88	1	384.500	12.250
74	78	79	90	89	1	394.500	12.250
75	79	80	91	90	1	404.500	12.250
76	80	81	92	91	1	414.500	12.250
77	81	82	93	92	1	424.500	12.250
78	82	83	94	93	1	434.500	12.250
79	83	84	95	94	1	444.500	12.250
80	84	85	96	95	1	454.500	12.250

CONSTRUCTION LAYER INFORMATION

SSSTIPN 1 LAYER OF GEOTEXTILE, P=2000 PFI, T=1000

```
*****  
*****  
* LOAD CASE ----- 2 *  
*****  
*****
```

LARGEST ELE NO IN THIS INCREMENT 78

LARGEST N P NO IN THIS INCREMENT 95

BAND WIDTH----- 40

TOTAL NUMBER OF EQUATIONS----- 222

NUMBER OF EQUATIONS IN BLOCK----- 64

NUMBER OF BLOCKS----- 3

NUMBER OF NIP FORCE CARDS----- 3

NUMBER OF PRESSURE CARDS----- 0

NUCLEAR POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP X-FORCE Y-FORCE

NP	X-FORCE	Y-FORCE
1	0.	0.
2	0.	0.
3	0.	0.
4	0.	0.
5	0.	0.
6	0.	0.
7	0.	0.
8	0.	0.
9	0.	0.
10	0.	0.
11	0.	0.
12	0.	0.
13	0.	0.
14	0.	0.
15	0.	0.
16	0.	0.
17	0.	0.
18	0.	0.
19	0.	0.
20	0.	0.
21	0.	0.
22	0.	0.
23	0.	0.
24	0.	0.
25	0.	0.
26	0.	0.
27	0.	0.
28	0.	0.
29	0.	0.
30	0.	0.
31	0.	0.
32	0.	0.
33	0.	0.
34	0.	0.
35	0.	0.
36	0.	0.
37	0.	0.
38	0.	0.
39	0.	0.
40	0.	0.
41	0.	0.
42	0.	0.
43	0.	0.
44	0.	0.
45	0.	0.
46	0.	0.
47	0.	0.
48	0.	0.
49	0.	0.
50	0.	0.
51	0.	0.
52	0.	0.
53	0.	0.
54	0.	0.
55	0.	0.
56	0.	0.
57	0.	0.
58	0.	0.
59	0.	0.
60	0.	0.
61	0.	0.
62	0.	0.
63	0.	0.
64	0.	0.
65	0.	0.
66	0.	0.
67	0.	0.
68	0.	0.
69	0.	0.
70	0.	0.
71	0.	0.
72	0.	0.
73	0.	0.
74	0.	0.
75	0.	0.
76	0.	0.
77	0.	0.
78	0.	0.
79	0.	0.
80	0.	0.
81	0.	0.
82	0.	0.
83	0.	0.
84	0.	0.
85	0.	0.
86	0.	0.
87	0.	0.
88	0.	0.
89	0.	0.
90	0.	0.
91	0.	0.
92	0.	-4.50
93	0.	-4.50
94	0.	-2.00
95	0.	0.

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0000	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0000	.0001	.0000	.0003	15
16	.0000	.0001	.0000	.0007	.0007	.0000	.0010	16
17	-.0008	.0001	.0000	-.0058	.0027	.0000	.0052	17
18	-.0128	.0010	.0000	-.2357	.0501	.0000	.2408	18
19	-.0085	-.0034	.0000	-.3348	-.0724	.0000	.3428	19
20	-.0080	.0002	.0000	-.1882	.1388	.0000	.2328	20
21	-.0081	.0001	.0000	-.3741	.1118	.0000	.1868	21
22	.0002	.0010	.0000	.0002	.0040	.0000	.0040	22
23	-.0013	-.0028	.0000	-.0008	-.0081	.0000	.0082	23
24	.0008	-.0087	.0000	.0008	-.0107	.0000	.0107	24
25	.0021	-.0020	.0000	.0021	-.0043	.0000	.0058	25
26	.0000	.0000	.0000	.0000	-.0018	.0000	.0018	26
27	.0000	.0000	.0000	.0000	.0002	.0000	.0002	27
28	.0000	.0000	.0000	.0000	-.0004	.0000	.0004	28
29	-.0051	.0000	.0000	-.0028	.0002	.0000	.0028	29
30	-.0293	.0088	.0000	-.3311	.0874	.0000	.3424	30
31	-.0478	.0034	.0000	-.4886	.0805	.0000	.4884	31
32	-.0518	-.0084	.0000	-.4888	-.1380	.0000	.5066	32
33	-.0470	-.0082	.0000	-.4718	-.1780	.0000	.4848	33
34	-.0587	.0128	.0000	-.3028	-.2441	.0000	.2890	34
35	-.0783	-.0103	.0000	-.2281	-.1888	.0000	.2312	35
36	-.0030	.0010	.0000	-.0017	.0188	.0000	.0188	36
37	.0018	-.0174	.0000	.0014	-.0184	.0000	.0184	37
38	.0028	.0037	.0000	.0013	-.0108	.0000	.0110	38
39	.0000	.0000	.0000	.0000	-.0027	.0000	.0027	39
40	.0000	.0000	.0000	.0000	.0002	.0000	.0002	40
41	.0000	.0000	.0000	.0002	.0004	.0000	.0004	41
42	-.0227	.0082	.0000	-.1811	.0812	.0000	.1804	42
43	-.0828	.0141	.0000	-.4323	.1513	.0000	.4580	43
44	-.0534	.0033	.0000	-.4281	.0728	.0000	.4323	44
45	-.0838	.0088	.0000	-.3881	.1882	.0000	.4447	45
46	-.0817	-.0112	.0000	-.3373	-.3883	.0000	.5161	46
47	-.0483	.0183	.0000	-.2881	-.3407	.0000	.4448	47
48	-.0508	.0124	.0000	-.2388	-.2880	.0000	.3582	48
49	-.0347	-.1188	.0000	-.1188	-.2824	.0000	.2820	49
50	-.0005	-.0288	.0000	-.0280	.0482	.0000	.0530	50
51	-.0001	.0030	.0000	.0133	-.0188	.0000	.0207	51
52	.0000	.0028	.0000	.0000	.0782	.0000	.0782	52
53	-.0503	.0038	.0000	-.3881	.1318	.0000	.8038	53
54	-.0514	-.0081	.0000	-.5048	-.1888	.0000	.5284	54
55	-.0581	-.0118	.0000	-.3888	-.3870	.0000	.4788	55
56	-.0583	.0177	.0000	-.2878	-.3838	.0000	.4378	56
57	-.0338	-.0108	.0000	-.1782	-.2807	.0000	.3384	57
58	-.0048	-.1188	.0000	-.0888	-.2884	.0000	.3041	58
59	-.0048	-.0308	.0000	-.0488	-.0873	.0000	.1078	59
60	-.0028	-.0028	.0000	-.0104	-.0124	.0000	.0182	60
61	.0000	-.0028	.0000	.0000	.0737	.0000	.0737	61
62	-.0488	-.0088	.0000	-.1288	-.0241	.0000	.1291	62
63	-.0781	-.0118	.0000	-.1534	-.1870	.0000	.2288	63
64	-.0847	.0184	.0000	-.1884	.1782	.0000	.2378	64
65	-.1083	.0272	.0000	-.1782	-.1782	.0000	.2807	65
66	-.0228	-.1218	.0000	-.0728	-.2238	.0000	.2384	66
67	-.0148	-.0328	.0000	-.0788	-.1200	.0000	.1434	67
68	-.0080	-.0030	.0000	-.0183	-.0383	.0000	.0384	68
69	-.0000	.0027	.0000	.0000	-.0282	.0000	.0282	69
70	-.1083	-.0087	.0000	-.1828	-.0881	.0000	.1788	70
71	-.1187	.0117	.0000	-.1777	-.1180	.0000	.2133	71
72	-.1282	.0218	.0000	-.1841	-.1182	.0000	.2188	72
73	-.1387	.0318	.0000	-.1888	-.1188	.0000	.2300	73
74	-.0383	-.1231	.0000	-.0878	-.2038	.0000	.2220	74
75	-.0218	-.0333	.0000	-.0488	-.1388	.0000	.1487	75
76	-.0080	-.0034	.0000	-.0088	-.0088	.0000	.0122	76
77	.0000	.0023	.0000	.0000	-.0028	.0000	.0028	77
78	-.1288	.0108	.0000	-.1808	-.0737	.0000	.1788	78
79	-.1307	.0218	.0000	-.1837	-.0828	.0000	.1983	79
80	-.1401	.0323	.0000	-.1748	-.0888	.0000	.1888	80
81	-.8142	-.3121	.0000	-.8817	-.3888	.0000	.7477	81
82	-.8887	-.2828	.0000	-.8218	-.3812	.0000	.6288	82
83	1.0717	.0438	.0000	1.0702	-.0477	.0000	1.0712	83
84	.0000	.1178	.0000	.0000	.1143	.0000	.1143	84
85	-.1410	.0222	.0000	-.1528	.0004	.0000	.1528	85
86	-.7023	.1848	.0000	-.7184	.1874	.0000	.7328	86
87	-.7041	.8883	.0000	-.7178	-.8887	.0000	.9328	87
88	.5744	-.8788	.0000	.5303	-.8328	.0000	.8284	88
89	1.4881	-.0887	.0000	1.4773	-.0878	.0000	1.4788	89
90	.0000	.3838	.0000	.0000	.3481	.0000	.3481	90
91	-.8081	-.1872	.0000	-.8081	.1872	.0000	.8382	91
92	-.8083	-.8730	.0000	-.8083	-.8730	.0000	.8384	92
93	-.3807	-.8831	.0000	-.3807	-.8831	.0000	1.0878	93
94	1.8448	-.0887	.0000	1.8448	-.0887	.0000	1.8470	94
95	.0000	.8818	.0000	.0000	.8818	.0000	.8818	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO. AXIAL FORCE

INCREMENTAL VALUES

1	.0081
2	.0038
3	.0158
4	.0807
5	.0883
6	.1218
7	.2581
8	.0038
9	.0008

TOTAL VALUES

1	.0818
2	.2700
3	.4638
4	.7887
5	.8983
6	.1071
7	.8981

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	528.7	332.8	228.1	158	.001	.001	.004	.003	-.001	.004	1
2	528.8	338.2	228.4	184	.002	-.010	-.018	.014	-.003	.017	2
3	498.3	314.8	218.1	158	.025	-.025	-.082	.040	-.039	.078	3
4	7	17.3	2	484	1.917	-.858	2.821	2.487	-1.208	3.876	4
5	6	17.0	2	484	.827	-.278	8.153	4.839	-3.532	8.171	5
6	7	17.8	2	484	.611	2.416	8.884	4.781	-3.046	7.787	6
7	7	18.4	2	483	-1.482	3.480	3.324	3.867	-1.970	5.866	7
8	6	18.8	2	484	-.802	1.818	-.818	1.878	-.864	2.842	8
9	482.7	342.8	188.0	224	.008	.128	.022	.127	.008	.122	9
10	477.0	387.8	182.8	238	-.011	-.310	-.028	-.211	-.012	-.223	10
11	474.7	371.4	181.0	243	-.007	-.202	-.070	-.208	-.013	-.220	11
12	500.8	382.8	207.8	208	.012	.081	-.048	.088	.008	.082	12
13	442.8	281.0	187.2	122	.062	.003	-.001	.003	.001	.001	13
14	480.9	271.2	208.8	121	.018	-.010	-.028	.021	-.017	.038	14
15	2.2	14.0	.7	473	1.887	-1.427	8.004	3.082	-2.822	5.884	15
16	2.8	18.7	.9	470	2.888	-2.088	8.738	5.502	-4.888	10.102	16
17	8	18.8	3	481	1.088	.387	8.174	5.320	-3.878	8.188	17
18	8	17.1	2	484	-1.830	3.428	8.878	5.187	-3.388	8.238	18
19	8	17.0	2	484	-2.328	4.078	8.813	5.443	-3.681	8.134	19
20	7	18.2	2	484	-1.878	4.483	8.837	5.302	-2.724	8.028	20
21	8	17.1	2	484	-1.380	2.818	2.832	3.340	-1.814	5.184	21
22	432.8	381.7	172.8	288	.030	-.318	-.082	-.320	-.032	-.312	22
23	408.8	308.8	168.8	232	-.008	.238	-.081	.241	-.013	.288	23
24	412.8	278.1	173.7	188	.021	.104	-.082	.114	.011	.103	24
25	408.8	228.7	187.1	093	-.001	.000	.002	.001	-.001	.002	25
26	0	1	0	488	-.818	-1.348	2.227	1.280	-1.819	3.708	26
27	4	13.4	1	484	2.888	-2.418	3.541	2.528	-2.943	6.488	27
28	2.3	13.8	8	471	1.018	-.771	1.787	1.378	-1.131	2.508	28
29	12.9	18.0	4.8	380	.003	1.287	2.107	1.888	-.888	2.472	29
30	8	18.0	2	484	-1.108	2.842	-.088	2.842	-1.108	3.848	30
31	8	18.7	2	484	-1.801	3.480	-1.800	3.828	-1.877	5.303	31
32	8	18.8	2	484	-1.881	3.238	-1.813	3.420	-1.738	5.188	32
33	7	17.8	2	484	-2.183	8.723	1.224	8.770	-2.200	7.970	33
34	7	17.3	2	484	-.888	4.348	1.077	4.801	-.888	5.288	34
35	310.8	278.8	121.2	280	-.078	.931	-.887	.807	-.188	.782	35
36	380.8	220.0	188.1	141	-.078	1.342	.881	1.400	-.132	1.522	36
37	11.8	288.8	4.0	488	.088	.241	17.481	8.892	-8.888	17.481	37
38	283.8	434.2	104.7	377	-1.803	2.038	18.211	10.674	-8.438	20.112	38
39	438.8	483.8	188.7	317	-1.821	3.888	10.441	7.088	-4.711	11.788	39
40	189.3	422.1	70.3	417	-1.141	4.821	2.058	4.894	-1.314	6.307	40
41	177.8	418.4	82.2	428	-1.488	8.288	-.488	8.277	-1.508	6.743	41
42	242.4	427.8	88.8	388	-1.384	4.282	-.838	4.323	-1.388	5.717	42
43	11.8	388.8	4.0	488	-1.130	3.733	-4.848	4.788	-2.182	6.868	43
44	884.8	814.8	271.8	279	.207	1.883	-2.870	2.707	-.817	3.322	44
45	1482.0	874.8	888.7	119	.048	.005	.320	.187	-.138	.322	45
46	120.8	412.8	41.7	480	.048	.048	.043	.061	-.063	.124	46
47	1	4.0	0	488	.048	.077	1.118	.870	-.447	1.117	47
48	88.7	408.2	22.8	472	-.184	4.000	4.418	5.211	.043	4.478	48
49	1	4.0	0	488	-.437	.878	2.788	1.844	-1.402	2.848	49
50	1	4.0	0	488	-1.077	.481	4.488	2.087	-2.873	4.780	50
51	801.7	483.4	184.4	280	-.083	2.782	-1.702	2.887	-.328	3.318	51
52	837.8	888.1	250.1	278	-.488	3.438	1.788	3.828	-.848	4.272	52
53	848.1	847.8	383.4	188	-.123	1.808	.294	1.820	-.134	1.884	53
54	1	4.0	0	488	.344	-.234	3.811	2.477	-2.388	4.848	54
55	11.8	388.8	4.0	488	.132	-.143	3.848	1.825	-1.838	3.840	55
56	11.8	388.8	4.0	488	.388	-.318	2.873	1.808	-1.738	3.848	56
57	1	4.0	0	488	-1.348	0.030	4.033	1.472	-2.788	4.281	57
58	1324.2	778.7	880.0	121	-.488	2.814	1.188	3.010	-.882	3.832	58
59	1473.0	828.8	873.8	084	-.382	2.788	-.381	2.788	-.382	3.127	59
60	448.8	488.8	171.8	312	-.108	.087	.040	.088	-.110	.208	60
61	1	4.0	0	488	.384	-.081	1.421	.887	-.883	1.480	61

82	222.1	424.9	78.9	.407	104	-.045	.058	.109	-.051	.180	82
83	241.2	427.4	86.2	.388	266	-.271	.256	.286	-.253	.549	83
84	.1	4.0	.0	.485	2.301	8.287	32.233	22.281	-10.892	32.883	84
85	.1	4.0	.0	.485	-7.522	23.631	.987	23.839	-7.530	31.188	85
86	.1	4.0	.0	.485	-3.574	14.172	-83.980	49.424	-38.826	86.250	86
87	.1	4.0	.0	.485	8.830	-3.718	-55.023	29.450	-28.838	55.888	87
88	180.8	421.0	87.0	.420	.082	-.081	.242	.132	-.131	.283	88
89	.1	4.0	.0	.485	7.143	-7.443	28.448	14.951	-15.251	30.201	89
70	.1	4.0	.0	.485	2.875	5.447	38.488	23.884	-15.572	38.588	70
71	.1	4.0	.0	.485	-15.049	28.787	4.581	29.804	-15.188	48.070	71
72	.1	4.0	.0	.485	-3.283	19.031	-25.158	23.812	-14.084	37.878	72
73	.1	4.0	.0	.485	15.927	-9.346	-24.035	20.728	-14.148	34.877	73
74	.1	4.0	.0	.485	14.033	-.288	-13.489	15.713	-2.838	19.649	74
75	522.3	488.8	203.8	.282	.018	.087	.068	.058	.019	.038	75
76	.1	4.0	.0	.485	-14.240	26.341	7.072	20.839	-14.888	35.237	76
77	.1	4.0	.0	.485	-12.910	21.188	-2.848	21.208	-12.958	34.183	77
78	.1	4.0	.0	.485	18.842	-13.414	-10.228	18.731	-14.203	33.834	78

FOUR NODE SOLID ELEMNTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THETA	SIG1/SIG3	LEVEL	ELE
1	.248	486	.008	486	.248	.119	2.282	1.985	.078	1
2	.281	538	.034	541	.258	.142	7.015	2.110	.093	2
3	.388	386	.142	502	.218	.142	43.834	2.301	.087	3
4	.390	.559	.005	560	.390	.085	7.985	1.435	1.837	4
5	.388	.537	.017	538	.358	.080	6.287	1.804	1.805	5
6	.485	.850	.015	651	.454	.088	4.347	1.433	1.888	6
7	.508	.708	.007	.710	.508	.101	2.094	1.387	2.017	7
8	.320	.811	-.002	511	.320	.085	-.470	1.597	1.910	8
9	.388	1.021	.038	1.023	.383	.330	3.327	2.822	1.92	9
10	.423	1.883	.084	1.488	.420	.533	2.918	3.528	.291	10
11	.438	1.439	-.136	1.487	.417	.520	27.500	3.480	.285	11
12	.383	.850	-.100	.870	.358	.288	-10.880	2.803	1.80	12
13	.148	.283	-.002	.283	.148	.088	-.918	1.862	.052	13
14	.187	.231	.058	.275	.188	.080	36.824	1.788	.045	14
15	.138	.187	.028	.208	.138	.039	20.714	1.588	.772	15
16	.280	.282	.042	.311	.221	.048	34.378	1.403	.892	16
17	.308	.398	.018	.400	.304	.048	12.018	1.318	.958	17
18	.388	.468	.014	.468	.384	.058	7.198	1.318	1.128	18
19	.318	.483	-.013	.483	.344	.088	8.428	1.341	1.171	19
20	.481	.808	.011	.807	.488	.088	5.588	1.241	1.180	20
21	.310	.418	.006	.418	.310	.084	3.128	1.350	1.085	21
22	.383	1.714	.080	1.720	.357	.881	3.814	4.817	.397	22
23	.318	1.228	-.138	1.248	.298	.478	-8.218	4.217	.287	23
24	.287	.807	-.107	.827	.227	.205	-15.730	2.801	1.38	24
25	.042	.081	.003	.081	.042	.025	3.488	2.171	.021	25
26	.008	.003	.007	.012	.008	.008	85.131	-1.872	-1.000	26
27	.181	.023	.040	.182	.012	.078	73.933	1.881	1.604	27
28	.098	.025	-.003	.098	.025	.037	-87.804	4.008	.738	28
29	.188	.198	.028	.213	.151	.031	28.089	1.408	.818	29
30	.188	.309	-.004	.309	.188	.082	-1.702	1.867	1.237	30
31	.227	.332	-.008	.333	.227	.083	-3.233	1.488	1.052	31
32	.183	.282	-.013	.283	.182	.088	-6.482	1.518	1.118	32
33	.800	.818	.001	.819	.800	.059	-.888	1.237	1.187	33
34	.878	.878	.005	.878	.878	.051	2.888	1.176	1.010	34
35	.438	.774	.041	.774	.438	.847	-1.428	3.488	.334	35
36	.088	.228	-.008	.228	.088	.088	-3.138	4.085	.074	36
37	.033	.080	-.083	.102	-.008	.085	-37.852	-12.858	-1.000	37
38	.082	.119	.044	.143	.038	.053	28.524	3.803	1.042	38
39	.113	.303	-.128	.388	-.047	.180	28.880	7.803	2.828	39
40	-.128	-.340	-.188	-.384	-.183	-.288	-17.805	-2.154	-1.000	40
41	-.828	-.150	-.180	-.188	-.882	-.374	-12.657	-.331	-1.000	41
42	-.887	-.857	.071	-.881	-.871	-.818	3.281	-.837	-1.000	42
43	.108	.708	-.182	.750	-.087	.342	-14.187	11.240	3.807	43
44	.374	1.827	-.135	1.843	.388	.882	-6.582	4.300	1.227	44
45	.030	.288	.384	.583	-.247	.408	35.784	-2.274	-1.000	45
46	-.028	.016	.008	-.032	.012	.010	64.438	2.832	.807	46
47	-.024	.181	.107	.208	-.033	.120	28.134	-8.380	-1.000	47
48	.188	.323	.287	.582	-.061	.307	37.738	-9.085	-1.000	48
49	-.008	.287	-.073	.313	-.022	.188	-12.808	-14.004	-1.000	49
50	-.019	.210	-.084	.227	-.038	.131	-14.508	-6.387	-1.000	50
51	-.019	1.170	-.877	1.410	-.220	.815	-22.548	-6.387	1.000	51
52	-.208	.874	.281	.843	-.277	.810	13.883	-3.397	-1.000	52
53	-.402	.350	.188	.388	-.438	.402	12.780	-.834	-1.000	53
54	.038	.088	.118	.187	-.088	.118	40.880	-2.581	-1.000	54
55	-.088	.083	.178	.198	-.184	.191	33.288	-1.077	-1.000	55
56	.088	.483	.111	.521	.080	.231	14.342	8.740	2.877	56
57	-.074	.115	-.083	.133	-.083	.113	-18.749	-1.433	-1.000	57
58	-1.734	1.111	-.778	1.308	-1.822	1.821	-14.204	-.878	-1.000	58
59	-2.112	.701	.821	.832	-2.243	1.537	11.807	-.371	-1.000	59
60	.342	.274	.081	.280	-.348	.314	5.800	-.804	-1.000	60
61	.021	.028	.007	.031	-.018	.008	38.488	1.988	.388	61
62	.438	.159	.134	.480	.104	.183	88.008	4.889	1.371	62
63	.772	-.273	.388	.882	-.383	.832	72.868	-2.303	-1.000	63
64	.688	.882	-.113	.737	.484	.122	-33.839	1.483	1.83	64
65	.847	.888	-.000	.888	.847	.021	828	1.085	.024	65
66	.811	.844	-.033	.885	.480	.027	-31.887	1.182	.088	66
67	-.238	-.318	-.021	-.323	-.230	.046	-13.808	1.400	1.48	67
68	.081	-.048	.182	.216	-.174	.185	55.482	-1.248	-1.000	68
69	.074	-.048	.088	.148	-.027	.087	48.687	-8.387	-1.000	69
70	.383	.451	-.023	.487	-.388	.050	-13.880	1.278	.103	70
71	.887	.839	.002	.838	.887	.026	1.981	1.080	.034	71
72	.384	.434	-.010	.438	.382	.022	-13.438	1.113	.042	72
73	.288	.288	-.010	.288	.278	.010	-48.887	1.070	.028	73
74	.883	.881	-.002	.888	.888	.004	-58.780	1.013	.005	74
75	.284	.488	.017	.488	.282	.088	5.470	1.800	.223	75
76	.248	.281	.003	.281	.248	.022	3.805	1.182	.088	76
77	.332	.378	-.001	.378	.332	.022	-1.317	1.122	.048	77
78	.250	.241	-.004	.282	.240	.008	-88.583	1.050	.019	78

SSTIPN. 1 LAYER OF GEOTEXTILE, P=2000 PPI, EXTENDED, T=1000

TOTAL NUMBER OF NODES-----	95
NUMBER OF BAR ELEMENTS-----	11
NUMBER OF DIFF BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO OF INTERFACE ELE IN PREEXIST PART-----	0
NUMBER OF INTERFACE ELE IN FOUNDATION-----	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	38
NUMBER OF PREEXISTING FOUNDATION-----	52
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	1
NUMBER OF LOAD CASES-----	2

SCALING FACTOR ----- 1.00000

ATMOSPHERIC PRESSURE --- 1.05800

UNIT WEIGHT OF WATER --- .03120

COMPUTATION SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO. 1	APPLY LOAD CASE	1
INCREMENT NO. 2	PUT ON LAYER NO	1
INCREMENT NO. 3	PUT ON LAYER NO	2
INCREMENT NO. 4	PUT ON LAYER NO	3
INCREMENT NO. 5	PUT ON LAYER NO	4
INCREMENT NO. 6	PUT ON LAYER NO	5
INCREMENT NO. 7	PUT ON LAYER NO	6
INCREMENT NO. 8	APPLY LOAD CASE	2

MODAL POINT INPUT DATA

NODE NUMBER	MODAL POINT COORDINATES		S.C CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	000	000	1	1	1
2	10.000	000	1	1	1
3	20.000	000	1	1	1
4	30.000	000	1	1	1
5	40.000	000	1	1	1
6	50.000	000	1	1	1
7	60.000	000	1	1	1
8	70.000	000	1	1	1
9	80.000	000	1	1	1
10	90.000	000	1	1	1
11	74.000	.000	1	1	1
12	82.000	.000	1	1	1
13	90.000	.000	1	1	1
14	10.000	4.000	0	0	0
15	20.000	4.000	0	0	0
16	30.000	4.000	0	0	0
17	40.000	4.000	0	0	0
18	50.000	4.000	0	0	0
19	60.000	4.000	0	0	0
20	70.000	4.000	0	0	0
21	80.000	4.000	0	0	0
22	90.000	4.000	0	0	0
23	74.000	4.000	0	0	0
24	82.000	4.000	0	0	0
25	90.000	4.000	0	0	0
26	10.000	7.000	1	0	1
27	20.000	7.000	1	0	1
28	30.000	7.000	0	0	0
29	40.000	7.000	0	0	0
30	50.000	7.000	0	0	0
31	60.000	7.000	0	0	0
32	70.000	7.000	0	0	0
33	80.000	7.000	0	0	0
34	90.000	7.000	0	0	0
35	74.000	7.000	0	0	0
36	82.000	7.000	0	0	0
37	90.000	7.000	0	0	0
38	10.000	10.000	1	0	1
39	20.000	10.000	1	0	1
40	30.000	10.000	0	0	0
41	40.000	10.000	0	0	0
42	50.000	10.000	0	0	0
43	60.000	10.000	0	0	0
44	70.000	10.000	0	0	0
45	80.000	10.000	0	0	0
46	90.000	10.000	0	0	0
47	74.000	10.000	0	0	0
48	82.000	10.000	0	0	0
49	90.000	10.000	0	0	0
50	10.000	13.000	0	0	0
51	20.000	13.000	0	0	0
52	30.000	13.000	1	0	1
53	40.000	13.000	0	0	0
54	50.000	13.000	0	0	0
55	60.000	13.000	0	0	0
56	70.000	13.000	0	0	0
57	80.000	13.000	0	0	0
58	90.000	13.000	0	0	0
59	74.000	13.000	0	0	0
60	82.000	13.000	0	0	0
61	90.000	13.000	1	0	1
62	42.000	14.000	0	0	0
63	50.000	14.000	0	0	0
64	60.000	14.000	0	0	0
65	70.000	14.000	0	0	0
66	80.000	14.000	0	0	0
67	90.000	14.000	0	0	0
68	74.000	14.000	0	0	0
69	82.000	14.000	0	0	0
70	90.000	14.000	1	0	1
71	10.000	17.000	0	0	0
72	20.000	17.000	0	0	0
73	30.000	17.000	1	0	1
74	40.000	17.000	0	0	0
75	50.000	17.000	0	0	0
76	60.000	17.000	0	0	0
77	70.000	17.000	0	0	0
78	80.000	17.000	0	0	0
79	90.000	17.000	0	0	0
80	74.000	17.000	0	0	0
81	82.000	17.000	0	0	0
82	90.000	17.000	1	0	1
83	10.000	20.000	0	0	0
84	20.000	20.000	0	0	0
85	30.000	20.000	1	0	1
86	40.000	20.000	0	0	0
87	50.000	20.000	0	0	0
88	60.000	20.000	0	0	0
89	70.000	20.000	0	0	0
90	80.000	20.000	0	0	0
91	90.000	20.000	0	0	0
92	74.000	20.000	0	0	0
93	82.000	20.000	0	0	0
94	90.000	20.000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	86	1.00	0

ELEM NO.	CONNECTED NODES I J	MATL NO
1	41 42	1
2	42 43	1
3	43 44	1
4	44 45	1
5	45 46	1
6	46 47	1
7	47 48	1
8	48 49	1
9	49 50	1
10	50 51	1
11	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S MODULUS CONSTANT	MODULUS EXPONENT	RATIO	BULK MODULUS CONSTANT	MODULUS EXPONENT	STRENGTH C	PARAMETERS PHI	DPHI	K0
1	0880	8000.00	800	800	1500.00	.800	00	38.00	00	50
2	0530	40.00	300	900	20.00	200	.08	00	00	.50
3	0800	1000.00	400	700	500.00	500	.50	40.00	00	50

FOUR NODES SOLID ELEMENT DATA

ELEM NO	CONNECTED NODES I J K L	MATL NO	ELEMENT CENTER COORDINATES X-ORD Y-ORD
1	1 2 3 4	2	8.000 2.000
2	2 3 4 5	2	15.000 2.000
3	3 4 5 6	2	22.000 2.000
4	4 5 6 7	2	29.000 2.000
5	5 6 7 8	2	36.000 2.000
6	6 7 8 9	2	43.000 2.000
7	7 8 9 10	2	50.000 2.000
8	8 9 10 11	2	57.000 2.000
9	9 10 11 12	2	64.000 2.000
10	10 11 12 13	2	71.000 2.000
11	11 12 13 14	2	78.000 2.000
12	12 13 14 15	2	85.000 2.000
13	13 14 15 16	2	92.000 2.000
14	14 15 16 17	2	99.000 2.000
15	15 16 17 18	2	106.000 2.000
16	16 17 18 19	2	113.000 2.000
17	17 18 19 20	2	120.000 2.000
18	18 19 20 21	2	127.000 2.000
19	19 20 21 22	2	134.000 2.000
20	20 21 22 23	2	141.000 2.000
21	21 22 23 24	2	148.000 2.000
22	22 23 24 25	2	155.000 2.000
23	23 24 25 26	2	162.000 2.000
24	24 25 26 27	2	169.000 2.000
25	25 26 27 28	2	176.000 2.000
26	26 27 28 29	2	183.000 2.000
27	27 28 29 30	2	190.000 2.000
28	28 29 30 31	2	197.000 2.000
29	29 30 31 32	2	204.000 2.000
30	30 31 32 33	2	211.000 2.000
31	31 32 33 34	2	218.000 2.000
32	32 33 34 35	2	225.000 2.000
33	33 34 35 36	2	232.000 2.000
34	34 35 36 37	2	239.000 2.000
35	35 36 37 38	2	246.000 2.000
36	36 37 38 39	2	253.000 2.000
37	37 38 39 40	2	260.000 2.000
38	38 39 40 41	2	267.000 2.000
39	39 40 41 42	2	274.000 2.000
40	40 41 42 43	2	281.000 2.000
41	41 42 43 44	2	288.000 2.000
42	42 43 44 45	2	295.000 2.000
43	43 44 45 46	2	302.000 2.000
44	44 45 46 47	2	309.000 2.000
45	45 46 47 48	2	316.000 2.000
46	46 47 48 49	2	323.000 2.000
47	47 48 49 50	2	330.000 2.000
48	48 49 50 51	2	337.000 2.000
49	49 50 51 52	2	344.000 2.000
50	50 51 52 53	2	351.000 2.000
51	51 52 53 54	2	358.000 2.000
52	52 53 54 55	2	365.000 2.000
53	53 54 55 56	2	372.000 2.000
54	54 55 56 57	2	379.000 2.000
55	55 56 57 58	2	386.000 2.000
56	56 57 58 59	2	393.000 2.000
57	57 58 59 60	2	400.000 2.000
58	58 59 60 61	2	407.000 2.000
59	59 60 61 62	2	414.000 2.000
60	60 61 62 63	2	421.000 2.000
61	61 62 63 64	2	428.000 2.000
62	62 63 64 65	2	435.000 2.000
63	63 64 65 66	2	442.000 2.000
64	64 65 66 67	2	449.000 2.000
65	65 66 67 68	2	456.000 2.000
66	66 67 68 69	2	463.000 2.000
67	67 68 69 70	2	470.000 2.000
68	68 69 70 71	2	477.000 2.000
69	69 70 71 72	2	484.000 2.000
70	70 71 72 73	2	491.000 2.000
71	71 72 73 74	2	498.000 2.000
72	72 73 74 75	2	505.000 2.000
73	73 74 75 76	2	512.000 2.000
74	74 75 76 77	2	519.000 2.000
75	75 76 77 78	2	526.000 2.000
76	76 77 78 79	2	533.000 2.000
77	77 78 79 80	2	540.000 2.000
78	78 79 80 81	2	547.000 2.000
79	79 80 81 82	2	554.000 2.000
80	80 81 82 83	2	561.000 2.000
81	81 82 83 84	2	568.000 2.000
82	82 83 84 85	2	575.000 2.000
83	83 84 85 86	2	582.000 2.000
84	84 85 86 87	2	589.000 2.000
85	85 86 87 88	2	596.000 2.000
86	86 87 88 89	2	603.000 2.000
87	87 88 89 90	2	610.000 2.000
88	88 89 90 91	2	617.000 2.000
89	89 90 91 92	2	624.000 2.000
90	90 91 92 93	2	631.000 2.000
91	91 92 93 94	2	638.000 2.000
92	92 93 94 95	2	645.000 2.000
93	93 94 95 96	2	652.000 2.000
94	94 95 96 97	2	659.000 2.000
95	95 96 97 98	2	666.000 2.000
96	96 97 98 99	2	673.000 2.000
97	97 98 99 100	2	680.000 2.000
98	98 99 100 101	2	687.000 2.000
99	99 100 101 102	2	694.000 2.000
100	100 101 102 103	2	701.000 2.000
101	101 102 103 104	2	708.000 2.000
102	102 103 104 105	2	715.000 2.000
103	103 104 105 106	2	722.000 2.000
104	104 105 106 107	2	729.000 2.000
105	105 106 107 108	2	736.000 2.000
106	106 107 108 109	2	743.000 2.000
107	107 108 109 110	2	750.000 2.000
108	108 109 110 111	2	757.000 2.000
109	109 110 111 112	2	764.000 2.000
110	110 111 112 113	2	771.000 2.000
111	111 112 113 114	2	778.000 2.000
112	112 113 114 115	2	785.000 2.000
113	113 114 115 116	2	792.000 2.000
114	114 115 116 117	2	799.000 2.000
115	115 116 117 118	2	806.000 2.000
116	116 117 118 119	2	813.000 2.000
117	117 118 119 120	2	820.000 2.000
118	118 119 120 121	2	827.000 2.000
119	119 120 121 122	2	834.000 2.000
120	120 121 122 123	2	841.000 2.000
121	121 122 123 124	2	848.000 2.000
122	122 123 124 125	2	855.000 2.000
123	123 124 125 126	2	862.000 2.000
124	124 125 126 127	2	869.000 2.000
125	125 126 127 128	2	876.000 2.000
126	126 127 128 129	2	883.000 2.000
127	127 128 129 130	2	890.000 2.000
128	128 129 130 131	2	897.000 2.000
129	129 130 131 132	2	904.000 2.000
130	130 131 132 133	2	911.000 2.000
131	131 132 133 134	2	918.000 2.000
132	132 133 134 135	2	925.000 2.000
133	133 134 135 136	2	932.000 2.000
134	134 135 136 137	2	939.000 2.000
135	135 136 137 138	2	946.000 2.000
136	136 137 138 139	2	953.000 2.000
137	137 138 139 140	2	960.000 2.000
138	138 139 140 141	2	967.000 2.000
139	139 140 141 142	2	974.000 2.000
140	140 141 142 143	2	981.000 2.000
141	141 142 143 144	2	988.000 2.000
142	142 143 144 145	2	995.000 2.000
143	143 144 145 146	2	1002.000 2.000
144	144 145 146 147	2	1009.000 2.000
145	145 146 147 148	2	1016.000 2.000
146	146 147 148 149	2	1023.000 2.000
147	147 148 149 150	2	1030.000 2.000
148	148 149 150 151	2	1037.000 2.000
149	149 150 151 152	2	1044.000 2.000
150	150 151 152 153	2	1051.000 2.000
151	151 152 153 154	2	1058.000 2.000
152	152 153 154 155	2	1065.000 2.000
153	153 154 155 156	2	1072.000 2.000
154	154 155 156 157	2	1079.000 2.000
155	155 156 157 158	2	1086.000 2.000
156	156 157 158 159	2	1093.000 2.000
157	157 158 159 160	2	1100.000 2.000
158	158 159 160 161	2	1107.000 2.000
159	159 160 161 162	2	1114.000 2.000
160	160 161 162 163	2	1121.000 2.000
161	161 162 163 164	2	1128.000 2.000
162	162 163 164 165	2	1135.000 2.000
163	163 164 165 166	2	1142.000 2.000
164	164 165 166 167	2	1149.000 2.000
165	165 166 167 168	2	1156.000 2.000
166	166 167 168 169	2	1163.000 2.000
167	167 168 169 170	2	1170.000 2.000
168	168 169 170 171	2	1177.000 2.000
169	169 170 171 172	2	1184.000 2.000
170	170 171 172 173	2	1191.000 2.000
171	171 172 173 174	2	1198.000 2.000
172	172 173 174 175	2	1205.000 2.000
173	173 174 175 176	2	1212.000 2.000
174	174 175 176 177	2	1219.000 2.000
175	175 176 177 178	2	1226.000 2.000
176	176 177 178 179	2	1233.000 2.000
177	177 178 179 180	2	1240.000 2.000
178	178 179 180 181	2	1247.000 2.000
179	179 180 181 182	2	1254.000 2.000
180	180 181 182 183	2	1261.000 2.000
181	181 182 183 184	2	1268.000 2.000
182	182 183 184 185	2	1275.000 2.000
183	183 184 185 186	2	1282.000 2.000
184	184 185 186 187	2	1289.000 2.000
185	185 186 187 188	2	1296.000 2.000
186	186 187 188 189	2	1303.000 2.000
187	187 188 189 190	2	1310.000 2.000
188	188 189 190 191	2	1317.000 2.000
189	189 190 191 192	2	1324.000 2.000
190	190 191 192 193	2	1331.000 2.000
191	191 192 193 194	2	1338.000 2.000
192	192 193 194 195	2	1345.000 2.000
193	193 194 195 196	2	1352.000 2.000
194	194 195 196 197	2	1359.000 2.000
195	195 196 197 198	2	1366.000 2.000
196	196 197 198 199	2	1373.000 2.000
197	197 198 199 200	2	1380.000 2.000
198	198 199 200 201	2	1387.000 2.000
199	199 200 201 202	2	1394.000 2.000
200	200 201 202 203	2	1401.000 2.000
201	201 202 203 204	2	1408.000 2.000
202	202 203 204 205	2	1415.000 2.000
203	203 204 205 206	2	1422.000 2.000
204	204 205 206 207	2	

* LOAD CASE ----- 2 *

LARGEST ELE NO IN THIS INCREMENT 78

LARGEST N P NO IN THIS INCREMENT 85

BAND WIDTH----- 40

TOTAL NUMBER OF EQUATIONS----- 222

NUMBER OF EQUATIONS IN BLOCK----- 84

NUMBER OF BLOCKS----- 3

NUMBER OF N.P. FORCE CARDS----- 3

NUMBER OF PRESSURE CARDS----- 0

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP X-FORCE Y-FORCE

NP	X-FORCE	Y-FORCE
1 0	0	0
2 0	0	0
3 0	0	0
4 0	0	0
5 0	0	0
6 0	0	0
7 0	0	0
8 0	0	0
9 0	0	0
10 0	0	0
11 0	0	0
12 0	0	0
13 0	0	0
14 0	0	0
15 0	0	0
16 0	0	0
17 0	0	0
18 0	0	0
19 0	0	0
20 0	0	0
21 0	0	0
22 0	0	0
23 0	0	0
24 0	0	0
25 0	0	0
26 0	0	0
27 0	0	0
28 0	0	0
29 0	0	0
30 0	0	0
31 0	0	0
32 0	0	0
33 0	0	0
34 0	0	0
35 0	0	0
36 0	0	0
37 0	0	0
38 0	0	0
39 0	0	0
40 0	0	0
41 0	0	0
42 0	0	0
43 0	0	0
44 0	0	0
45 0	0	0
46 0	0	0
47 0	0	0
48 0	0	0
49 0	0	0
50 0	0	0
51 0	0	0
52 0	0	0
53 0	0	0
54 0	0	0
55 0	0	0
56 0	0	0
57 0	0	0
58 0	0	0
59 0	0	0
60 0	0	0
61 0	0	0
62 0	0	0

83 0
 84 0
 85 0
 86 0
 87 0
 88 0
 89 0
 90 0
 91 0
 92 0
 93 0
 94 0
 95 0

LOAD CASE : 2 ITERATION : 2

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0001	.0000	.0000	.0007	.0000	.0007	14
15	.0000	.0000	.0000	.0008	.0003	.0000	.0007	15
16	.0000	.0001	.0000	.0008	.0004	.0000	.0007	16
17	.0006	.0002	.0000	.0048	.0023	.0000	.0056	17
18	.0198	.0087	.0000	.2028	.0387	.0000	.2086	18
19	.0394	.0028	.0000	.2848	.0881	.0000	.3024	19
20	.0418	.0030	.0000	.1888	.1288	.0000	.2254	20
21	.0471	.0006	.0000	.0788	.0000	.0000	.1702	21
22	.0010	.0010	.0000	.0007	.0058	.0000	.0060	22
23	.0010	.0018	.0000	.0001	.0048	.0000	.0049	23
24	.0007	.0058	.0000	.0014	.0108	.0000	.0107	24
25	.0018	.0020	.0000	.0023	.0084	.0000	.0088	25
26	.0000	.0004	.0000	.0000	.0028	.0000	.0028	26
27	.0000	.0001	.0000	.0000	.0002	.0000	.0012	27
28	.0002	.0001	.0000	.0017	.0004	.0000	.0017	28
29	.0000	.0001	.0000	.0013	.0001	.0000	.0013	29
30	.0183	.0058	.0000	.1756	.0894	.0000	.1854	30
31	.0328	.0177	.0000	.2788	.0708	.0000	.2874	31
32	.0882	.0112	.0000	.3583	.1127	.0000	.3756	32
33	.0883	.0088	.0000	.3286	.2483	.0000	.4108	33
34	.1023	.0270	.0000	.2807	.2540	.0000	.3881	34
35	.0914	.0442	.0000	.2041	.1914	.0000	.2788	35
36	.0008	.0048	.0000	.0015	.0133	.0000	.0134	36
37	.0078	.0113	.0000	.0024	.0185	.0000	.0187	37
38	.0020	.0038	.0000	.0020	.0104	.0000	.0106	38
39	.0000	.0018	.0000	.0000	.0048	.0000	.0048	39
40	.0000	.0002	.0000	.0000	.0017	.0000	.0017	40
41	.0057	.0020	.0000	.0010	.0012	.0000	.0021	41
42	.0151	.0034	.0000	.0788	.0184	.0000	.0812	42
43	.0388	.0148	.0000	.1884	.1028	.0000	.2155	43
44	.0371	.0213	.0000	.2144	.0888	.0000	.2264	44
45	.0401	.0173	.0000	.2188	.1508	.0000	.2484	45
46	.0423	.0084	.0000	.1980	.3321	.0000	.3856	46
47	.0448	.0288	.0000	.1708	.3418	.0000	.3817	47
48	.0418	.0882	.0000	.1381	.3173	.0000	.3483	48
49	.0092	.0828	.0000	.0408	.2178	.0000	.2213	49
50	.0018	.0287	.0000	.0043	.0470	.0000	.0482	50
51	.0008	.0031	.0000	.0019	.0119	.0000	.0121	51
52	.0000	.0020	.0000	.0000	.0081	.0000	.0081	52
53	.0380	.0216	.0000	.4801	.1103	.0000	.4927	53
54	.0389	.0174	.0000	.4088	.1324	.0000	.4276	54
55	.0383	.0084	.0000	.2188	.3111	.0000	.3804	55
56	.0348	.0287	.0000	.1838	.3558	.0000	.3813	56
57	.0288	.0883	.0000	.1172	.3431	.0000	.3875	57
58	.0189	.0883	.0000	.0489	.2500	.0000	.2853	58
59	.0080	.0284	.0000	.0413	.0900	.0000	.1032	59
60	.0033	.0033	.0000	.0044	.0110	.0000	.0120	60

61	0000	0021	0000	0000	0063	0000	0063	61
62	0310	0172	0000	0790	0028	0000	0790	62
63	0237	0067	0000	0781	1355	0000	1554	63
64	0282	0288	0000	0770	1015	0000	2084	64
65	0226	0777	0000	0706	2437	0000	2538	65
66	0238	0877	0000	0810	1788	0000	1898	66
67	0183	0318	0000	0327	1247	0000	1288	67
68	0084	0039	0000	0080	0102	0000	0130	68
69	0000	0019	0000	0000	0044	0000	0044	69
70	0238	0039	0000	0888	0358	0000	0882	70
71	0238	0031	0000	0849	0879	0000	1038	71
72	0249	0288	0000	0578	1388	0000	1483	72
73	3115	0983	0000	3483	2228	0000	4178	73
74	0317	0880	0000	0838	1882	0000	1718	74
75	0218	0327	0000	0378	1380	0000	1311	75
76	0084	0049	0000	0089	0101	0000	0142	76
77	0000	0017	0000	0000	0032	0000	0032	77
78	8488	0058	0000	8888	0483	0000	8814	78
79	8430	0022	0000	7073	0813	0000	7100	79
80	8888	0578	0000	7161	1488	0000	7308	80
81	8124	2448	0000	8348	2880	0000	8782	81
82	0135	2831	0000	0258	3448	0000	3484	82
83	8018	0887	0000	8003	0808	0000	8033	83
84	0000	0828	0000	0000	0783	0000	0783	84
85	8788	0032	0000	8818	0134	0000	8817	85
86	8804	0883	0000	8868	0887	0000	8920	86
87	8774	3896	0000	8444	3887	0000	7871	87
88	8818	8772	0000	8881	8084	0000	8021	88
89	2787	1374	0000	2748	1400	0000	3081	89
90	0000	2043	0000	0000	2028	0000	2028	90
91	8410	0881	0000	8410	0881	0000	8444	91
92	8424	3700	0000	8424	3700	0000	7414	92
93	8400	8804	0000	8400	8804	0000	8840	93
94	1137	3188	0000	1137	3188	0000	3358	94
95	0000	3088	0000	0000	3088	0000	3088	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO AXIAL FORCE

INCREMENTAL VALUES

1	0984
2	1088
3	0228
4	0300
5	0182
6	0372
7	0483
8	2428
9	0887
10	0088
11	0081

TOTAL VALUES

1	6427
2	8877
3	2801
4	0412
5	1880
6	3822
7	5188
8	7188
9	2717
10	0181
11	0143

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS	MOD	BULK	MOD	SHEAR	MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE						
1	538	0	341	3	232	5	159	003	006	003	007	002	004	1						
2	504	3	321	4	218	3	159	-001	002	014	010	-008	014	2						
3	511	1	321	2	221	1	156	022	-023	084	036	-036	071	3						
4	5	6	17	2	2	494	1	846	-475	2.315	2	156	3	140	4					
5	5	6	17	2	2	494	1	788	406	7	080	4	131	-2	558	7	089	5		
6	5	6	18	0	2	494	1	775	2	433	8.360	4.424	2.670	7	089	6				
7	5	6	18	4	2	494	-1	348	3	488	3	823	4	073	-1	553	6	023	7	
8	5	6	18	8	2	494	-	975	1	861	-	819	2	017	-1	031	3	048	8	
9	488	7	336	7	186	8	223	-004	135	006	138	-004	138	9						
10	471	4	360	5	190	8	235	-010	183	020	184	-010	204	10						
11	475	7	372	0	191	4	243	-005	201	-078	208	-012	220	11						
12	514	2	366	7	212	4	210	014	102	-045	108	008	098	12						
13	504	5	298	8	223	9	127	012	006	004	012	005	007	13						
14	383	5	224	5	171	5	116	002	012	038	011	-026	037	14						
15	1	3	13.6	6	478	8	494	-	894	-	861	1	842	-1	810	3	152	15		
16	5	7	18.4	2	439	2	504	-1	539	3	738	3	235	-2	271	5	808	16		
17	1	7	17.0	2	483	1	433	1	433	1	558	4	724	3	241	-1	553	4	893	17
18	5	6	17.2	2	494	-	835	2	723	7	839	3	884	-1	385	5	489	18		
19	5	6	17.1	2	494	-	834	2	698	6	331	5	137	-3	272	6	408	19		
20	5	6	18.2	2	494	-	2	056	4	810	4	327	5	435	-2	882	4	118	20	
21	5	6	17.2	2	494	-	1	289	3	232	2	242	3	485	-1	552	5	046	21	
22	426	0	341	7	170	3	381	-015	248	031	248	-016	305	22						
23	406	3	307	10	154	9	232	-003	231	-102	242	-014	250	23						
24	465	3	311	0	155	3	181	027	113	-049	119	-020	099	24						
25	371	8	222	1	184	8	130	038	-006	049	049	-017	086	25						
26	4	1	13.4	1	485	3	485	3	485	1	311	1	778	-	675	1	487	26		
27	0	1	13.8	8	477	1	420	-	879	804	1	485	-1	044	2	530	27			
28	1	9	14.8	8	477	1	056	-	700	-	857	1	125	-	759	1	884	28		
29	2	0	14.8	7	477	7	700	861	-031	708	865	080	080	29						
30	5	6	15.2	2	494	-	320	2	043	-2	896	2	880	-	887	3	837	30		
31	5	6	15.8	2	494	-	805	2	804	-4	004	3	778	-1	879	5	457	31		
32	5	6	15.8	2	494	-	1	513	3	558	-4	222	4	320	-2	277	6	597	32	
33	5	6	17.8	2	494	-	1	882	5	804	-2	170	5	880	-2	038	7	698	33	
34	5	6	17.2	2	494	-	232	3	330	-	227	3	333	-	235	4	058	34		
35	308	8	289	7	121	9	275	-013	450	-	484	-017	471	35						
36	348	8	217	4	150	2	154	-001	084	084	084	-001	083	36						
37	271	5	431	3	97	9	387	421	-032	18	584	9	084	-8	085	18	580	37		
38	366	3	444	1	146	3	316	-278	1	813	18	519	10	569	-8	364	18	583	38	
39	426	8	452	5	181	5	322	-1	304	3	710	10	303	6	932	-4	526	11	458	39
40	245	5	427	8	87	9	387	-	886	4	522	2	301	4	752	-1	226	5	878	40
41	182	3	417	5	56	7	432	-	883	5	124	-	132	5	125	-	882	8	110	41
42	11	3	388	8	4	0	485	-	883	4	148	-	881	4	202	-1	019	5	221	42
43	233	7	426	4	83	3	402	-	784	3	427	-3	038	3	819	-1	256	5	176	43
44	639	7	588	1	280	8	275	-	276	1	790	-2	158	2	351	-	285	2	837	44
45	1112	4	873	8	488	2	137	-	039	086	030	068	-041	108	45					
46	1	4	4.0	0	485	-	050	040	-	171	081	-	102	153	46					
47	11	9	388	8	4	0	485	-	035	048	248	137	-	124	261	47				
48	324	0	438	2	118	7	385	-	049	090	157	125	-	084	210	48				
49	11	2	388	8	4	0	485	-	381	200	857	425	-	816	1	061	49			
50	288	8	429	4	82	2	393	-	285	284	112	300	-	281	1	581	50			
51	577	3	485	2	227	2	271	-	306	2	858	-	310	3	169	51				
52	749	2	870	9	282	1	232	-	226	2	758	-	230	2	988	52				
53	512	7	485	1	199	4	288	-	078	082	084	087	082	188	53					
54	11	2	388	1	4	0	485	-	034	042	092	081	-	048	103	54				
55	288	8	433	5	104	5	378	021	-013	120	087	-	058	126	55					
56	1	4	4.0	0	485	3	809	1	425	15	887	10	380	-5	446	15	826	56		
57	1	4	4.0	0	485	-1	828	1	620	14	481	7	344	-7	552	14	896	57		
58	671	3	638	8	348	3	227	-	301	2	830	708	2	488	-	340	3	209	58	
59	671	6	588	5	283	3	278	-	283	2	780	-	884	2	778	-	321	3	099	59
60	382	4	443	4	134	4	346	-	110	107	031	106	-	111	219	60				
61	1	4	4.0	0	485	-	046	-	359	52	830	26	113	-26	518	52	631	61		

62	1	4	4.0	0	485	1	887	-1	728	59	700	29	975	-29	833	59	808	62
63	1	4	4.0	0	485	3	885	-3	135	54	801	27	788	-27	237	55	025	63
64	1	4	4.0	0	485	-	803	8	229	84	285	35	231	-29	405	84	836	64
65	1	4	4.0	0	485	-	798	21	889	44	132	23	842	-18	048	51	980	65
66	1	4	4.0	0	485	-4	032	18	502	-32	821	28	893	-13	122	38	715	66
67	1	4	4.0	0	485	3	881	-1	402	-31	419	17	059	-14	770	31	829	67
68	1	4	4.0	0	485	3	885	-	103	-1	800	5	889	-	308	4	186	68
69	220	5	424	7	78	3	408	138	-117	048	141	-	119	1	260	69		
70	1	4	4.0	0	485	1	325	8	178	-9	872	9	183	-1	669	10	822	70
71	1	4	4.0	0	485	-	748	22	238	22	183	28	101	-9	608	35	709	71
72	1	4	4.0	0	485	-	746	20	052	44	347	31	667	-21	862	53	429	72
73	1	4	4.0	0	485	5	471	-2	019	13	303	5	369	-5	908	15	287	73
74	1	4	4.0	0	485	-	088	-017	-2	207	1	140	-1	068	2	209	74	
75	1282	0	848	1	843	0	005	-008	015	074	042	-036	078	75				
76	557	1	375	1	218	5	289	-1	147	183	-	218	218	-	149	384	76	
77	1	4	4.0	0	485	-1	147	8	069	13	867	11	444	-11	522	22	966	77
78	1	4	4.0	0	485	1	012	3	899	13	432	9	204	-4	494	13	898	78

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THEYA	SIG1/SIG3	LEVEL	ELE				
1	284	817	007	517	264	128	1	803	1	988	082	1		
2	238	482	038	488	232	133	4	571	2	141	089	2		
3	343	388	127	444	228	128	2	117	2	117	088	3		
4	374	546	006	546	174	088	1	584	1	468	1	715	4	
5	371	549	015	580	389	090	4	654	1	489	1	805	5	
6	488	854	014	659	485	097	4	090	1	419	1	548	6	
7	528	728	008	728	527	101	2	346	1	382	2	015	7	
8	331	523	002	523	331	098	-	467	1	580	1	920	8	
9	324	1	046	003	1	046	381	2	229	3	229	218	9	
10	399	1	380	037	1	382	397	4	92	2	184	3	479	10
11	444	1	438	152	1	461	422	5	20	3	476	3	465	11
12	378	820	084	828	362	287	-	584	2	584	1	86	12	
13	195	307	008	308	194	057	4	289	1	583	040	13		
14	118	220	085	251	087	032	28	899	1	889	087	14		
15	048	183	011	185	037	038	7	489	1	886	773	15		

43	128	1.178	294	1.2	68	586	-14 748	20.528	7.258	43
44	141	1.128	085	1.134	123	501	4.883	8.814	2.793	44
45	190	423	113	443	210	328	10.158	-2.108	-1.000	45
46	003	022	009	028	000	013	71.848	-2886.018	-1.000	46
47	088	144	100	208	009	108	71.119	-22.838	-1.000	47
48	037	407	187	488	041	283	22.880	-11.822	-1.000	48
49	228	014	088	023	283	173	-21.037	088	-1.000	49
50	108	819	119	841	128	138	10.358	-4.183	-1.000	50
51	421	909	571	1.121	832	878	-20.322	-1.773	-1.000	51
52	482	811	210	942	522	732	8.351	-1.803	-1.000	52
53	283	382	118	591	284	333	10.084	-1.344	-1.000	53
54	058	083	033	02	024	083	43.510	-5.828	-1.000	54
55	128	088	212	173	102	212	46.968	-3.182	-1.000	55
56	293	488	044	578	282	088	13.183	1.697	289	56
57	042	188	102	224	018	120	-28.427	-14.178	-1.000	57
58	787	1.087	208	1.119	788	884	-8.211	-1.418	-1.000	58
59	1.087	828	117	833	-1.088	848	3.978	-588	-1.000	59
60	308	371	052	328	312	318	4.688	-1.043	-1.000	60
61	008	001	023	020	027	023	39.828	747	-1.000	61
62	131	102	088	214	019	088	48.400	11.244	3.808	62
63	278	131	172	390	018	187	58.338	23.880	8.542	63
64	378	428	059	488	339	083	-34.180	1.373	138	64
65	888	772	004	772	888	043	-2.892	1.128	047	65
66	808	539	013	844	801	021	-18.081	1.088	031	66
67	188	308	011	310	188	081	-5.188	1.848	241	67
68	180	188	014	188	180	022	70.388	1.287	111	68
69	322	088	047	342	088	231	81.328	-3.488	-1.000	69
70	408	414	031	443	381	031	-42.722	1.183	080	70
71	883	703	008	704	881	022	12.111	1.088	024	71
72	418	487	018	483	409	027	20.470	1.132	048	72
73	210	283	008	283	210	022	8.323	1.208	077	73
74	011	020	001	020	011	004	7.844	1.778	288	74
75	101	233	478	888	438	504	38.337	-1.301	-1.000	75
76	308	1.017	478	1.170	488	814	-17.840	-2.548	-1.000	76
77	007	058	008	038	008	017	8.740	8.818	2.188	77
78	204	223	005	224	202	011	14.813	1.108	040	78

SSTIPH 1 LAYER OF GEOTEXTILE, P=4000 PPI, T=0

TOTAL NUMBER OF NODES-----	95
NUMBER OF BAR ELEMENTS-----	9
NUMBER OF DIFF. BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO OF INTERFACE ELE IN PREEXIST PART-----	0
NUMBER OF INTERFACE ELE IN FOUNDATION-----	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	38
NUMBER OF NODES IN FOUNDATION-----	92
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	6
NUMBER OF LOAD CASES-----	1

CALING FACTOR ----- 1 00000

ATNOSPHERIC PRESSURE --- 1 08800

UNIT WEIGHT OF WATER --- 03120

COMPUTATION SEQUENCE FOR A TOTAL OF 7 INCREMENTS

INCREMENT NO	1	PUT ON LAYER NO	1
INCREMENT NO	2	PUT ON LAYER NO	2
INCREMENT NO	3	PUT ON LAYER NO	3
INCREMENT NO	4	PUT ON LAYER NO	4
INCREMENT NO	5	PUT ON LAYER NO	5
INCREMENT NO	6	PUT ON LAYER NO	6
INCREMENT NO	7	APPLY LOAD CASE	1

MODAL POINT INPUT DATA

NODE NUMBER	MODAL POINT COORDINATES		B.C. CODE		
	X-ORD	Y-ORD	X	Y	Z
1	000	000	1	1	1
2	10.000	.000	1	1	1
3	20.000	.000	1	1	1
4	30.000	.000	1	1	1
5	38.000	.000	1	1	1
6	42.000	.000	1	1	1
7	50.000	.000	1	1	1
8	54.000	.000	1	1	1
9	58.000	.000	1	1	1
10	66.000	.000	1	1	1
11	74.000	.000	1	1	1
12	82.000	.000	1	1	1
13	90.000	.000	1	1	1
14	7.000	4.000	0	0	0
15	10.000	4.000	0	0	0
16	20.000	4.000	0	0	0
17	30.000	4.000	0	0	0
18	38.000	4.000	0	0	0
19	42.000	4.000	0	0	0
20	50.000	4.000	0	0	0
21	54.000	4.000	0	0	0
22	58.000	4.000	0	0	0
23	66.000	4.000	0	0	0
24	74.000	4.000	0	0	0
25	82.000	4.000	0	0	0
26	90.000	4.000	0	0	0
27	7.000	7.000	0	0	0
28	10.000	7.000	0	0	0
29	20.000	7.000	0	0	0
30	30.000	7.000	0	0	0
31	38.000	7.000	0	0	0
32	42.000	7.000	0	0	0
33	50.000	7.000	0	0	0
34	54.000	7.000	0	0	0
35	58.000	7.000	0	0	0
36	66.000	7.000	0	0	0
37	74.000	7.000	0	0	0
38	82.000	7.000	0	0	0
39	90.000	7.000	0	0	0
40	7.000	10.000	0	0	0
41	10.000	10.000	0	0	0
42	20.000	10.000	0	0	0
43	30.000	10.000	0	0	0
44	38.000	10.000	0	0	0
45	42.000	10.000	0	0	0
46	50.000	10.000	0	0	0
47	54.000	10.000	0	0	0
48	58.000	10.000	0	0	0
49	66.000	10.000	0	0	0
50	74.000	10.000	0	0	0
51	82.000	10.000	0	0	0
52	90.000	10.000	0	0	0
53	7.000	11.500	0	0	0
54	10.000	11.500	0	0	0
55	20.000	11.500	0	0	0
56	30.000	11.500	0	0	0
57	38.000	11.500	0	0	0
58	42.000	11.500	0	0	0
59	50.000	11.500	0	0	0
60	54.000	11.500	0	0	0
61	58.000	11.500	1	0	1
62	42.000	13.000	0	0	0
63	50.000	13.000	0	0	0
64	54.000	13.000	0	0	0
65	58.000	13.000	0	0	0
66	66.000	13.000	0	0	0
67	74.000	13.000	0	0	0
68	82.000	13.000	0	0	0
69	90.000	13.000	1	0	1
70	48.000	14.000	0	0	0
71	50.000	14.000	0	0	0
72	54.000	14.000	0	0	0
73	58.000	14.000	0	0	0
74	66.000	14.000	0	0	0
75	74.000	14.000	0	0	0
76	82.000	14.000	0	0	0
77	90.000	14.000	1	0	1
78	50.000	15.000	0	0	0
79	54.000	15.000	0	0	0
80	58.000	15.000	0	0	0
81	66.000	15.000	0	0	0
82	74.000	15.000	0	0	0
83	82.000	15.000	0	0	0
84	90.000	15.000	1	0	1
85	54.000	16.000	0	0	0
86	58.000	16.000	0	0	0
87	66.000	16.000	0	0	0
88	74.000	16.000	0	0	0
89	82.000	16.000	1	0	1
90	90.000	16.000	1	0	1
91	58.000	17.000	0	0	0
92	66.000	17.000	0	0	0
93	74.000	17.000	0	0	0
94	82.000	17.000	0	0	0
95	90.000	17.000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	120	1.00	0

ELBY NO	CONNECTED NODES I J	MATL NO.
1	43 44	1
2	44 45	1
3	45 46	1
4	46 47	1
5	47 48	1
6	48 49	1
7	49 50	1
8	50 51	1
9	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S MODULUS CONSTANT	EXPO-NENT	RATIO	BULK MODULUS CONSTANT	EXPO-NENT	STRENGTH C	PARAMETERS PHI	DPHI	Kb
1	0880	8000.00	.500	.500	1500.00	.800	00	35.00	00	50
2	0830	4000.00	.300	.800	700.00	.200	05	00	00	50
3	0800	1000.00	.400	.700	500.00	.500	.50	40.00	00	50

NODES SOLID ELEMENT DATA

ELET NO	CONNECTED NODES I J K L	MATL NO.	ELEMENT CENTER COORDINATES X-ORD Y-ORD
1	1 2 15 14	3	5.000 2.000
2	2 3 16 15	3	15.000 2.000
3	3 4 17 16	3	25.000 2.000
4	4 5 18 17	2	33.000 2.000
5	5 6 19 18	2	39.000 2.000
6	6 7 20 19	2	45.000 2.000
7	7 8 21 20	2	52.000 2.000
8	8 9 22 21	2	58.000 2.000
9	9 10 23 22	3	62.000 2.000
10	10 11 24 23	3	70.000 2.000
11	11 12 25 24	3	78.000 2.000
12	12 13 26 25	3	86.000 2.000
13	14 15 28 27	3	5.000 5.500
14	15 16 29 28	3	15.000 5.500
15	16 17 30 29	2	25.000 5.500
16	17 18 31 30	2	33.000 5.500
17	18 19 32 31	2	39.000 5.500
18	19 20 33 32	2	45.000 5.500
19	20 21 34 33	2	52.000 5.500
20	21 22 35 34	2	58.000 5.500
21	22 23 36 35	2	62.000 5.500
22	23 24 37 36	3	70.000 5.500
23	24 25 38 37	3	78.000 5.500
24	25 26 39 38	3	86.000 5.500
25	27 28 41 40	3	5.000 8.500
26	28 29 42 41	2	15.000 8.500
27	29 30 43 42	2	25.000 8.500
28	30 31 44 43	2	33.000 8.500
29	31 32 45 44	2	39.000 8.500
30	32 33 46 45	2	45.000 8.500
31	33 34 47 46	2	52.000 8.500
32	34 35 48 47	2	58.000 8.500
33	35 36 49 48	2	62.000 8.500
34	36 37 50 49	2	70.000 8.500
35	37 38 51 50	3	78.000 8.500
36	38 39 52 51	3	86.000 8.500
37	43 44 53 53	1	24.500 10.750
38	44 45 54 53	1	33.000 10.750
39	45 46 55 54	1	42.000 10.750
40	46 47 56 55	1	52.000 10.750
41	47 48 57 56	1	62.000 10.750
42	48 49 58 57	1	70.000 10.750
43	49 50 59 58	1	78.000 10.750
44	50 51 60 59	1	86.000 10.750
45	51 52 61 60	1	95.000 12.250
46	52 53 62 61	1	10.500 12.250
47	54 55 63 62	1	46.000 12.250
48	55 56 64 63	1	52.000 12.250
49	56 57 65 64	1	58.000 12.250
50	57 58 66 65	1	62.000 12.250
51	58 59 67 66	1	70.000 12.250
52	59 60 68 67	1	78.000 12.250
53	60 61 69 68	1	86.000 12.250
54	62 63 71 70	1	47.000 13.500
55	63 64 72 71	1	52.000 13.500
56	64 65 73 72	1	58.000 13.500
57	65 66 74 73	1	62.000 13.500
58	66 67 75 74	1	70.000 13.500
59	67 68 76 75	1	78.000 13.500
60	68 69 77 76	1	86.000 13.500
61	70 71 78 78	1	49.000 14.500
62	71 72 79 76	1	52.000 14.500
63	72 73 80 79	1	56.000 14.500
64	73 74 81 80	1	62.000 14.500
65	74 75 82 81	1	70.000 14.500
66	75 76 83 82	1	78.000 14.500
67	76 77 84 83	1	86.000 14.500
68	78 79 85 85	1	53.000 15.500
69	79 80 86 86	1	58.000 15.500
70	80 81 87 86	1	62.000 15.500
71	81 82 88 87	1	70.000 15.500
72	82 83 89 88	1	78.000 15.500
73	83 84 90 89	1	86.000 15.500
74	85 86 91 91	1	57.000 16.500
75	86 87 92 91	1	62.000 16.500
76	87 88 93 92	1	70.000 16.500
77	88 89 94 93	1	78.000 16.500
78	89 90 95 94	1	86.000 16.500

CONSTRUCTION LAYER INFORMATION

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP	X-FORCE	Y-FORCE
1	0.	0.
2	0.	0.
3	0.	0.
4	0.	0.
5	0.	0.
6	0.	0.
7	0.	0.
8	0.	0.
9	0.	0.
10	0.	0.
11	0.	0.
12	0.	0.
13	0.	0.
14	0.	0.
15	0.	0.
16	0.	0.
17	0.	0.
18	0.	0.
19	0.	0.
20	0.	0.
21	0.	0.
22	0.	0.
23	0.	0.
24	0.	0.
25	0.	0.
26	0.	0.
27	0.	0.
28	0.	0.
29	0.	0.
30	0.	0.
31	0.	0.
32	0.	0.
33	0.	0.
34	0.	0.
35	0.	0.
36	0.	0.
37	0.	0.
38	0.	0.
39	0.	0.
40	0.	0.
41	0.	0.
42	0.	0.
43	0.	0.
44	0.	0.
45	0.	0.
46	0.	0.
47	0.	0.
48	0.	0.
49	0.	0.
50	0.	0.
51	0.	0.
52	0.	0.
53	0.	0.
54	0.	0.
55	0.	0.
56	0.	0.
57	0.	0.
58	0.	0.
59	0.	0.
60	0.	0.
61	0.	0.
62	0.	0.
63	0.	0.
64	0.	0.
65	0.	0.
66	0.	0.
67	0.	0.
68	0.	0.
69	0.	0.
70	0.	0.
71	0.	0.
72	0.	0.
73	0.	0.
74	0.	0.
75	0.	0.
76	0.	0.
77	0.	0.
78	0.	0.
79	0.	0.
80	0.	0.
81	0.	0.
82	0.	0.
83	0.	0.
84	0.	0.
85	0.	0.
86	0.	0.
87	0.	0.
88	0.	0.
89	0.	0.
90	0.	0.
91	0.	0.
92	0.	-4.50
93	0.	-4.50
94	0.	-2.00
95	0.	0.

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0000	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0001	.0000	.0000	.0001	15
16	.0000	.0001	.0000	.0003	.0008	.0000	.0007	16
17	.0010	.0008	.0000	.0083	.0024	.0000	.0088	17
18	.0304	.0086	.0000	.2218	.0451	.0000	.2284	18
19	.0428	.0074	.0000	.3188	.0760	.0000	.3283	19
20	.0087	.0287	.0000	.1688	.1470	.0000	.2222	20
21	.0007	.0010	.0000	.0088	.1388	.0000	.1478	21
22	.0001	.0000	.0000	.0003	.0048	.0000	.0048	22
23	.0010	.0018	.0000	.0008	.0047	.0000	.0047	23
24	.0008	.0088	.0000	.0007	.0108	.0000	.0110	24
25	.0021	.0022	.0000	.0020	.0087	.0000	.0081	25
26	.0000	.0004	.0000	.0000	.0020	.0000	.0020	26
27	.0000	.0000	.0000	.0000	.0001	.0000	.0001	27
28	.0000	.0000	.0000	.0000	.0002	.0000	.0002	28
29	.0000	.0001	.0000	.0014	.0000	.0000	.0014	29
30	.0280	.0108	.0000	.2082	.0638	.0000	.2178	30
31	.0444	.0148	.0000	.3110	.0888	.0000	.3228	31
32	.0842	.0134	.0000	.3828	.1374	.0000	.4084	32
33	.0334	.0812	.0000	.2887	.2881	.0000	.4188	33
34	.0348	.0088	.0000	.2288	.2222	.0000	.3288	34
35	.0483	.0184	.0000	.1822	.1837	.0000	.2308	35
36	.0003	.0032	.0000	.0018	.0118	.0000	.0120	36
37	.0018	.0112	.0000	.0011	.0200	.0000	.0201	37
38	.0024	.0042	.0000	.0011	.0113	.0000	.0114	38
39	.0000	.0018	.0000	.0000	.0030	.0000	.0030	39
40	.0000	.0000	.0000	.0000	.0001	.0000	.0001	40
41	.0000	.0000	.0000	.0001	.0001	.0000	.0003	41
42	.0108	.0087	.0000	.1107	.0387	.0000	.1178	42
43	.0471	.0228	.0000	.2884	.0372	.0000	.2781	43
44	.0478	.0131	.0000	.2883	.0823	.0000	.2711	44
45	.0800	.0188	.0000	.2488	.1888	.0000	.3188	45
46	.0488	.0723	.0000	.2128	.4080	.0000	.4801	46
47	.0424	.0131	.0000	.1778	.3194	.0000	.3888	47
48	.0328	.0188	.0000	.1420	.2883	.0000	.3018	48
49	.0183	.0708	.0000	.0884	.2088	.0000	.2147	49
50	.0011	.0271	.0000	.0088	.0488	.0000	.0481	50
51	.0008	.0038	.0000	.0038	.0148	.0000	.0182	51
52	.0000	.0022	.0000	.0000	.0728	.0000	.0728	52
53	.0421	.0132	.0000	.8127	.1188	.0000	.8282	53
54	.0408	.0187	.0000	.4287	.1713	.0000	.4817	54
55	.0387	.0732	.0000	.2242	.3888	.0000	.4840	55
56	.0172	.0188	.0000	.1784	.3288	.0000	.3872	56
57	.0288	.0178	.0000	.1387	.3011	.0000	.3372	57
58	.0138	.0738	.0000	.0878	.2403	.0000	.2471	58
59	.0088	.0301	.0000	.0437	.0872	.0000	.1088	59
60	.0032	.0040	.0000	.0081	.0138	.0000	.0188	60
61	.0000	.0023	.0000	.0000	.0888	.0000	.0888	61
62	.0334	.0188	.0000	.0804	.0327	.0000	.0888	62
63	.0728	.0748	.0000	.1720	.2060	.0000	.2384	63
64	.0734	.0118	.0000	.1244	.1818	.0000	.1980	64
65	.0088	.0188	.0000	.0888	.1808	.0000	.1987	65
66	.0171	.0748	.0000	.0808	.2072	.0000	.2188	66
67	.0131	.0318	.0000	.0418	.1088	.0000	.1183	67
68	.0088	.0047	.0000	.0181	.0380	.0000	.0413	68
69	.0000	.0022	.0000	.0000	.0287	.0000	.0287	69
70	.0787	.0407	.0000	.1088	.0812	.0000	.1340	70
71	.0880	.0744	.0000	.1288	.1888	.0000	.2032	71
72	.0888	.0118	.0000	.1431	.0882	.0000	.1840	72
73	.1084	.0018	.0000	.1431	.1287	.0000	.1904	73
74	.0231	.0787	.0000	.0888	.1883	.0000	.1944	74
75	.0178	.0318	.0000	.0388	.1287	.0000	.1314	75
76	.0088	.0082	.0000	.0114	.0108	.0000	.0188	76
77	.0000	.0017	.0000	.0000	.0034	.0000	.0034	77
78	.0882	.0713	.0000	.1083	.1103	.0000	.1883	78
79	.0888	.0111	.0000	.1120	.0389	.0000	.1168	79
80	.1033	.0014	.0000	.1200	.0882	.0000	.1321	80
81	.2814	.2788	.0000	.2782	.3178	.0000	.4228	81
82	.2318	.2488	.0000	.7208	.3074	.0000	.7872	82
83	.1188	.0420	.0000	.1178	.0483	.0000	.1188	83
84	.0000	.1187	.0000	.0000	.1183	.0000	.1183	84
85	.0848	.0111	.0000	.1008	.0081	.0000	.1013	85
86	.1083	.0088	.0000	.1124	.0188	.0000	.1138	86
87	.1074	.4834	.0000	.1180	.8188	.0000	.8284	87
88	.8881	.8382	.0000	.8834	.8887	.0000	1.0344	88
89	.8783	.0813	.0000	.8782	.0839	.0000	1.8781	89
90	.0000	.3711	.0000	.0000	.3883	.0000	.3883	90
91	.0430	.0072	.0000	.0430	.0072	.0000	.0438	91
92	.0448	.4882	.0000	.0448	.4882	.0000	.4982	92
93	.7888	.8017	.0000	.7888	.8017	.0000	1.1788	93
94	.8488	.0830	.0000	.8488	.0830	.0000	1.8484	94
95	.0000	.8881	.0000	.0000	.8881	.0000	.8881	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS - INTERNAL MEMBER FORCES

ELEMENT NO. AXIAL FORCE

INCREMENTAL VALUES

1	.0180
2	.0428
3	.0231
4	.1818
5	.1092
6	.3371
7	.2272
8	.0100
9	.0072

TOTAL VALUES

1	.0014
2	.1853
3	.5389
4	.0474
5	.0749
6	.1258
7	.7782

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	525.8	330.2	227.5	.155	.001	.000	.002	.001	-.001	.002	1
2	525.1	332.3	226.3	.151	.001	.002	.008	.010	-.001	.011	2
3	532.2	333.8	230.4	.155	.025	-.023	-.054	.037	-.035	.072	3
4	.6	17.1	.2	.484	1.805	-.594	2.483	2.332	-1.121	3.452	4
5	.6	17.1	.2	.484	1.805	412	7.748	4.475	-3.280	7.755	5
6	.7	18.0	.2	.464	1.833	2.813	8.451	4.675	-2.754	7.466	6
7	.7	18.4	.2	.493	-1.305	3.548	2.552	3.882	-1.719	5.501	7
8	.6	16.9	.2	.484	-.701	1.770	-.957	1.860	-.791	2.550	8
9	470.2	345.5	182.5	.221	.005	.118	.002	.118	.005	.113	9
10	478.7	359.5	183.3	.238	-.008	.135	.037	.137	-.010	.208	10
11	474.4	372.3	180.7	.244	-.008	.208	-.088	.213	-.013	.228	11
12	500.8	352.3	207.7	.205	.012	.085	-.048	.103	.008	.097	12
13	435.9	258.8	184.4	.121	.001	.002	-.002	.002	.000	.002	13
14	435.9	272.6	206.8	.117	.008	.008	.017	.012	-.010	.022	14
15	1.9	15.5	.8	.477	1.058	-1.036	3.068	1.889	-1.842	3.712	15
16	7.5	18.5	2.6	.420	2.881	-1.702	4.332	3.554	-2.552	6.145	16
17	1.9	16.5	.8	.461	1.379	.311	5.484	3.829	-1.949	5.587	17
18	.6	17.1	.2	.484	-1.513	3.458	4.531	4.351	-2.408	6.753	18
19	.6	17.1	.2	.484	-2.138	4.057	4.033	4.656	-2.738	7.392	19
20	.7	18.2	.2	.484	-1.541	4.239	3.088	4.628	-1.930	6.557	20
21	.6	17.2	.2	.484	-1.018	2.768	1.726	2.955	-1.208	4.162	21
22	481.8	382.1	180.5	.261	.008	.271	.046	.273	-.007	.260	22
23	408.7	310.8	155.7	.233	-.008	.245	-.078	.251	-.014	.255	23
24	410.2	276.0	172.8	.187	.019	.111	-.081	.120	.010	.110	24
25	408.7	228.5	187.0	.093	-.001	.000	.001	.001	-.001	.002	25
26	.0	.1	.0	.485	.581	-.683	1.618	.984	-1.064	2.030	26
27	1.9	13.6	.8	.476	1.772	-1.219	2.051	2.081	-1.537	3.627	27
28	3.1	13.7	1.0	.482	.855	-.487	-.100	.855	-.489	1.357	28
29	1.3	14.2	.5	.484	.514	.910	1.004	1.232	-.173	1.079	29
30	.5	15.1	.2	.484	-.604	2.734	-1.157	2.826	-.658	3.722	30
31	.5	15.8	.2	.484	-1.189	3.335	-4.035	4.105	-1.958	6.063	31
32	.5	15.8	.2	.484	-1.287	3.183	-2.720	3.848	-1.870	5.218	32
33	.7	17.8	.2	.484	-1.547	4.855	-.951	4.972	-1.583	6.535	33
34	.4	17.2	.2	.484	-.320	3.872	-.178	3.874	-.021	3.853	34
35	308.6	278.7	120.3	.283	-.020	.485	-.038	.488	-.021	.505	35
36	361.4	220.1	158.5	.140	-.018	1.217	-.387	1.248	-.045	1.291	36
37	163.0	417.6	58.8	.432	.023	.255	.170	8.824	-8.845	17.570	37
38	684.5	487.5	287.5	.225	.774	2.008	20.060	10.714	-8.476	20.163	38
39	335.9	440.0	124.0	.359	-1.625	3.830	10.444	7.048	-4.642	11.681	39
40	11.9	398.8	4.0	.485	-1.130	4.273	-.175	4.274	-1.131	5.406	40
41	270.5	421.1	97.5	.347	-.910	4.935	-.031	4.935	-.910	5.845	41
42	236.7	428.8	84.5	.401	-.822	4.317	-.220	4.318	-.822	5.314	42
43	208.8	423.4	74.2	.412	-.927	3.528	-3.341	4.083	-1.484	5.567	43
44	670.1	588.1	282.3	.277	.189	1.854	-2.788	2.852	-.599	3.251	44
45	1318.4	823.5	571.8	.150	.035	.021	.288	.171	-.115	.287	45
46	430.3	483.0	163.0	.320	.081	.051	.220	.105	-.134	.236	46
47	.1	4.0	.0	.485	.228	.078	2.142	1.225	-.922	2.147	47
48	411.2	480.2	154.8	.328	-.037	.075	.455	.254	-.215	.489	48
49	.1	4.0	.0	.485	-1.018	.075	1.410	.422	-1.352	1.783	49
50	389.0	444.3	137.1	.348	-.236	1.572	.100	1.573	-.237	1.810	50
51	981.9	702.3	405.2	.211	-.257	3.828	.241	3.832	-.281	3.893	51
52	937.4	898.5	331.8	.262	-.231	3.075	.985	3.099	-.255	3.354	52
53	897.8	472.0	220.1	.267	-.130	1.822	.298	1.833	-.141	1.974	53
54	.1	4.0	.0	.485	.115	-.018	3.204	1.755	-.543	3.314	54
55	1086.2	624.0	497.0	.102	.053	.018	.085	.073	-.001	.073	55
56	.1	4.0	.0	.485	-.882	-.710	8.537	2.572	-3.955	8.537	56
57	.1	4.0	.0	.485	-.527	-.620	5.889	2.411	-3.558	5.889	57
58	1370.1	783.5	618.8	.104	-.484	2.778	1.427	2.868	-.648	3.653	58
59	1661.1	903.1	775.8	.070	-.045	2.717	1.154	2.832	-.161	2.953	59
60	413.4	480.8	158.8	.327	-.123	.100	.080	.104	-.127	.231	60
61	.1	4.0	.0	.485	.504	-.319	1.222	.629	-.844	1.473	61

62		4.0		.485	.068	-1.127	-1.887	988	-1.029	1.886	82
63	448.1	455.1	189.4	.314	.220	-1.126	.175	241	-1.147	.388	83
64		4.0	.0	.485	.461	9.788	13.918	13.488	-3.258	18.741	84
65		4.0	.0	.485	-6.329	21.980	-25.594	28.881	-11.280	38.141	85
66		4.0	.0	.485	-2.588	13.908	-95.703	54.228	-42.888	97.111	86
67		4.0	.0	.485	8.818	-4.002	-67.498	30.720	-27.808	58.526	87
68		4.0	.0	.485	.051	-0.005	-2.228	1.137	-1.081	2.228	88
69	203.9	422.7	72.1	.415	.226	-2.115	.228	.283	-.243	.496	89
70		4.0	.0	.485	1.008	10.778	-2.788	10.870	.812	10.158	90
71		4.0	.0	.485	-12.338	28.918	-14.488	28.203	-13.624	41.827	91
72		4.0	.0	.485	-8.887	18.413	-34.183	25.437	-15.222	41.389	92
73		4.0	.0	.485	18.837	-12.079	-28.488	21.988	-17.227	39.212	93
74		4.0	.0	.485	28.9	-18.1	-8.090	3.108	-2.888	8.104	94
75	887.2	505.2	235.7	.287	.023	.084	.008	.084	.022	.041	95
76		4.0	.0	.485	-11.122	18.313	8.100	18.532	-11.351	28.873	96
77		4.0	.0	.485	-9.883	18.288	-0.529	18.831	-10.358	28.888	97
78		4.0	.0	.485	20.188	-14.784	-10.780	20.888	-15.574	38.538	98

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THEY1	SIG1/SIG3	LEVEL	ELE
1	.244	.481	.004	.481	.244	.118	4.988	1.971	.078	1
2	.252	.524	.020	.525	.251	.137	4.127	2.084	.080	2
3	.381	.375	.130	.488	.238	.120	45.483	2.081	.087	3
4	.382	.882	.008	.882	.381	.088	1.891	1.487	1.704	4
5	.374	.882	.018	.884	.373	.080	8.077	1.488	1.810	5
6	.488	.882	.014	.882	.487	.088	4.081	1.408	1.877	6
7	.348	.838	.002	.838	.348	.101	1.888	1.377	2.018	7
8	.349	.882	.001	.882	.349	.086	-0.880	1.589	1.800	8
9	.418	1.408	.073	.413	.411	.317	-1.118	2.415	1.86	9
10	.418	1.408	.073	.413	.411	.801	4.171	3.441	2.77	10
11	.411	.473	.127	.488	.424	.831	-8.830	3.488	.289	11
12	.388	.478	.089	.486	.340	.278	-10.478	2.837	.188	12
13	.138	.278	.003	.278	.138	.070	-1.302	2.008	.053	13
14	.187	.248	.034	.288	.154	.082	20.708	1.873	.038	14
15	.102	.177	.013	.177	.100	.038	8.788	1.783	.784	15
16	.284	.270	.038	.318	.238	.038	80.310	1.329	.781	16
17	.383	.433	.017	.437	.380	.043	11.830	1.288	.870	17
18	.381	.480	.008	.480	.380	.088	4.884	1.280	1.104	18
19	.374	.488	.008	.488	.373	.087	4.048	1.308	1.180	19
20	.814	.828	.007	.828	.813	.082	3.883	1.224	1.180	20
21	.380	.488	.008	.488	.380	.087	1.818	1.302	1.081	21
22	.217	1.880	.089	1.887	.410	.573	4.888	3.788	.317	22
23	.322	1.278	.128	1.284	.308	.488	-7.887	4.283	.308	23
24	.288	.833	.104	.860	.229	.218	-14.488	2.887	1.48	24
25	.041	.081	.002	.081	.041	.028	2.480	2.207	.022	25
26	.002	.008	.003	.003	.007	.008	88.888	-.442	-1.000	26
27	.087	.008	.007	.088	.008	.041	88.481	18.887	.829	27
28	.081	.024	.008	.082	.023	.034	-83.180	3.888	.888	28
29	.180	.248	.008	.248	.180	.043	4.124	1.843	.888	29
30	.224	.328	.008	.328	.223	.083	-2.818	1.473	1.088	30
31	.280	.384	.011	.388	.259	.083	-5.818	1.413	1.088	31
32	.218	.318	.014	.321	.218	.083	-7.972	1.487	1.082	32
33	.478	.888	.001	.888	.478	.088	-7.433	1.231	1.108	33
34	.478	.871	.003	.871	.478	.088	1.801	1.201	1.888	34
35	.480	.820	.008	.820	.480	.082	-1.088	3.117	.341	35
36	.081	.288	.008	.288	.081	.104	-1.888	5.081	.088	36
37	.101	.078	.188	.278	.088	.188	-48.738	-2.887	-1.000	37
38	.078	.104	.018	.113	.070	.022	27.878	1.823	.231	38
39	.033	.373	.188	.484	.088	.281	24.612	-8.018	-1.000	39
40	-.110	.137	.108	.178	-.148	.182	-20.333	-1.178	-1.000	40
41	-.804	.388	.073	.381	-.810	.488	4.318	-.882	-1.000	41
42	-.808	.423	.104	.433	-.818	.824	8.724	-.704	-1.000	42

43	-.088	.812	-.443	1.080	-.284	.887	-20.788	-4.247	-1.000	43
44	.202	1.278	.027	1.278	.202	.837	1.482	8.332	1.882	44
45	.018	.402	.312	.878	-.188	.388	28.188	-3.888	-1.000	45
46	-.083	.037	.028	.042	-.088	.088	11.188	-.477	-1.000	46
47	.081	.128	.111	.218	-.008	.114	38.888	-24.789	-1.000	47
48	.028	.433	.408	.888	-.227	.488	31.708	-3.020	-1.000	48
49	-.082	.123	.184	.218	-.188	.184	-30.229	-1.384	-1.000	49
50	-.084	.381	.311	.543	-.218	.380	27.489	-2.811	-1.000	50
51	-.480	1.034	-.842	1.414	-.830	1.122	-24.318	-1.703	-1.000	51
52	-.384	.888	.177	.889	-.408	.849	7.887	-2.178	-1.000	52
53	-.302	.380	.131	.378	-.328	.382	10.828	-1.148	-1.000	53
54	.088	.078	.081	.148	-.018	.082	41.008	-10.188	-1.000	54
55	.817	.328	.382	.882	.081	.381	88.227	8.383	3.120	55
56	.118	.038	.134	.214	-.084	.138	53.283	-3.328	-1.000	56
57	.088	.213	.120	.211	-.001	.141	-28.212	-243.283	-1.000	57
58	-.883	1.108	-.318	1.182	-.718	.840	-8.828	-1.820	-1.000	58
59	-1.808	.738	.108	.740	-1.813	1.177	2.883	-.488	-1.000	59
60	-.428	.244	.102	.288	-.440	.380	8.480	-.888	-1.000	60
61	.018	.024	.003	.028	.018	.008	18.808	1.407	.228	61
62	-.138	.088	.104	.228	-.012	.108	80.344	17.880	8.182	62
63	1.184	-.188	-.389	1.251	-.283	.782	78.288	-4.948	-1.000	63
64	.488	.882	-.021	.882	.481	.084	-28.888	1.418	1.84	64
65	-.848	.788	-.082	.772	-.848	.083	-10.071	1.187	.073	65
66	.482	.483	-.038	.518	.437	.041	-24.224	1.188	.070	66
67	.228	.331	-.018	.338	.228	.088	-10.188	1.487	.181	67
68	.049	.038	.014	.089	.028	.018	88.038	2.003	.373	68
69	.418	.288	.218	.478	.401	.401	72.827	1.488	.008	69
70	.822	.891	-.037	.807	.808	.081	-23.802	1.200	.074	70
71	.883	.831	-.008	.831	.882	.028	-8.708	1.088	.032	71
72	.387	.423	-.014	.427	.382	.022	-18.718	1.117	.044	72
73	.274	.288	-.011	.301	.270	.018	-21.478	1.113	.042	73
74	.013	.021	.000	.021	.013	.004	-2.588	1.833	.238	74
75	.388	.887	.018	.889	.388	.107	4.804	1.804	.224	75
76	.282	.332	.002	.332	.282	.020	2.889	1.138	.082	76
77	.338	.378	-.003	.378	.338	.020	-3.782	1.118	.044	77
78	.248	.238	-.004	.247	.233	.007	-70.821	1.089	.022	78

SSTIPH. 1 LAYER OF GEOTEXTILE, P=6000 PPI, T=1000

TOTAL NUMBER OF NODES-----	95
NUMBER OF BAR ELEMENTS-----	9
NUMBER OF DIFF BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO OF INTERFACE ELE IN PREEXIST PART-----	0
NUMBER OF INTERFACE "ELE" IN FOUNDATION-----	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	38
NUMBER OF NODES IN FOUNDATION-----	52
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	6
NUMBER OF LOAD CASES-----	2

CALING FACTOR ----- 1 00000

ATMOSPHERIC PRESSURE --- 1 06800

UNIT WEIGHT OF WATER --- 03120

DISPUTATION SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO	1	APPLY LOAD CASE	1
INCREMENT NO	2	PUT ON LAYER NO	1
INCREMENT NO	3	PUT ON LAYER NO	2
INCREMENT NO	4	PUT ON LAYER NO	3
INCREMENT NO	5	PUT ON LAYER NO	4
INCREMENT NO	6	PUT ON LAYER NO	5
INCREMENT NO	7	PUT ON LAYER NO	6
INCREMENT NO	8	APPLY LOAD CASE	2

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		B.C. CODE		
	X-ORD	Y-ORD	X	Y	Z
1	000	000	1	1	1
2	10 000	000	1	1	1
3	20 000	000	1	1	1
4	30 000	000	1	1	1
5	38 000	000	1	1	1
6	42 000	000	1	1	1
7	50 000	000	1	1	1
8	54 000	000	1	1	1
9	58 000	000	1	1	1
10	66 000	000	1	1	1
11	74 000	000	1	1	1
12	82 000	000	1	1	1
13	90 000	000	1	1	1
14	000	4 000	0	0	0
15	10 000	4 000	0	0	0
16	20 000	4 000	0	0	0
17	30 000	4 000	0	0	0
18	38 000	4 000	0	0	0
19	42 000	4 000	0	0	0
20	50 000	4 000	0	0	0
21	54 000	4 000	0	0	0
22	58 000	4 000	0	0	0
23	66 000	4 000	0	0	0
24	74 000	4 000	0	0	0
25	82 000	4 000	0	0	0
26	90 000	4 000	0	0	0
27	000	7 000	0	0	1
28	10 000	7 000	0	0	1
29	20 000	7 000	0	0	1
30	30 000	7 000	0	0	1
31	38 000	7 000	0	0	1
32	42 000	7 000	0	0	1
33	50 000	7 000	0	0	1
34	54 000	7 000	0	0	1
35	58 000	7 000	0	0	1
36	66 000	7 000	0	0	1
37	74 000	7 000	0	0	1
38	82 000	7 000	0	0	1
39	90 000	7 000	0	0	1
40	000	10 000	0	0	1
41	10 000	10 000	0	0	1
42	20 000	10 000	0	0	1
43	30 000	10 000	0	0	1
44	38 000	10 000	0	0	1
45	42 000	10 000	0	0	1
46	50 000	10 000	0	0	1
47	54 000	10 000	0	0	1
48	58 000	10 000	0	0	1
49	66 000	10 000	0	0	1
50	74 000	10 000	0	0	1
51	82 000	10 000	0	0	1
52	90 000	10 000	0	0	1
53	000	11 500	0	0	1
54	10 000	11 500	0	0	1
55	20 000	11 500	0	0	1
56	30 000	11 500	0	0	1
57	38 000	11 500	0	0	1
58	42 000	11 500	0	0	1
59	50 000	11 500	0	0	1
60	54 000	11 500	0	0	1
61	58 000	11 500	0	0	1
62	42 000	13 000	0	0	0
63	50 000	13 000	0	0	0
64	54 000	13 000	0	0	0
65	58 000	13 000	0	0	0
66	66 000	13 000	0	0	0
67	74 000	13 000	0	0	0
68	82 000	13 000	0	0	0
69	90 000	13 000	0	0	0
70	48 000	14 000	0	0	0
71	50 000	14 000	0	0	0
72	54 000	14 000	0	0	0
73	58 000	14 000	0	0	0
74	66 000	14 000	0	0	0
75	74 000	14 000	0	0	0
76	82 000	14 000	0	0	0
77	90 000	14 000	0	0	0
78	50 000	15 000	0	0	0
79	54 000	15 000	0	0	0
80	58 000	15 000	0	0	0
81	66 000	15 000	0	0	0
82	74 000	15 000	0	0	0
83	82 000	15 000	0	0	0
84	90 000	15 000	0	0	0
85	54 000	16 000	0	0	0
86	58 000	16 000	0	0	0
87	66 000	16 000	0	0	0
88	74 000	16 000	0	0	0
89	82 000	16 000	0	0	0
90	90 000	16 000	0	0	0
91	58 000	17 000	0	0	0
92	66 000	17 000	0	0	0
93	74 000	17 000	0	0	0
94	82 000	17 000	0	0	0
95	90 000	17 000	0	0	0

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
126		1.00	6.1

ELMT NO.	CONNECTED NODES	MATL NO.
	I J	
1	43 44	1
2	44 45	1
3	45 46	1
4	46 47	1
5	47 48	1
6	48 49	1
7	49 50	1
8	50 51	1
9	51 52	1

SOIL MATERIAL PROPERTY DATA

MATE	UNIT	WY	YOUNG'S MODULUS CONSTANT	EXPO	RATIO	BULK MODULUS CONSTANT	EXPO	STRENGTH C	PARAMETERS PHI	DPHI	KB	
1	.0880	6000	00	500	.500	1500.00	.800	00	35	00	00	50
2	.0530	40	00	300	.800	20.00	.200	05	00	00	50	
3	.0800	1000	00	400	.700	500.00	.500	50	40	00	.50	

FOUR-NODE SOLID ELEMENT DATA

ELET NO.	CONNECTED NODES	MATL NO.	ELEMENT CENTER COORDINATES
	I J K L		X-ORD Y-ORD
1	1 2 15 14	3	5.000 2.000
2	2 3 18 15	3	15.000 2.000
3	3 4 17 15	3	25.000 2.000
4	4 5 16 14	2	35.000 2.000
5	5 6 19 18	2	39.000 2.000
6	6 7 20 18	2	48.000 2.000
7	7 8 21 20	2	52.000 2.000
8	8 9 22 21	2	56.000 2.000
9	9 10 23 22	3	62.000 2.000
10	10 11 24 23	3	70.000 2.000
11	11 12 25 24	3	78.000 2.000
12	12 13 28 25	3	88.000 2.000
13	13 14 28 27	3	9.000 5.500
14	14 15 29 28	3	15.000 5.500
15	15 16 30 29	2	25.000 5.500
16	16 17 31 30	2	35.000 5.500
17	17 18 32 31	2	39.000 5.500
18	18 19 33 32	2	48.000 5.500
19	19 20 33 34	2	52.000 5.500
20	21 22 35 34	2	56.000 5.500
21	22 23 36 35	2	62.000 5.500
22	23 24 37 35	3	70.000 5.500
23	24 25 38 37	3	78.000 5.500
24	25 26 39 38	3	88.000 5.500
25	27 28 41 40	3	9.000 8.500
26	28 29 42 41	2	15.000 8.500
27	29 30 43 42	2	25.000 8.500
28	30 31 44 43	2	35.000 8.500
29	31 32 45 44	2	39.000 8.500
30	32 33 46 45	2	48.000 8.500
31	33 34 47 46	2	52.000 8.500
32	34 35 48 47	2	56.000 8.500
33	35 36 49 48	2	62.000 8.500
34	36 37 50 49	2	70.000 8.500
35	37 38 51 50	3	78.000 8.500
36	38 39 52 51	3	88.000 8.500
37	43 44 53 53	1	34.500 10.750
38	44 45 54 54	1	39.000 10.750
39	45 46 55 54	1	48.000 10.750
40	46 47 56 55	1	52.000 10.750
41	47 48 57 56	1	56.000 10.750
42	48 49 58 57	1	62.000 10.750
43	49 50 59 58	1	70.000 10.750
44	50 51 60 59	1	78.000 10.750
45	51 52 61 60	1	88.000 10.750
46	52 53 62 62	1	40.500 12.250
47	53 54 63 62	1	48.000 12.250
48	54 55 64 63	1	52.000 12.250
49	55 56 65 64	1	56.000 12.250
50	57 58 66 65	1	62.000 12.250
51	58 59 67 66	1	70.000 12.250
52	59 60 68 67	1	78.000 12.250
53	60 61 69 68	1	88.000 12.250
54	62 63 71 70	1	47.000 13.500
55	63 64 72 71	1	52.000 13.500
56	64 65 73 72	1	58.000 13.500
57	65 66 74 73	1	62.000 13.500
58	66 67 75 74	1	70.000 13.500
59	67 68 76 75	1	78.000 13.500
60	68 69 77 75	1	88.000 13.500
61	70 71 78 78	1	49.000 14.500
62	71 72 79 78	1	52.000 14.500
63	72 73 80 79	1	58.000 14.500
64	73 74 81 80	1	62.000 14.500
65	74 75 82 81	1	70.000 14.500
66	75 76 83 82	1	78.000 14.500
67	76 77 84 83	1	88.000 14.500
68	78 79 85 85	1	53.000 15.500
69	79 80 86 85	1	58.000 15.500
70	80 81 87 86	1	62.000 15.500
71	81 82 88 87	1	70.000 15.500
72	82 83 89 88	1	78.000 15.500
73	83 84 90 89	1	88.000 15.500
74	85 86 91 91	1	57.000 16.500
75	86 87 92 91	1	62.000 16.500
76	87 88 93 92	1	70.000 16.500
77	88 89 94 93	1	78.000 16.500
78	89 90 95 94	1	88.000 16.500

CONSTRUCTION LAYER INFORMATION

MODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS):

NP	X-FORCE	Y-FORCE
1 0	0	0
2 0	0	0
3 0	0	0
4 0	0	0
5 0	0	0
6 0	0	0
7 0	0	0
8 0	0	0
9 0	0	0
10 0	0	0
11 0	0	0
12 0	0	0
13 0	0	0
14 0	0	0
15 0	0	0
16 0	0	0
17 0	0	0
18 0	0	0
19 0	0	0
20 0	0	0
21 0	0	0
22 0	0	0
23 0	0	0
24 0	0	0
25 0	0	0
26 0	0	0
27 0	0	0
28 0	0	0
29 0	0	0
30 0	0	0
31 0	0	0
32 0	0	0
33 0	0	0
34 0	0	0
35 0	0	0
36 0	0	0
37 0	0	0
38 0	0	0
39 0	0	0
40 0	0	0
41 0	0	0
42 0	0	0
43 0	0	0
44 0	0	0
45 0	0	0
46 0	0	0
47 0	0	0
48 0	0	0
49 0	0	0
50 0	0	0
51 0	0	0
52 0	0	0
53 0	0	0
54 0	0	0
55 0	0	0
56 0	0	0
57 0	0	0
58 0	0	0
59 0	0	0
60 0	0	0
61 0	0	0
62 0	0	0
63 0	0	0
64 0	0	0
65 0	0	0
66 0	0	0
67 0	0	0
68 0	0	0
69 0	0	0
70 0	0	0
71 0	0	0
72 0	0	0
73 0	0	0
74 0	0	0
75 0	0	0
76 0	0	0
77 0	0	0
78 0	0	0
79 0	0	0
80 0	0	0
81 0	0	0
82 0	0	0
83 0	0	0
84 0	0	0
85 0	0	0
86 0	0	0
87 0	0	0
88 0	0	0
89 0	0	0
90 0	0	0
91 0	0	0
92 0	-4 50	0
93 0	-4 50	0
94 0	-2 00	0
95 0	0	0

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0000	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0002	.0000	.0000	.0002	15
16	.0000	.0001	.0000	.0005	.0007	.0000	.0008	16
17	.0008	.0002	.0000	.0052	.0028	.0000	.0083	17
18	.0217	.0105	.0000	.2247	.0491	.0000	.2300	18
19	.0418	.0008	.0000	.3303	.0879	.0000	.3372	19
20	.0389	.0110	.0000	.2038	.1283	.0000	.2447	20
21	.0220	.0187	.0000	.0839	.1872	.0000	.1782	21
22	.0008	.0006	.0000	.0005	.0056	.0000	.0056	22
23	.0010	.0018	.0000	.0005	.0051	.0000	.0051	23
24	.0008	.0057	.0000	.0001	.0107	.0000	.0107	24
25	.0000	.0000	.0000	.0000	.0000	.0000	.0000	25
26	.0000	.0004	.0000	.0000	.0020	.0000	.0020	26
27	.0000	.0000	.0000	.0000	.0001	.0000	.0001	27
28	.0000	.0000	.0000	.0000	.0003	.0000	.0003	28
29	.0000	.0000	.0000	.0000	.0000	.0000	.0000	29
30	.0222	.0000	.0000	.27.3	.0775	.0000	.2828	30
31	.0398	.0204	.0000	.3788	.0859	.0000	.3864	31
32	.0538	.0056	.0000	.4124	.1205	.0000	.4297	32
33	.0744	.0171	.0000	.3880	.2884	.0000	.4433	33
34	.0840	.0232	.0000	.3018	.2693	.0000	.4044	34
35	.0802	.0358	.0000	.2107	.1901	.0000	.2838	35
36	.0007	.0044	.0000	.0008	.0133	.0000	.0133	36
37	.0014	.0109	.0000	.0007	.0184	.0000	.0184	37
38	.0022	.0038	.0000	.0008	.0110	.0000	.0111	38
39	.0000	.0018	.0000	.0000	.0032	.0000	.0032	39
40	.0000	.0000	.0000	.0000	.0000	.0000	.0000	40
41	.0000	.0000	.0000	.0000	.0000	.0000	.0000	41
42	.0155	.0088	.0000	.1128	.0844	.0000	.1300	42
43	.0387	.0122	.0000	.3324	.1318	.0000	.3585	43
44	.0381	.0274	.0000	.3248	.0668	.0000	.3314	44
45	.0400	.0070	.0000	.3028	.1720	.0000	.3442	45
46	.0330	.0198	.0000	.2543	.3624	.0000	.4427	46
47	.0277	.0268	.0000	.2186	.3658	.0000	.4267	47
48	.0254	.0452	.0000	.1822	.3103	.0000	.3599	48
49	.0214	.0828	.0000	.0741	.2282	.0000	.2441	49
50	.0021	.0255	.0000	.0258	.0462	.0000	.0520	50
51	.0004	.0087	.0000	.0123	.0182	.0000	.0203	51
52	.0000	.0022	.0000	.0000	.0787	.0000	.0787	52
53	.0380	.0224	.0000	.5121	.1254	.0000	.5272	53
54	.0381	.0071	.0000	.4303	.1355	.0000	.4518	54
55	.0248	.0202	.0000	.2337	.3821	.0000	.4228	55
56	.0225	.0275	.0000	.1721	.3737	.0000	.4114	56
57	.0200	.0488	.0000	.1884	.3383	.0000	.3819	57
58	.0122	.0880	.0000	.0572	.2582	.0000	.2844	58
59	.0084	.0288	.0000	.0442	.0967	.0000	.1084	59
60	.0031	.0038	.0000	.0101	.0133	.0000	.0187	60
61	.0000	.0021	.0000	.0000	.0744	.0000	.0744	61
62	.0310	.0072	.0000	.0878	.0031	.0000	.0879	62
63	.0323	.0203	.0000	.0863	.1883	.0000	.1891	63
64	.0312	.0280	.0000	.0884	.2033	.0000	.2210	64
65	.0171	.0488	.0000	.0713	.2288	.0000	.2377	65
66	.0185	.0888	.0000	.0825	.1847	.0000	.1950	66
67	.0158	.0318	.0000	.0772	.1188	.0000	.1418	67
68	.0081	.0044	.0000	.0168	.0378	.0000	.0408	68
69	.0000	.0021	.0000	.0000	.0288	.0000	.0288	69
70	.0274	.0063	.0000	.0814	.0515	.0000	.0802	70
71	.0289	.0202	.0000	.0871	.1209	.0000	.1363	71
72	.0327	.0290	.0000	.0722	.1900	.0000	.1865	72
73	.0384	.0585	.0000	.0810	.1851	.0000	.2021	73
74	.0289	.0873	.0000	.0855	.1821	.0000	.1758	74
75	.0212	.0323	.0000	.0397	.1388	.0000	.1424	75
76	.0082	.0052	.0000	.0101	.0107	.0000	.0147	76
77	.0000	.0018	.0000	.0000	.0033	.0000	.0033	77
78	.0281	.0198	.0000	.0479	.0818	.0000	.0948	78
79	.0288	.0232	.0000	.0483	.1033	.0000	.1141	79
80	.0322	.0544	.0000	.0323	.1248	.0000	.1427	80
81	.4091	.2258	.0000	.4328	.2758	.0000	.5128	81
82	.2951	.2648	.0000	.2830	.3518	.0000	.4515	82
83	.7491	.0501	.0000	.7478	.0842	.0000	.7495	83
84	.0000	.0821	.0000	.0000	.0848	.0000	.0848	84
85	.0230	.0285	.0000	.0291	.0480	.0000	.0581	85
86	.0267	.0525	.0000	.0340	.0781	.0000	.0834	86
87	.0254	.3878	.0000	.0111	.3822	.0000	.3835	87
88	.0037	.5249	.0000	.0010	.5815	.0000	.5815	88
89	.5908	.1018	.0000	.5900	.1040	.0000	.5991	89
90	.0000	.2415	.0000	.0000	.2395	.0000	.2395	90
91	.0138	.0519	.0000	.0138	.0519	.0000	.0637	91
92	.0105	.3591	.0000	.0105	.3591	.0000	.3592	92
93	.0184	.5298	.0000	.0184	.5298	.0000	.5301	93
94	.2819	.2887	.0000	.2819	.2887	.0000	.3708	94
95	.0000	.3883	.0000	.0000	.3883	.0000	.3883	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO	AXIAL FORCE
INCREMENTAL VALUES	
1	.0071
2	.0142
3	.0409
4	.0868
5	.2104
6	.2885
7	.0264
8	.0052
TOTAL VALUES	
1	1780
2	4357
3	7273
4	6400
5	11221
6	13218
7	10245

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-2	GAMMAX	ELE
1	528.2	331.8	228.5	158	.001	.000	.003	.002	-.001	.003	1
2	523.8	334.2	225.2	183	.001	.003	.013	.013	-.002	.015	2
3	513.7	322.9	222.2	158	.024	-.024	.055	.036	-.037	.073	3
4	6	17.2	2	494	1.828	-.848	2.487	2.348	-1.163	3.808	4
5	6	17.1	2	494	.880	.238	7.912	4.527	-3.412	7.939	5
6	7	17.3	2	494	.760	.340	7.068	4.788	-3.045	7.640	6
7	.7	18.4	.2	493	-1.500	3.858	3.871	4.301	-2.145	6.447	7
8	.8	18.9	.2	484	-1.041	2.035	-.839	2.091	-1.097	3.188	8
9	488.5	344.2	180.2	224	.000	.133	.011	.133	.000	.134	9
10	478.5	369.8	183.7	238	-.008	.137	.035	.199	-.008	.208	10
11	475.1	371.8	181.1	243	-.007	.203	-.083	.207	-.012	.219	11
12	488.8	381.4	208.9	206	.011	.098	-.044	.100	.008	.094	12
13	439.9	288.1	188.1	122	.001	.002	-.001	.002	.001	.002	13
14	488.8	292.4	207.8	120	.012	-.007	.024	.019	-.013	.030	14
15	2.2	13.7	1.8	472	1.372	-1.280	4.064	2.477	-2.385	4.842	15
16	4.3	15.9	1.8	483	2.721	-1.883	6.564	4.424	-3.570	7.988	16
17	1.1	16.5	2	489	1.180	.283	6.833	4.088	-2.838	6.895	17
18	.6	17.1	2	484	-1.148	3.048	8.215	4.284	-2.388	8.682	18
19	6	17.1	2	494	-2.187	4.038	6.472	5.418	-3.848	8.988	19
20	.7	18.2	2	494	-2.177	4.944	4.246	5.528	-2.782	8.281	20
21	6	17.2	2	484	-1.322	3.211	2.370	3.502	-1.813	5.118	21
22	488.3	381.2	178.8	282	-.007	.282	.048	.264	-.008	.283	22
23	407.0	308.3	185.1	232	-.007	.237	-.070	.242	-.012	.254	23
24	406.2	273.9	171.0	188	.018	.111	-.054	.118	.009	.109	24
25	408.9	229.7	187.1	093	-.001	.000	.001	.001	-.001	.002	25
26	6	13.4	1	498	5.78	-1.073	1.520	8.773	-1.370	2.242	26
27	6	13.8	1	494	2.481	-1.978	2.149	2.898	-2.225	4.923	27
28	4.0	13.8	1.6	480	8.19	-.886	5.86	8.79	-.648	1.525	28
29	17.9	18.9	5.8	295	0.88	1.177	9.79	1.388	-.080	1.458	29
30	5	15.0	2	484	-.862	2.474	1.410	2.628	-.813	5.438	30
31	8	15.8	2	484	-1.101	3.225	-2.954	3.881	-1.887	5.238	31
32	8	15.8	2	484	-1.804	3.812	-3.528	4.152	-2.144	6.298	32
33	7	17.8	2	494	-1.873	5.538	-.529	5.546	-1.882	7.428	33
34	7	17.2	2	484	-.428	3.380	.848	4.030	-.478	4.507	34
35	301.7	270.0	117.7	202	-.025	.532	.620	.522	-.150	.747	35
36	389.6	219.8	187.5	142	-.072	1.344	.589	1.387	-.125	1.522	36
37	11.9	388.8	4.0	485	-.008	.289	17.772	9.013	-8.782	17.774	37
38	220.2	424.7	78.3	408	-.783	1.895	18.807	10.664	-9.432	20.087	38
39	323.2	438.1	118.4	385	-1.448	4.284	10.783	7.536	-4.887	12.223	39
40	122.3	412.7	42.2	449	-1.118	5.027	2.105	5.202	-1.293	8.498	40
41	232.8	428.4	83.3	402	-.822	4.886	-.584	5.000	-.838	5.838	41
42	215.8	424.1	78.5	410	-.847	4.134	-.939	4.177	-.890	5.187	42
43	11.9	389.8	4.0	485	-.973	3.583	-3.933	4.315	-1.704	6.019	43
44	748.8	670.1	281.8	282	.186	1.882	-2.888	2.697	-.649	3.348	44
45	1472.4	872.0	684.8	128	.041	.013	.316	.185	-.131	.317	45
46	11.9	388.8	4.0	485	-.040	.070	-.230	.142	-.113	.268	46
47	11.9	388.8	4.0	485	-.070	.086	.698	.388	-.350	.715	47
48	311.8	438.6	113.8	370	-.140	.184	.137	.198	-.184	.351	48
49	11.9	388.8	4.0	485	-.385	.290	.219	.307	-.383	.690	49
50	289.9	431.1	97.3	387	-.283	3.20	.081	.322	-.285	3.817	50
51	1084.7	887.9	455.4	189	-.080	2.621	1.554	2.840	-.259	3.099	51
52	894.9	512.8	233.8	272	-.480	3.428	1.882	3.598	-.830	4.229	52
53	881.4	471.0	217.2	289	-.125	1.828	.283	1.835	-.135	1.970	53
54	1	4.0	0	485	.028	.021	.367	.158	-.181	.370	54
55	387.7	448.3	149.1	334	-.044	.003	.149	.101	-.054	.185	55
56	1	4.0	0	485	-.087	.417	1.800	1.098	-.778	1.872	56
57	1	4.0	0	485	-.154	.844	1.742	1.134	-.744	1.877	57
58	1400.9	822.9	625.4	120	-.285	2.800	.801	2.851	-.348	3.197	58
59	938.0	680.8	387.4	207	-.308	2.747	-.514	2.789	-.331	3.100	59
60	380.7	447.4	148.2	337	-.111	.107	.038	.109	-.112	.221	60
61	1	4.0	0	485	.118	-.058	1.037	.593	-.498	1.051	61

82	.1	4.0	.0	485	054	-.007	.057	089	-.022	.091	82
83	434.4	453.8	184.7	318	138	-.081	.124	155	-.098	.253	83
84	.1	4.0	.0	485	2.271	8.852	18.906	14.775	-5.652	20.427	84
85	.1	4.0	.0	485	-4.570	21.185	3.822	21.327	-4.712	26.039	85
86	.1	4.0	.0	485	-3.025	16.508	-55.887	38.248	-22.755	59.014	86
87	.1	4.0	.0	485	4.812	-2.222	-38.793	20.890	-18.500	38.390	87
88	.1	4.0	.0	485	-.008	.017	-.302	.157	-.146	.303	88
89	325.4	438.4	119.2	364	108	-.087	.088	117	-.098	.213	89
90	.1	4.0	.0	485	2.337	8.515	-18.584	12.977	-4.125	17.102	90
91	.1	4.0	.0	485	-4.810	20.509	-3.158	21.008	-4.707	28.714	91
92	.1	4.0	.0	485	-5.585	18.288	18.007	19.884	-9.612	29.568	92
93	.1	4.0	.0	485	8.385	-4.881	4.878	8.801	-8.297	14.097	93
94	.1	4.0	.0	485	.082	-.052	-3.481	1.752	-1.712	3.484	94
95	875.2	508.8	274.2	231	-.001	.034	.118	.077	-.044	.122	95
96	273.1	431.5	98.8	345	-.218	.302	-.118	.347	-.284	.611	96
97	.1	4.0	.0	485	-5.379	8.587	8.862	8.309	-7.091	15.400	97
98	.1	4.0	.0	485	5.517	-.886	8.958	7.821	-3.190	11.011	98

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	YAU-XY	SIG-1	SIG-3	YAU-MAX	YMEYA	SIG1/SIG3	LEVEL	ELE
1	.247	484	.007	.484	.247	.119	1.756	1.961	.078	1
2	.257	522	.028	.535	.284	.141	5.831	2.106	.092	2
3	.350	366	.128	.484	.220	.129	43.066	2.124	.087	3
4	.377	545	.005	.547	.376	.087	1.894	1.452	1.701	4
5	.381	528	.018	.540	.380	.090	5.172	1.500	1.799	5
6	.424	657	.018	.689	.423	.088	4.895	1.424	1.861	6
7	.434	731	-.005	.731	.433	.088	2.474	1.381	2.031	7
8	.332	528	-.002	.525	.332	.096	-4.478	1.579	1.825	8
9	.342	.082	.018	1.053	.347	.386	1.442	3.082	1.111	9
10	.420	.418	.088	1.424	.415	.504	3.857	3.428	.277	10
11	.437	.448	.121	1.468	.435	.516	8.728	3.466	.283	11
12	.348	884	-.091	.880	.335	.274	-9.701	2.844	1.64	12
13	.142	.280	-.002	.280	.142	.089	-1.157	1.873	.082	13
14	.184	.243	.048	.270	.187	.056	29.195	1.721	.042	14
15	.118	.182	.020	.188	.113	.038	14.860	1.888	.751	15
16	.248	.270	.033	.294	.224	.036	35.459	1.314	.702	16
17	.307	.381	.014	.394	.304	.045	5.467	1.284	.893	17
18	.374	.480	.011	.481	.373	.054	5.741	1.288	1.078	18
19	.388	.478	.013	.481	.384	.056	8.897	1.320	1.187	19
20	.523	.642	.010	.644	.522	.061	4.710	1.233	1.217	20
21	.372	.481	.005	.481	.372	.055	2.596	1.294	1.094	21
22	.424	1.801	.088	1.607	.417	.595	4.273	3.854	.328	22
23	.314	1.235	-.114	1.249	.300	.475	-8.970	4.188	.285	23
24	.241	.627	-.093	.649	.218	-.215	-12.858	2.957	1.46	24
25	.082	.081	.002	.081	.082	.024	2.730	2.163	.021	25
26	.008	-.002	.006	.010	-.008	.018	84.875	-2.023	1.000	26
27	.157	.016	.034	.157	.057	.085	74.461	20.841	1.301	27
28	.081	.030	-.020	.088	.023	.032	-71.028	3.802	.848	28
29	.177	.182	-.002	.183	.177	.003	-18.710	1.036	.063	29
30	.201	.318	-.006	.318	.201	.058	-2.863	1.566	1.174	30
31	.287	.389	-.008	.380	.258	.082	-4.437	1.404	1.031	31
32	.226	.342	-.015	.344	.232	.055	-8.007	1.475	1.109	32
33	.520	.838	-.003	.838	.520	.058	-1.266	1.222	1.153	33
34	.508	.608	.004	.608	.508	.049	2.340	1.181	.974	34
35	.438	.784	.007	.784	.438	.081	.306	4.038	.388	35
36	.045	.266	-.008	.288	.045	.111	-2.010	5.909	.098	36
37	.022	.047	-.008	.108	-.028	.070	-39.829	-2.888	-1.000	37
38	.023	.111	.038	.125	.008	.068	20.299	13.883	4.819	38
39	.028	.308	.078	.327	.008	.189	14.648	38.227	13.088	39
40	-.088	.224	-.084	.232	-.108	.170	-9.315	-2.182	-1.000	40
41	-.187	.448	-.085	.481	-.182	.308	5.182	-2.787	-1.000	41
42	-.438	.491	-.089	.498	-.440	.468	-4.267	-1.127	-1.000	42
43	.151	.729	-.127	.755	.124	-.316	-11.850	8.085	1.890	43
44	.174	1.411	-.098	1.419	.168	.527	-4.486	8.555	2.808	44
45	-.002	.366	.352	.578	-.215	.397	31.212	-2.889	-1.000	45
46	-.020	-.004	.005	-.002	-.022	.010	16.747	.110	-1.000	46
47	.045	.171	.112	.227	-.021	.129	30.254	-11.526	-1.000	47
48	.024	.440	.194	.518	-.042	.281	21.849	-11.812	-1.000	48
49	-.385	-.108	-.127	-.051	-.441	.195	-22.388	.118	-1.000	49
50	-.185	.484	-.245	.564	-.285	.415	18.081	-2.126	-1.000	50
51	-.470	1.085	-.715	1.380	-.751	1.051	-21.448	-1.798	-1.000	51
52	-.321	.791	-.249	.844	-.374	.609	12.088	-2.287	-1.000	52
53	.247	.376	.088	.383	.281	.323	3.322	1.489	-1.000	53
54	.064	.051	.080	.138	-.042	.080	43.871	-3.287	-1.000	54
55	.277	.240	.254	.513	.005	.254	47.084	107.805	39.702	55
56	.101	.268	.057	.268	.084	.101	17.204	3.423	.901	56
57	.070	.189	-.087	.251	.018	.117	-28.188	14.275	4.874	57
58	-.774	1.172	-.332	1.188	-.831	1.010	-9.634	-1.430	-1.000	58
59	-1.392	.509	-.205	.620	-1.413	1.021	5.801	-.446	-1.000	59
60	-.302	.307	.056	.312	-.307	.310	5.190	-1.015	-1.000	60
61	.011	.018	.002	.020	.010	.008	15.306	1.883	.388	61
62	.133	.102	.084	.213	.023	.085	49.732	8.407	3.125	62
63	.720	-.089	.255	.794	-.182	.478	73.902	-4.887	-1.000	63
64	.474	.589	-.075	.610	.432	.088	-28.882	1.411	1.53	64
65	.683	.707	.002	.707	.683	.013	2.327	1.088	.021	65
66	.547	.579	-.022	.591	.535	.028	-26.867	1.103	.038	66
67	.201	.295	-.018	.298	.198	.050	-9.112	1.504	1.88	67
68	.041	.021	.014	.048	.014	.017	82.221	3.501	.929	68
69	.385	.113	.122	.385	.145	.284	78.255	-2.687	1.000	69
70	.400	.470	-.034	.484	.386	.049	-22.299	1.254	.095	70
71	.857	.694	-.001	.894	.857	.019	-1.827	1.057	.021	71
72	.420	.458	.007	.457	.418	.019	10.855	1.052	.036	72
73	.220	.283	.002	.283	.220	.014	4.312	1.749	.085	73
74	.010	.019	.001	.019	.010	.004	4.618	1.841	.312	74
75	.087	.301	.319	.520	-.142	.336	35.725	-3.718	-1.000	75
76	.137	.907	-.315	.994	-.225	.610	-15.548	-4.424	-1.000	76
77	.081	.087	.004	.088	.080	.014	3.107	1.459	1.71	77
78	.204	.218	.004	.217	.203	.007	15.490	1.086	.025	78

SSTIPN: 1 LAYER OF GEOTEXTILE, P=4000 PPI, EXTENDED, T=1000

TOTAL NUMBER OF NODES-----	95
NUMBER OF BAR ELEMENTS-----	11
NUMBER OF DIFF BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO OF INTERFACE ELE IN PREEXIST PART-----	0
NUMBER OF INTERFACE ELE IN FOUNDATION-----	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	38
NUMBER OF NODES IN FOUNDATION-----	52
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	4
NUMBER OF LOAD CASES-----	2

CALING FACTOR ----- 1 00000

ATMOSPHERIC PRESSURE --- 1 05800

UNIT WEIGHT OF WATER --- 02120

CONSTRUCTION SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO 1	APPLY LOAD CASE 1
INCREMENT NO 2	PUT ON LAYER NO 1
INCREMENT NO 3	PUT ON LAYER NO 2
INCREMENT NO 4	PUT ON LAYER NO 3
INCREMENT NO 5	PUT ON LAYER NO 4
INCREMENT NO 6	PUT ON LAYER NO 5
INCREMENT NO 7	PUT ON LAYER NO 6
INCREMENT NO 8	APPLY LOAD CASE 2

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		B.C. CODE		
	X-ORD	Y-ORD	X	Y	Z
1	000	000	1	1	1
2	10.000	000	1	1	1
3	20.000	000	1	1	1
4	30.000	000	1	1	1
5	38.000	000	1	1	1
6	42.000	000	1	1	1
7	50.000	000	1	1	1
8	54.000	000	1	1	1
9	58.000	000	1	1	1
10	63.000	000	1	1	1
11	74.000	000	1	1	1
12	82.000	000	1	1	1
13	90.000	000	1	1	1
14	10.000	4.000	0	0	0
15	10.000	4.000	0	0	0
16	20.000	4.000	0	0	0
17	30.000	4.000	0	0	0
18	38.000	4.000	0	0	0
19	42.000	4.000	0	0	0
20	50.000	4.000	0	0	0
21	54.000	4.000	0	0	0
22	58.000	4.000	0	0	0
23	66.000	4.000	0	0	0
24	74.000	4.000	0	0	0
25	82.000	4.000	0	0	0
26	90.000	4.000	1	0	1
27	10.000	7.000	1	0	1
28	10.000	7.000	0	0	0
29	20.000	7.000	0	0	0
30	30.000	7.000	0	0	0
31	38.000	7.000	0	0	0
32	42.000	7.000	0	0	0
33	50.000	7.000	0	0	0
34	54.000	7.000	0	0	0
35	58.000	7.000	0	0	0
36	66.000	7.000	0	0	0
37	74.000	7.000	0	0	0
38	82.000	7.000	0	0	0
39	90.000	7.000	1	0	1
40	10.000	10.000	1	0	1
41	10.000	10.000	0	0	0
42	20.000	10.000	0	0	0
43	30.000	10.000	0	0	0
44	38.000	10.000	0	0	0
45	42.000	10.000	0	0	0
46	50.000	10.000	0	0	0
47	54.000	10.000	0	0	0
48	58.000	10.000	0	0	0
49	66.000	10.000	0	0	0
50	74.000	10.000	0	0	0
51	82.000	10.000	0	0	0
52	90.000	10.000	1	0	1
53	10.000	11.500	0	0	0
54	20.000	11.500	0	0	0
55	30.000	11.500	0	0	0
56	38.000	11.500	0	0	0
57	42.000	11.500	0	0	0
58	50.000	11.500	0	0	0
59	54.000	11.500	0	0	0
60	66.000	11.500	0	0	0
61	74.000	11.500	1	0	1
62	42.000	13.000	0	0	0
63	50.000	13.000	0	0	0
64	54.000	13.000	0	0	0
65	58.000	13.000	0	0	0
66	66.000	13.000	0	0	0
67	74.000	13.000	0	0	0
68	82.000	13.000	0	0	0
69	90.000	13.000	1	0	1
70	10.000	14.000	0	0	0
71	20.000	14.000	0	0	0
72	30.000	14.000	0	0	0
73	38.000	14.000	0	0	0
74	42.000	14.000	0	0	0
75	50.000	14.000	0	0	0
76	54.000	14.000	0	0	0
77	60.000	14.000	1	0	1
78	50.000	15.000	0	0	0
79	54.000	15.000	0	0	0
80	58.000	15.000	0	0	0
81	66.000	15.000	0	0	0
82	74.000	15.000	0	0	0
83	82.000	15.000	0	0	0
84	90.000	15.000	1	0	1
85	14.000	16.000	0	0	0
86	24.000	16.000	0	0	0
87	34.000	16.000	0	0	0
88	44.000	16.000	0	0	0
89	54.000	16.000	0	0	0
90	64.000	16.000	1	0	1
91	74.000	17.000	0	0	0
92	84.000	17.000	0	0	0
93	94.000	17.000	0	0	0
94	82.000	17.000	0	0	0
95	90.000	17.000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
	120.	1.00	0.6

ELMY NO.	CONNECTED NODES I J	MATL NO.
1	41 42	1
2	42 43	1
3	43 44	1
4	44 45	1
5	45 46	1
6	46 47	1
7	47 48	1
8	48 49	1
9	49 50	1
10	50 51	1
11	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S MODULUS CONSTANT	MODULUS EXPONENT	RATIO	BULK MODULUS CONSTANT	MODULUS EXPONENT	STRENGTH C	PARAMETERS PHI	DPHI	K0
1	0800	8000.00	1.00	.800	1500.00	1.00	.00	35.00	.00	.50
2	0830	40.00	.300	.800	20.00	.200	.05	.00	.00	.50
3	0800	1000.00	.400	.700	500.00	.500	.50	40.00	.00	.50

FOUR NODES SOLID ELEMENT DATA

ELEM NO	CONNECTED NODES I J K L	MATL NO	ELEMNT CENTER COORDINATES X-ORD Y-ORD
1	1 2 15 14	3	5.000 2.000
2	2 3 13 15	3	15.000 2.000
3	3 4 17 16	3	25.000 2.000
4	4 5 18 17	2	35.000 2.000
5	5 6 19 18	2	39.000 2.000
6	6 7 20 18	2	43.000 2.000
7	7 8 21 20	2	52.000 2.000
8	8 9 22 21	2	56.000 2.000
9	9 10 23 22	3	62.000 2.000
10	10 11 24 23	3	70.000 2.000
11	11 12 25 24	3	78.000 2.000
12	12 13 26 25	3	86.000 2.000
13	13 14 27 26	3	8.000 5.500
14	14 15 28 27	3	15.000 5.500
15	15 16 29 28	2	25.000 5.500
16	16 17 30 28	2	33.000 5.500
17	17 18 31 30	2	39.000 5.500
18	18 19 32 31	2	46.000 5.500
19	20 21 34 33	2	52.000 5.500
20	21 22 35 34	2	58.000 5.500
21	22 23 36 35	2	62.000 5.500
22	23 24 37 36	3	70.000 5.500
23	24 25 38 37	3	78.000 5.500
24	25 26 39 38	3	86.000 5.500
25	27 28 41 40	3	5.000 8.500
26	28 29 42 41	2	15.000 8.500
27	29 30 43 42	2	25.000 8.500
28	30 31 44 43	2	33.000 8.500
29	31 32 45 44	2	39.000 8.500
30	32 33 46 45	2	46.000 8.500
31	33 34 47 46	2	52.000 8.500
32	34 35 48 47	2	58.000 8.500
33	35 36 49 48	2	62.000 8.500
34	36 37 50 48	2	70.000 8.500
35	37 38 51 50	3	78.000 8.500
36	38 39 52 51	3	86.000 8.500
37	43 44 53 53	1	34.500 10.750
38	44 45 54 53	1	39.000 10.750
39	45 46 55 54	1	46.000 10.750
40	46 47 56 55	1	52.000 10.750
41	47 48 57 56	1	58.000 10.750
42	48 49 58 57	1	62.000 10.750
43	49 50 59 58	1	70.000 10.750
44	50 51 60 59	1	78.000 10.750
45	51 52 61 60	1	86.000 10.750
46	53 54 62 62	1	40.500 12.250
47	54 55 63 62	1	46.000 12.250
48	55 56 64 63	1	52.000 12.250
49	56 57 65 64	1	58.000 12.250
50	57 58 66 65	1	62.000 12.250
51	58 59 67 66	1	70.000 12.250
52	59 60 68 67	1	78.000 12.250
53	60 61 69 68	1	86.000 12.250
54	62 63 71 70	1	47.000 13.500
55	63 64 72 71	1	52.000 13.500
56	64 65 73 72	1	58.000 13.500
57	65 66 74 73	1	62.000 13.500
58	66 67 75 74	1	70.000 13.500
59	67 68 76 75	1	78.000 13.500
60	68 69 77 76	1	86.000 13.500
61	70 71 78 78	1	48.000 14.500
62	71 72 79 78	1	52.000 14.500
63	72 73 80 79	1	58.000 14.500
64	73 74 81 80	1	62.000 14.500
65	74 75 82 81	1	70.000 14.500
66	75 76 83 82	1	78.000 14.500
67	76 77 84 83	1	86.000 14.500
68	77 78 85 85	1	53.000 15.500
69	78 79 86 85	1	58.000 15.500
70	80 81 87 86	1	62.000 15.500
71	81 82 88 87	1	70.000 15.500
72	82 83 89 88	1	78.000 15.500
73	83 84 90 89	1	86.000 15.500
74	84 85 91 91	1	57.000 16.500
75	85 86 92 91	1	62.000 16.500
76	86 87 93 92	1	70.000 16.500
77	87 88 94 93	1	78.000 16.500
78	88 89 95 94	1	86.000 16.500

```

*****
*****
*****
LOAD CASE ----- 2
*****
*****

```

LARGEST ELE. NO IN THIS INCREMENT 76

LARGEST N P NO IN THIS INCREMENT 92

BAND WIDTH----- 40

TOTAL NUMBER OF EQUATIONS----- 222

NUMBER OF EQUATIONS IN BLOCK----- 84

NUMBER OF BLOCKS----- 3

NUMBER OF N.P. FORCE CARDS----- 3

NUMBER OF PRESSURE CARDS----- 0

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP X-FORCE Y-FORCE

NP	X-FORCE	Y-FORCE
1 0	0.	0.
2 0	0.	0.
3 0	0.	0.
4 0	0.	0.
5 0	0.	0.
6 0	0.	0.
7 0	0.	0.
8 0	0.	0.
9 0	0.	0.
10 0	0.	0.
11 0	0.	0.
12 0	0.	0.
13 0	0.	0.
14 0	0.	0.
15 0	0.	0.
16 0	0.	0.
17 0	0.	0.
18 0	0.	0.
19 0	0.	0.
20 0	0.	0.
21 0	0.	0.
22 0	0.	0.
23 0	0.	0.
24 0	0.	0.
25 0	0.	0.
26 0	0.	0.
27 0	0.	0.
28 0	0.	0.
29 0	0.	0.
30 0	0.	0.
31 0	0.	0.
32 0	0.	0.
33 0	0.	0.
34 0	0.	0.
35 0	0.	0.
36 0	0.	0.
37 0	0.	0.
38 0	0.	0.
39 0	0.	0.
40 0	0.	0.
41 0	0.	0.
42 0	0.	0.
43 0	0.	0.
44 0	0.	0.
45 0	0.	0.
46 0	0.	0.
47 0	0.	0.
48 0	0.	0.
49 0	0.	0.
50 0	0.	0.
51 0	0.	0.
52 0	0.	0.
53 0	0.	0.
54 0	0.	0.
55 0	0.	0.
56 0	0.	0.
57 0	0.	0.
58 0	0.	0.
59 0	0.	0.
60 0	0.	0.
61 0	0.	0.
62 0	0.	0.

63 0. 0
 64 0. 0
 65 0. 0
 66 0. 0
 67 0. 0
 68 0. 0
 69 0. 0
 70 0. 0
 71 0. 0
 72 0. 0
 73 0. 0
 74 0. 0
 75 0. 0
 76 0. 0
 77 0. 0
 78 0. 0
 79 0. 0
 80 0. 0
 81 0. 0
 82 0. 0
 83 0. 0
 84 0. 0
 85 0. 0
 86 0. 0
 87 0. 0
 88 0. 0
 89 0. 0
 90 0. 0
 91 0. 0
 92 0. -4.80
 93 0. -4.80
 94 0. -2.00
 95 0. 0

LOAD CASE : 2 ITERATION : 2

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0001	.0000	.0000	.0010	.0000	.0010	14
15	.0001	.0001	.0000	.0008	.0008	.0000	.0008	15
16	.0000	.0001	.0000	.0004	.0008	.0000	.0007	16
17	.0005	.0002	.0000	.0048	.0023	.0000	.0053	17
18	.0177	.0000	.0000	.1889	.0289	.0000	.1908	18
19	.0310	.0013	.0000	.2852	.0742	.0000	.2754	19
20	.0277	.0089	.0000	.1859	.1232	.0000	.1887	20
21	.0179	.0158	.0000	.0889	.1413	.0000	.1536	21
22	.0004	.0008	.0000	.0002	.0048	.0000	.0049	22
23	.0008	.0018	.0000	.0001	.0047	.0000	.0047	23
24	.0008	.0058	.0000	.0015	.0107	.0000	.0108	24
25	.0022	.0024	.0000	.0024	.0087	.0000	.0088	25
26	.0000	.0004	.0000	.0000	.0028	.0000	.0028	26
27	.0000	.0002	.0000	.0000	.0018	.0000	.0018	27
28	.0002	.0001	.0000	.0020	.0010	.0000	.0023	28
29	.0001	.0001	.0000	.0003	.0008	.0000	.0006	29
30	.0143	.0084	.0000	.1411	.0638	.0000	.1510	30
31	.0267	.0118	.0000	.2328	.0818	.0000	.2383	31
32	.0420	.0018	.0000	.2978	.1285	.0000	.3235	32
33	.0598	.0137	.0000	.2577	.2405	.0000	.3525	33
34	.0887	.0187	.0000	.2189	.2373	.0000	.3228	34
35	.0866	.0300	.0000	.1535	.1840	.0000	.2248	35
36	.0002	.0038	.0000	.0020	.0124	.0000	.0126	36
37	.0019	.0113	.0000	.0027	.0188	.0000	.0200	37
38	.0025	.0041	.0000	.0025	.0108	.0000	.0111	38
39	.0000	.0016	.0000	.0000	.0048	.0000	.0048	39
40	.0000	.0002	.0000	.0000	.0025	.0000	.0025	40
41	.0008	.0002	.0000	.0088	.0020	.0000	.0088	41
42	.0120	.0018	.0000	.0752	.0188	.0000	.0777	42
43	.0230	.0137	.0000	.1408	.0943	.0000	.1685	43
44	.0247	.0123	.0000	.1512	.0418	.0000	.1589	44
45	.0288	.0038	.0000	.1507	.1701	.0000	.2273	45
46	.0274	.0187	.0000	.1337	.3281	.0000	.3524	46
47	.0244	.0224	.0000	.1113	.3231	.0000	.3417	47
48	.0221	.0375	.0000	.0872	.2808	.0000	.2748	48
49	.0584	.0733	.0000	.0300	.2035	.0000	.2058	49
50	.0008	.0278	.0000	.0027	.0487	.0000	.0488	50
51	.0001	.0038	.0000	.0014	.0128	.0000	.0128	51
52	.0000	.0023	.0000	.0000	.0078	.0000	.0078	52
53	.0239	.0123	.0000	.1280	.0637	.0000	.1513	53
54	.0238	.0039	.0000	.2117	.1309	.0000	.2488	54
55	.0240	.0189	.0000	.1537	.2888	.0000	.3380	55
56	.0224	.0239	.0000	.1155	.3251	.0000	.3450	56
57	.0124	.0388	.0000	.0831	.2883	.0000	.2981	57
58	.0082	.0747	.0000	.0389	.2329	.0000	.2359	58
59	.0088	.0288	.0000	.0433	.0948	.0000	.1043	59
60	.0027	.0040	.0000	.0037	.0119	.0000	.0124	60

61	.0000	0025	.0000	.0000	.0059	0000	0059	61
62	.0214	0038	.0000	.0539	.0220	0000	0522	62
63	.0222	.0189	.0000	.0555	.1348	.0000	1462	63
64	.0238	.0244	.0000	.0593	.1858	.0000	1799	64
65	.0269	.0381	.0000	.0630	.1852	0000	1958	65
66	.0183	.0782	.0000	.0484	.1843	0000	1712	66
67	.0121	.0314	.0000	.0288	.1248	.0000	1278	67
68	.0082	.0047	.0000	.0076	.0110	.0000	0134	68
69	0000	0027	0000	.0000	.0038	.0000	0030	69
70	.0193	.0084	.0000	.0393	.0470	.0000	0613	70
71	.0206	.0188	.0000	.0423	.0887	.0000	0962	71
72	.0216	.0244	.0000	.0464	.1175	.0000	1260	72
73	.0237	.0378	.0000	.0504	.1385	0000	1455	73
74	.0228	.0770	.0000	.0498	.1428	0000	1510	74
75	.0193	.0317	.0000	.0300	.1383	.0000	1388	75
76	.0090	.0048	.0000	.0104	.0102	.0000	0146	76
77	0000	.0022	.0000	.0000	.0025	.0000	.0028	77
78	.0191	.0188	.0000	.0287	.0888	.0000	0988	78
79	.0191	.0247	.0000	.0303	.0888	.0000	0988	79
80	.0216	.0362	.0000	.0346	.0862	0000	0928	80
81	.4070	.2112	.0000	.4212	.2503	0000	4900	81
82	2807	.2864	.0000	2820	.3277	.0000	4323	82
83	.7820	.0492	.0000	.7804	.0833	.0000	7823	83
84	.0000	.0824	.0000	.0000	.0691	.0000	0891	84
85	.0187	.0248	.0000	.0201	.0419	.0000	0488	85
86	.0188	.0247	.0000	.0244	.0551	.0000	0602	86
87	.0180	.3392	.0000	.0205	.3689	.0000	3878	87
88	.0008	.5283	.0000	.0034	.5344	.0000	5344	88
89	.8899	.1008	.0000	.8887	.1032	.0000	8977	89
90	0000	.2419	.0000	.0000	.2402	.0000	2402	90
91	.0219	.0343	.0000	.0219	.0343	.0000	0405	91
92	.0188	.3401	.0000	.0188	.3401	.0000	3408	92
93	.0202	.5316	.0000	.0202	.5316	.0000	5319	93
94	.2880	.2283	.0000	.2880	.2283	.0000	3275	94
95	.0000	.3888	.0000	.0000	.3888	.0000	3888	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO.	AXIAL FORCE
INCREMENTAL VALUES	
1	.1324
2	.1328
3	.0342
4	.0432
5	.0068
6	.0887
7	.0880
8	.2388
9	.1030
10	.6092
11	.0016
TOTAL VALUES	8002
1	.7172
2	.2081
3	.0101
4	.2564
5	.6717
6	.7208
7	.8893
8	.4083
9	.0201
10	.0208
11	.0208

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	540.4	342.4	233.0	.160	.003	.006	.001	.006	.003	.003	1
2	503.8	317.8	217.6	.154	.001	.001	.018	.008	-.009	.018	2
3	523.0	327.9	226.5	.158	.022	-.022	.018	.034	-.033	.067	3
4	.6	17.2	.2	.494	1.534	-.353	2.224	2.049	-.366	2.916	4
5	.7	17.3	.2	.494	1.534	-.353	6.510	3.874	-2.635	6.510	5
6	.7	18.0	.2	.494	.883	2.467	5.570	4.092	-2.308	5.388	6
7	.7	18.4	.2	.493	-1.201	3.306	2.923	3.739	-1.634	5.373	7
8	.6	17.0	.2	.494	-.750	1.828	-.860	1.914	-.637	2.751	8
9	473.1	344.3	194.2	.218	.002	.120	-.003	.120	.002	.119	9
10	472.2	322.5	190.9	.237	-.010	.193	-.020	.193	-.010	.204	10
11	476.5	372.5	191.7	.243	-.008	.208	-.041	.212	-.013	.225	11
12	512.8	388.3	211.8	.210	.015	.108	-.048	.112	.008	.102	12
13	518.8	308.6	230.5	.127	.013	.008	.001	-.113	.008	.008	13
14	374.0	223.4	188.2	.173	-.010	-.010	-.034	-.037	-.027	.034	14
15	1.7	13.5	.2	.494	.728	-.880	1.883	1.203	-1.337	2.540	15
16	7.2	18.7	.2	.494	2.288	-1.290	2.820	2.784	-1.778	4.583	16
17	1.8	17.0	.2	.494	1.778	-.442	3.582	2.644	-1.823	3.667	17
18	6	17.2	.2	.494	-.133	2.824	3.258	3.438	-1.840	4.976	18
19	6	17.1	.2	.494	-1.887	3.555	4.534	4.329	-2.532	6.931	19
20	7	18.2	.2	.494	-1.588	4.282	2.591	4.297	-1.843	6.370	20
21	6	17.1	.2	.494	-.870	2.781	1.578	2.940	-1.129	4.089	21
22	431.4	346.2	172.4	.251	-.014	.261	.029	.261	.015	.296	22
23	408.3	307.7	165.0	.232	-.005	.238	-.109	.250	-.016	.286	23
24	460.0	312.4	193.1	.181	.031	.114	-.058	.124	.022	.102	24
25	182.1	218.3	153.2	.149	.053	-.007	.071	.069	-.028	.093	25
26	.0	.1	.0	.495	.324	-.382	1.278	.708	-.738	1.445	26
27	.0	.1	.0	.495	1.032	-.008	-.588	1.075	-.052	2.127	27
28	2.2	13.6	.8	.472	.850	-.805	-.905	.987	-.842	1.829	28
29	1.7	14.6	.8	.480	.339	-.895	-.598	1.047	-.387	1.880	29
30	3.0	15.2	.2	.454	-.319	2.189	2.831	2.748	-.894	3.745	30
31	6	18.8	.2	.494	-.788	2.888	-2.338	3.721	-1.831	5.383	31
32	6	18.5	.2	.494	-1.118	3.034	-4.594	4.058	-2.137	6.188	32
33	7	17.7	.2	.494	-1.330	4.782	-1.778	4.932	-1.470	6.403	33
34	.7	17.2	.2	.493	-.178	3.831	-.311	3.837	-.181	3.818	34
35	289.7	258.2	113.3	.279	-.007	.478	-.115	.482	-.014	.498	35
36	344.8	217.0	149.1	.186	.007	.083	-.004	.083	.007	.076	36
37	445.7	458.2	153.6	.314	.184	-.083	-.001	.428	-.304	.733	37
38	481.4	459.0	173.9	.303	.710	1.118	5.368	3.603	-1.778	8.366	38
39	471.9	458.0	181.1	.303	.480	2.793	5.541	4.379	-2.048	6.425	39
40	198.4	421.7	89.3	.418	-.747	3.881	2.014	4.071	-.958	5.029	40
41	173.8	418.9	60.9	.427	-.785	4.435	-.310	4.499	-.690	5.189	41
42	388.9	442.7	132.1	.381	-.817	3.842	.038	3.842	-.117	4.459	42
43	442.8	464.9	168.7	.318	-.848	3.289	-2.878	3.741	-1.118	4.858	43
44	808.0	729.8	313.2	.285	.288	1.798	-2.184	2.328	-.281	2.809	44
45	1308.9	754.7	587.2	.111	.031	.085	-.015	.088	.031	.087	45
46	342.1	448.2	142.3	.340	-.018	.027	.168	.093	-.080	.173	46
47	.1	4.0	.0	.495	-.002	.034	.834	.334	-.302	.635	47
48	315.9	437.1	115.5	.484	-.017	.082	.508	.278	-.233	.511	48
49	.1	4.0	.0	.495	-.154	.084	.922	.429	-.519	.947	49
50	.1	4.0	.0	.495	-.182	.182	.885	.94	-.825	1.019	50
51	815.8	613.7	331.7	.230	-.224	2.816	-.462	2.816	-.225	3.041	51
52	638.2	584.5	249.5	.278	-.189	2.739	-.419	2.754	-.203	2.957	52
53	802.7	582.7	386.3	.188	-.071	.072	.048	.075	-.074	.150	53
54	189.9	420.9	86.4	.421	.036	-.015	.048	.062	-.040	.102	54
55	385.9	448.1	148.4	.334	.048	.008	.049	.058	-.004	.062	55
56	508.5	468.8	198.0	.287	.088	.008	.098	.108	-.017	.125	56
57	.1	4.0	.0	.495	-.102	.084	.572	.282	-.310	.801	57
58	1818.9	827.3	708.7	.072	-.223	2.778	-.884	2.788	-.210	3.015	58
59	2192.5	1098.3	1098.3	.000	-.208	2.872	-.833	2.707	-.243	2.949	59
60	483.7	487.8	177.5	.308	-.111	.084	.060	.088	-.118	.203	60
61	.1	4.0	.0	.495	.047	-.038	.183	.108	-.088	.201	61

62	339.0	440.2	124.8	.388	.035	.004	.043	.048	-.007	.084	62
63	484.3	488.4	173.4	.310	.078	-.058	.120	.098	-.078	.179	63
64	.1	4.0	.0	.495	2.404	6.884	20.437	14.988	-8.908	20.874	64
65	.1	4.0	.0	.495	-4.471	18.781	4.300	19.970	-4.860	24.831	65
66	.1	4.0	.0	.495	-3.003	18.297	-8.328	19.284	-22.990	58.274	66
67	.1	4.0	.0	.495	4.528	-2.250	-38.889	20.973	-18.588	39.571	67
68	148.0	418.9	81.5	.438	.037	-.007	.038	.038	-.007	.010	68
69	383.4	443.8	134.8	.348	.089	-.074	.088	.089	-.088	.184	69
70	.1	4.0	.0	.495	2.387	6.328	-16.708	12.940	-4.225	17.188	70
71	.1	4.0	.0	.495	-4.481	18.541	-3.838	18.877	-4.818	24.295	71
72	.1	4.0	.0	.495	-8.587	18.738	18.604	13.106	-9.888	28.071	72
73	.1	4.0	.0	.495	8.374	-4.888	5.087	8.848	-5.338	14.184	73
74	.1	4.0	.0	.495	.073	-.036	-3.885	1.862	-1.915	3.888	74
75	527.9	487.4	208.3	.278	.000	.027	.187	.098	-.071	.189	75
76	446.5	455.8	170.8	.312	-.118	.207	-.189	.238	-.147	.381	76
77	.1	4.0	.0	.495	-5.388	6.537	8.548	8.215	-7.038	15.251	77
78	.1	4.0	.0	.495	5.487	-.884	9.112	7.867	-3.234	11.102	78

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THETA	SIG1/SIG3	LEVEL	ELE
1	-.288	518	.001	518	288	127	282	1.988	.082	1
2	.233	461	.040	467	227	130	8.868	2.151	.088	2
3	.348	378	.118	442	.241	120	4.172	1.887	.080	3
4	375	548	.004	548	375	.088	1.481	1.489	1.725	4
5	381	581	.003	582	.380	.091	4.248	1.478	1.815	5
6	470	683	.012	684	.489	.097	3.558	1.414	1.945	6
7	528	725	.007	728	.525	.100	1.898	1.380	1.898	7
8	347	528	-.008	538	347	.095	-.782	1.548	1.906	8
9	333	588	-.002	588	333	.327	-.589	2.983	1.989	9
10	387	1.378	.037	388	.388	.492	2.134	3.484	2.78	10
11	347	1.480	-.184	442	.423	.531	-3.641	3.508	2.89	11
12	380	860	-.102	868	.372	.293	-10.181	2.878	1.88	12
13	205	.307	.003	307	.205	.051	1.622	1.488	.025	13
14	087	.225	.081	248	.085	.082	20.862	3.843	.077	14
15	078	185	.008	185	.078	.039	3.770	2.018	.784	15
16	280	288	.029	313	.255	.028	41.054	1.228	.577	16
17	340	422	.011	424	.339	.042	7.350	1.251	.849	17
18	375	479	.007	479	.375	.052	3.708	1.280	1.047	18
19	387	478	.008	478	.387	.058	4.845	1.305	1.117	19
20	511	628	.008	628	.511	.058	3.119	1.228	1.154	20
21	381	488	.003	487	.381	.053	1.748	1.294	1.080	21
22	378	1.530	.017	532	.373	.579	2.350	4.108	.332	22
23	331	1.250	-.162	1.284	.298	.494	-10.787	4.338	.308	23
24	328	893	-.112	725	.298	.214	-15.758	2.448	.133	24
25	282	.089	.123	.318	.023	.148	81.733	14.011	.133	25
26	.001	.001	-.000	.001	.001	.000	58.238	1.588	.004	26
27	.081	.000	-.007	.081	.000	.026	-83.960	-133.788	.000	27
28	.082	.088	-.025	.082	.018	.028	-88.878	5.322	.757	28
29	188	248	-.001	248	.188	.050	1.888	1.488	.808	29
30	.000	.008	-.008	.007	.207	.050	-4.880	1.882	.988	30
31	284	384	-.011	387	.283	.051	-8.087	1.404	1.022	31
32	227	328	-.018	329	.224	.052	-10.230	1.488	1.048	32
33	460	598	-.008	598	.468	.058	-3.082	1.223	1.080	33
34	484	589	-.002	589	.484	.047	-1.055	1.192	.947	34
35	502	1.742	.134	1.787	.488	.834	-6.079	3.601	.325	35
36	070	.331	-.071	.331	.070	.130	-1.488	4.728	1.08	36
37	581	1.06	-.171	838	.051	.297	-72.081	12.587	4.307	

43	-.051	1 070	- 838	1.357	- 338	.848	-24.298	-4.017	-1 000	43
44	.215	1 130	128	1 147	1 198	475	7.704	5.803	1 785	44
45	-.135	.428	088	427	- 143	290	5.798	-3.088	-1 000	45
46	-.092	.080	033	.074	- 016	045	23.120	-4.513	-1 000	46
47	102	127	143	.257	- 029	143	42.478	-8.884	-1 000	47
48	113	400	318	.808	- 088	348	32.571	-6.801	-1 000	48
49	-.067	138	.151	.218	- 147	182	-27.823	-1.483	-1 000	49
50	.028	337	-.078	385	009	173	-12.984	41.220	14.951	50
51	-.068	1 110	-.387	1 228	- 185	709	-18.887	-8.841	-1 000	51
52	-.208	785	044	787	- 208	497	2.558	-3.779	-1 000	52
53	.428	388	145	424	- 452	458	9.942	- 537	-1 000	53
54	175	.071	117	247	- 001	124	57.485	-264.998	-1.000	54
55	418	.200	128	475	141	.187	85.089	3.380	885	55
56	728	248	247	831	144	344	67.038	5.785	1 779	56
57	108	227	- 131	-312	020	144	-32.834	12.983	4.447	57
58	-.322	983	- 389	1 089	-.998	1 034	-11.088	-1 071	-1 000	58
59	-1.850	820	190	834	-1.844	1 339	4.083	- 452	-1 000	59
60	-.200	222	114	242	- 489	350	9.540	- 528	-1 000	60
61	.050	028	043	084	005	045	51.908	-17.914	-1 000	61
62	.274	150	098	328	088	.118	81.088	3.413	887	62
63	.441	128	288	548	- 232	390	88.484	-2.341	-1 000	63
64	477	551	- 081	603	425	089	-32.712	1.421	158	64
65	808	712	- 023	717	605	058	-12.074	1.188	088	65
66	538	585	- 016	589	534	032	-14.828	1.121	.045	66
67	188	285	-.007	288	185	050	-4.281	1.542	201	67
68	088	085	001	089	084	002	78.689	1.048	017	68
69	.333	138	153	378	184	281	73.570	-2.088	1 000	69
70	451	487	- 038	400	415	039	-42.743	1.188	070	70
71	877	733	- 012	738	875	031	-11.780	1.091	034	71
72	373	407	007	408	371	018	11.542	1.102	038	72
73	.228	.288	007	270	.224	023	8.780	1.208	078	73
74	010	.018	001	018	010	004	3.530	1.848	315	74
75	080	.219	344	504	- 188	380	39.705	-2.578	-1 000	75
76	138	880	- 340	1 085	- 233	888	-18.543	-4.881	-1 000	76
77	.088	088	004	088	088	014	18.018	1.488	174	77
78	204	218	004	217	203	007	15.855	1.088	028	78

SSTIPH: 1 LAYER OF GEOTEXTILE, P=2000 PPI, E=5%, T=0

TOTAL NUMBER OF NODES-----	99
NUMBER OF BAR ELEMENTS-----	9
NUMBER OF DIFF BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO OF INTERFACE ELE IN PREEXIST PART-----	0
NUMBER OF INTERFACE ELE IN FOUNDATION-----	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	31
NUMBER OF NODES IN FOUNDATION-----	52
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	6
NUMBER OF LOAD CASES-----	1

CALING FACTOR ----- 1 00000

ATMOSPHERIC PRESSURE --- 1 05800

UNIT WEIGHT OF WATER --- 03120

COMPUTATION SEQUENCE FOR A TOTAL OF 7 INCREMENTS

INCREMENT NO	1	PUT ON LAYER NO	1
INCREMENT NO	2	PUT ON LAYER NO	2
INCREMENT NO	3	PUT ON LAYER NO	3
INCREMENT NO	4	PUT ON LAYER NO	4
INCREMENT NO	5	PUT ON LAYER NO	5
INCREMENT NO	6	PUT ON LAYER NO	6
INCREMENT NO	7	APPLY LOAD CASE	1

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		S.C. CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	000	000	1	1	1
2	10.000	000	1	1	1
3	20.000	000	1	1	1
4	30.000	000	1	1	1
5	40.000	000	1	1	1
6	50.000	000	1	1	1
7	60.000	000	1	1	1
8	70.000	000	1	1	1
9	80.000	000	1	1	1
10	88.000	000	1	1	1
11	74.000	000	1	1	1
12	82.000	000	1	1	1
13	90.000	000	1	1	1
14	00.000	4.000	0	0	0
15	10.000	4.000	0	0	0
16	20.000	4.000	0	0	0
17	30.000	4.000	0	0	0
18	40.000	4.000	0	0	0
19	50.000	4.000	0	0	0
20	60.000	4.000	0	0	0
21	70.000	4.000	0	0	0
22	80.000	4.000	0	0	0
23	88.000	4.000	0	0	0
24	74.000	4.000	0	0	0
25	82.000	4.000	0	0	0
26	90.000	4.000	1	0	1
27	00.000	7.000	1	0	1
28	10.000	7.000	0	0	0
29	20.000	7.000	0	0	0
30	30.000	7.000	0	0	0
31	40.000	7.000	0	0	0
32	50.000	7.000	0	0	0
33	60.000	7.000	0	0	0
34	70.000	7.000	0	0	0
35	80.000	7.000	0	0	0
36	88.000	7.000	0	0	0
37	74.000	7.000	0	0	0
38	82.000	7.000	0	0	0
39	90.000	7.000	1	0	1
40	00.000	10.000	1	0	1
41	10.000	10.000	0	0	0
42	20.000	10.000	0	0	0
43	30.000	10.000	0	0	0
44	40.000	10.000	0	0	0
45	50.000	10.000	0	0	0
46	60.000	10.000	0	0	0
47	70.000	10.000	0	0	0
48	80.000	10.000	0	0	0
49	88.000	10.000	0	0	0
50	74.000	10.000	0	0	0
51	82.000	10.000	0	0	0
52	90.000	10.000	1	0	1
53	00.000	11.500	0	0	0
54	10.000	11.500	0	0	0
55	20.000	11.500	0	0	0
56	30.000	11.500	0	0	0
57	40.000	11.500	0	0	0
58	50.000	11.500	0	0	0
59	60.000	11.500	0	0	0
60	70.000	11.500	0	0	0
61	80.000	11.500	1	0	1
62	42.000	13.000	0	0	0
63	50.000	13.000	0	0	0
64	60.000	13.000	0	0	0
65	70.000	13.000	0	0	0
66	80.000	13.000	0	0	0
67	90.000	13.000	0	0	0
68	00.000	14.000	0	0	0
69	10.000	14.000	1	0	1
70	20.000	14.000	0	0	0
71	30.000	14.000	0	0	0
72	40.000	14.000	0	0	0
73	50.000	14.000	0	0	0
74	60.000	14.000	0	0	0
75	70.000	14.000	0	0	0
76	80.000	14.000	0	0	0
77	90.000	14.000	1	0	1
78	00.000	15.000	0	0	0
79	10.000	15.000	0	0	0
80	20.000	15.000	0	0	0
81	30.000	15.000	0	0	0
82	40.000	15.000	0	0	0
83	50.000	15.000	0	0	0
84	60.000	15.000	1	0	1
85	70.000	15.000	0	0	0
86	80.000	15.000	0	0	0
87	90.000	15.000	0	0	0
88	00.000	16.000	0	0	0
89	10.000	16.000	0	0	0
90	20.000	16.000	1	0	1
91	30.000	16.000	0	0	0
92	40.000	16.000	0	0	0
93	50.000	16.000	0	0	0
94	60.000	16.000	0	0	0
95	70.000	16.000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	240	1.00	0

ELMT NO	CONNECTED NODES I J	MATL NO
1	43 44	1
2	44 45	1
3	45 46	1
4	46 47	1
5	47 48	1
6	48 49	1
7	49 50	1
8	50 51	1
9	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WY	YOUNG'S MODULUS CONSTANT	EXPONENT	RATIO	BULK MODULUS CONSTANT	EXPONENT	STRENGTH C	PHI	DPHI	KB
1	0880	8000.00	.500	.500	1500.00	.800	00	35.00	00	.50
2	0830	40 00	.300	.900	20 00	.200	05	00	00	.50
3	0800	1000 00	.400	.700	500 00	.500	.50	40 00	00	.50

POOR NODES SOLID ELEMENT DATA

ELET NO	CONNECTED NODES I J K L	MATL NO	ELEMENT CENTER COORDINATES X-ORD Y-ORD
1	1 2 15 14	3	5 000 2.000
2	2 3 16 15	3	15 000 2 000
3	3 4 17 16	3	25 000 2 000
4	4 5 18 17	2	33 000 2 000
5	5 6 19 18	2	38 000 2 000
6	6 7 20 19	2	46 000 2 000
7	7 8 21 20	2	52 000 2 000
8	8 9 22 21	2	58 000 2 000
9	9 10 23 22	3	62 000 2 000
10	10 11 24 23	3	70 000 2 000
11	11 12 25 24	3	78 000 2 000
12	12 13 26 25	3	86 000 2 000
13	13 14 27 26	3	5 000 5 500
14	14 15 28 27	3	15 000 5 500
15	15 16 29 28	2	25 000 5 500
16	16 17 30 29	2	31 000 5 500
17	17 18 31 30	2	38 000 5 500
18	18 19 32 31	2	46 000 5 500
19	19 20 33 32	2	52 000 5 500
20	20 21 34 33	2	62 000 5 500
21	22 23 35 34	2	68 000 5 500
22	23 24 36 35	2	70 000 5 500
23	24 25 37 36	3	78 000 5 500
24	25 26 38 37	3	86 000 5 500
25	26 27 39 38	3	5 000 8 500
26	27 28 40 39	3	15 000 8 500
27	28 29 41 40	2	25 000 8 500
28	29 30 42 41	2	33 000 8 500
29	30 31 43 42	2	38 000 8 500
30	31 32 44 43	2	46 000 8 500
31	32 33 45 44	2	52 000 8 500
32	33 34 46 45	2	58 000 8 500
33	34 35 47 46	2	62 000 8 500
34	35 36 48 47	2	70 000 8 500
35	36 37 49 48	2	78 000 8 500
36	37 38 50 49	3	86 000 8 500
37	38 39 51 50	3	34 500 10 750
38	39 40 52 51	3	39 000 10 750
39	40 41 53 52	1	46 000 10 750
40	41 42 54 53	1	52 000 10 750
41	42 43 55 54	1	58 000 10 750
42	43 44 56 55	1	62 000 10 750
43	44 45 57 56	1	70 000 10 750
44	45 46 58 57	1	78 000 10 750
45	46 47 59 58	1	86 000 10 750
46	47 48 60 59	1	10 500 12 250
47	48 49 61 60	1	12 250 12 250
48	49 50 62 61	1	12 250 12 250
49	50 51 63 62	1	12 250 12 250
50	51 52 64 63	1	12 250 12 250
51	52 53 65 64	1	12 250 12 250
52	53 54 66 65	1	12 250 12 250
53	54 55 67 66	1	12 250 12 250
54	55 56 68 67	1	12 250 12 250
55	56 57 69 68	1	12 250 12 250
56	57 58 70 69	1	12 250 12 250
57	58 59 71 70	1	12 250 12 250
58	59 60 72 71	1	12 250 12 250
59	60 61 73 72	1	12 250 12 250
60	61 62 74 73	1	12 250 12 250
61	62 63 75 74	1	12 250 12 250
62	63 64 76 75	1	12 250 12 250
63	64 65 77 76	1	12 250 12 250
64	65 66 78 77	1	12 250 12 250
65	66 67 79 78	1	12 250 12 250
66	67 68 80 79	1	12 250 12 250
67	68 69 81 80	1	12 250 12 250
68	69 70 82 81	1	12 250 12 250
69	70 71 83 82	1	12 250 12 250
70	71 72 84 83	1	12 250 12 250
71	72 73 85 84	1	12 250 12 250
72	73 74 86 85	1	12 250 12 250
73	74 75 87 86	1	12 250 12 250
74	75 76 88 87	1	12 250 12 250
75	76 77 89 88	1	12 250 12 250
76	77 78 90 89	1	12 250 12 250
77	78 79 91 90	1	12 250 12 250
78	79 80 92 91	1	12 250 12 250
79	80 81 93 92	1	12 250 12 250
80	81 82 94 93	1	12 250 12 250
81	82 83 95 94	1	12 250 12 250
82	83 84 96 95	1	12 250 12 250
83	84 85 97 96	1	12 250 12 250
84	85 86 98 97	1	12 250 12 250
85	86 87 99 98	1	12 250 12 250
86	87 88 100 99	1	12 250 12 250

CONSTRUCTION LAYER INFORMATION

SSSTIPN: 1 LAYER OF GEOTEXTILE, P=2000 PPI, E=5%, T=0

```
.....  
.....  
* LOAD CASE ----- 1  
.....  
.....
```

LARGEST ELE NO IN THIS INCREMENT 78

LARGEST N P NO IN THIS INCREMENT 95

BAND WIDTH----- 40

TOTAL NUMBER OF EQUATIONS----- 222

NUMBER OF EQUATIONS IN BLOCK----- 85

NUMBER OF BLOCKS----- 3

NUMBER OF N P FORCE CARDS----- 3

NUMBER OF PRESSURE CARDS----- 0

MODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP	X-FORCE	Y-FORCE
1 0.	0	0
2 0.	0	0
3 0.	0	0
4 0.	0	0
5 0.	0	0
6 0.	0	0
7 0.	0	0
8 0.	0	0
9 0.	0	0
10 0.	0	0
11 0.	0	0
12 0.	0	0
13 0.	0	0
14 0.	0	0
15 0.	0	0
16 0.	0	0
17 0.	0	0
18 0.	0	0
19 0.	0	0
20 0.	0	0
21 0.	0	0
22 0.	0	0
23 0.	0	0
24 0.	0	0
25 0.	0	0
26 0.	0	0
27 0.	0	0
28 0.	0	0
29 0.	0	0
30 0.	0	0
31 0.	0	0
32 0.	0	0
33 0.	0	0
34 0.	0	0
35 0.	0	0
36 0.	0	0
37 0.	0	0
38 0.	0	0
39 0.	0	0
40 0.	0	0
41 0.	0	0
42 0.	0	0
43 0.	0	0
44 0.	0	0
45 0.	0	0
46 0.	0	0
47 0.	0	0
48 0.	0	0
49 0.	0	0
50 0.	0	0
51 0.	0	0
52 0.	0	0
53 0.	0	0
54 0.	0	0
55 0.	0	0
56 0.	0	0
57 0.	0	0
58 0.	0	0
59 0.	0	0
60 0.	0	0
61 0.	0	0
62 0.	0	0
63 0.	0	0
64 0.	0	0
65 0.	0	0
66 0.	0	0
67 0.	0	0
68 0.	0	0
69 0.	0	0
70 0.	0	0
71 0.	0	0
72 0.	0	0
73 0.	0	0
74 0.	0	0
75 0.	0	0
76 0.	0	0
77 0.	0	0
78 0.	0	0
79 0.	0	0
80 0.	0	0
81 0.	0	0
82 0.	0	0
83 0.	0	0
84 0.	0	0
85 0.	0	0
86 0.	0	0
87 0.	0	0
88 0.	0	0
89 0.	0	0
90 0.	0	0
91 0.	0	0
92 0.	-4.50	0
93 0.	-4.50	0
94 0.	-2.00	0
95 0.	0	0

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0000	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0001	.0000	.0000	.0001	15
16	.0000	.0000	.0000	.0002	.0000	.0000	.0002	16
17	.0003	.0001	.0000	.0008	.0020	.0000	.0043	17
18	.0133	.0088	.0000	.1908	.0348	.0000	.1940	18
19	.0288	.0028	.0000	.2771	.0483	.0000	.2857	19
20	.0301	.0080	.0000	.1682	.1237	.0000	.2098	20
21	.0182	.0147	.0000	.0688	.1427	.0000	.1865	21
22	.0008	.0004	.0000	.0003	.0053	.0000	.0053	22
23	.0010	.0019	.0000	.0004	.0051	.0000	.0051	23
24	.0008	.0088	.0000	.0011	.0108	.0000	.0110	24
25	.0021	.0020	.0000	.0020	.0083	.0000	.0088	25
26	.0000	.0005	.0000	.0000	.0028	.0000	.0028	26
27	.0000	.0000	.0000	.0000	.0001	.0000	.0001	27
28	.0000	.0000	.0000	.0000	.0001	.0000	.0002	28
29	.0000	.0000	.0000	.0011	.0061	.0000	.0061	29
30	.0112	.0028	.0000	.1583	.0818	.0000	.1684	30
31	.0188	.0127	.0000	.2488	.0879	.0000	.2557	31
32	.0388	.0108	.0000	.3234	.1188	.0000	.3438	32
33	.0882	.0110	.0000	.2878	.2411	.0000	.3753	33
34	.0770	.0187	.0000	.2828	.2387	.0000	.3481	34
35	.0788	.0318	.0000	.1828	.1778	.0000	.2948	35
36	.0007	.0048	.0000	.0014	.0130	.0000	.0131	36
37	.0018	.0118	.0000	.0018	.0200	.0000	.0204	37
38	.0023	.0037	.0000	.0014	.0103	.0000	.0104	38
39	.0000	.0019	.0000	.0000	.0043	.0000	.0043	39
40	.0000	.0000	.0000	.0000	.0001	.0000	.0001	40
41	.0000	.0000	.0000	.0001	.0002	.0000	.0002	41
42	.0053	.0033	.0000	.0878	.0234	.0000	.0718	42
43	.0210	.0088	.0000	.1744	.0770	.0000	.1908	43
44	.0217	.0183	.0000	.1727	.0868	.0000	.1861	44
45	.0218	.0188	.0000	.1848	.1878	.0000	.2278	45
46	.0223	.0181	.0000	.1408	.3284	.0000	.3558	46
47	.0221	.0187	.0000	.1188	.3187	.0000	.3288	47
48	.0224	.0378	.0000	.0883	.2828	.0000	.2981	48
49	.0088	.0388	.0000	.0233	.2187	.0000	.2188	49
50	.0004	.0282	.0000	.0071	.0480	.0000	.0488	50
51	.0008	.0028	.0000	.0028	.0117	.0000	.0120	51
52	.0000	.0024	.0000	.0000	.0088	.0000	.0088	52
53	.0221	.0184	.0000	.4383	.1041	.0000	.4604	53
54	.0221	.0188	.0000	.3810	.1388	.0000	.3888	54
55	.0218	.0127	.0000	.1781	.3084	.0000	.3538	55
56	.0187	.0184	.0000	.1281	.3188	.0000	.3422	56
57	.0104	.0387	.0000	.0833	.2887	.0000	.3018	57
58	.0104	.0880	.0000	.0482	.2488	.0000	.2818	58
59	.0084	.0311	.0000	.0380	.0871	.0000	.1042	59
60	.0037	.0033	.0000	.0088	.0118	.0000	.0127	60
61	.0000	.0028	.0000	.0000	.0087	.0000	.0087	61
62	.0220	.0188	.0000	.0881	.0088	.0000	.0888	62
63	.0211	.0128	.0000	.0888	.1323	.0000	.1447	63
64	.0202	.0178	.0000	.0884	.1801	.0000	.1708	64
65	.0184	.0413	.0000	.0888	.1888	.0000	.1980	65
66	.0204	.0878	.0000	.0888	.1787	.0000	.1871	66
67	.0188	.0330	.0000	.0318	.1288	.0000	.1308	67
68	.0078	.0043	.0000	.0088	.0111	.0000	.0148	68
69	.0000	.0028	.0000	.0000	.0047	.0000	.0047	69
70	.0178	.0088	.0000	.0388	.0388	.0000	.0527	70
71	.0181	.0128	.0000	.0438	.0887	.0000	.0870	71
72	.0208	.0180	.0000	.0474	.1380	.0000	.1197	72
73	.0274	.0371	.0000	.0888	.1880	.0000	.1871	73
74	.0278	.0884	.0000	.0881	.1874	.0000	.1871	74
75	.0210	.0333	.0000	.0384	.1388	.0000	.1474	75
76	.0100	.0048	.0000	.0118	.0108	.0000	.0188	76
77	.0000	.0021	.0000	.0000	.0038	.0000	.0038	77
78	.0181	.0128	.0000	.0288	.0482	.0000	.0581	78
79	.0188	.0180	.0000	.0103	.0888	.0000	.0724	79
80	.0213	.0388	.0000	.0382	.0810	.0000	.0878	80
81	.0217	.0814	.0000	.0287	.1388	.0000	.1407	81
82	.0472	.2888	.0000	.2378	.3808	.0000	.4240	82
83	.0000	.0808	.0000	.8727	.0880	.0000	.8780	83
84	.0000	.0888	.0000	.0000	.0818	.0000	.0818	84
85	.0188	.0181	.0000	.0202	.0388	.0000	.0421	85
86	.0170	.0342	.0000	.0233	.0878	.0000	.0822	86
87	.0188	.2883	.0000	.0223	.2817	.0000	.2827	87
88	.0078	.4738	.0000	.0120	.8288	.0000	.8288	88
89	.0000	.1188	.0000	.8124	.1183	.0000	.8288	89
90	.0000	.2278	.0000	.0000	.2288	.0000	.2288	90
91	.0118	.0338	.0000	.0118	.0388	.0000	.0387	91
92	.0118	.2802	.0000	.0119	.2802	.0000	.2804	92
93	.0188	.4782	.0000	.0188	.4782	.0000	.4788	93
94	.0221	.2414	.0000	.0221	.2414	.0000	.3883	94
95	.0000	.3821	.0000	.0000	.3821	.0000	.3821	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS - INTERNAL MEMBER FORCES

ELEMENT NO	AXIAL FORCE
INCREMENTAL VALUES	
1	.0242
2	.0108
3	.0089
4	.0078
5	.0177
6	.4788
7	.1828
8	.0108
9	.0237
TOTAL VALUES	
1	.0288
2	.3842
3	.7148
4	.1254
5	.1272
6	.1888
7	.0881

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	524.7	328.3	227.1	155	.001	.000	.001	.001	-.001	.002	1
2	521.8	331.4	224.8	181	.001	-.007	-.007	.008	-.001	.002	2
3	541.0	338.5	234.5	184	.021	-.019	.048	.032	-.029	.061	3
4	6	17.1	2	484	1.554	-.457	2.172	2.028	-.921	2.960	4
5	6	17.2	2	484	.718	.438	8.718	3.938	-2.784	8.722	5
6	7	18.0	2	484	.674	2.473	8.818	4.207	-2.468	8.678	6
7	7	18.4	2	484	-1.284	3.330	3.214	3.837	-1.781	5.598	7
8	6	18.8	2	484	-.857	1.850	-.852	1.915	-.923	2.838	8
9	487.3	343.3	191.3	221	.001	.129	.008	.129	.000	.129	9
10	477.1	348.1	182.3	237	-.008	.159	.028	.200	-.070	.211	10
11	477.7	372.4	182.3	242	-.008	.102	-.074	.204	-.012	.221	11
12	602.8	358.5	208.3	209	.013	.088	-.042	.103	.004	.098	12
13	434.0	288.2	193.6	121	.000	.001	-.001	.002	.000	.002	13
14	440.8	288.6	208.1	116	.006	.008	.014	.010	-.004	.018	14
15	1.7	13.5	6	478	.807	-.831	2.307	1.403	-1.427	2.830	15
16	9.7	18.9	3.5	398	2.290	-1.380	3.084	2.851	-1.942	4.783	16
17	1.8	17.0	8	482	1.388	.221	4.088	2.818	-1.338	4.251	17
18	6	17.1	2	484	-.898	2.734	3.867	3.970	-1.735	5.308	18
19	6	17.1	2	484	-1.891	3.524	8.215	4.804	-2.771	7.375	19
20	7	18.2	2	484	-1.733	4.442	3.843	4.838	-2.230	7.189	20
21	6	17.2	2	484	-1.148	3.007	1.974	3.230	-1.371	4.801	21
22	488.0	381.8	182.3	246	-.010	.284	.041	.288	-.012	.287	22
23	403.8	303.8	184.0	230	-.008	.238	-.098	.244	-.014	.258	23
24	430.8	293.7	184.0	192	.021	.112	-.045	.117	.016	.101	24
25	408.8	229.8	188.9	093	-.001	.000	.000	.000	-.001	.001	25
26	3	13	1	485	.324	-.311	.921	.923	-.439	1.232	26
27	3.5	13.7	1	487	1.320	-.818	.880	1.401	-.887	2.298	27
28	2.7	13.7	9	486	.730	-.408	-.988	.918	-.594	1.513	28
29	1.3	14.2	5	484	.868	.713	-.461	.861	.397	4.842	29
30	6	18.1	2	484	.373	2.118	3.288	2.821	-1.178	4.084	30
31	6	18.8	2	484	-.703	2.788	-4.841	4.001	-1.948	5.950	31
32	6	18.6	2	484	-1.141	3.078	-4.788	4.158	-2.221	6.378	32
33	7	17.8	2	484	-1.495	5.122	-2.122	5.288	-1.685	6.954	33
34	7	17.1	2	483	-.241	3.848	-.131	3.847	-.222	4.088	34
35	274.5	248.0	107.1	281	-.028	.481	-.072	.494	-.028	.522	35
36	348.2	217.8	151.8	152	-.009	.094	.012	.094	-.009	.103	36
37	11.9	398.8	4.0	495	.009	.240	17.982	9.107	-.858	17.984	37
38	448.5	458.6	170.9	312	-.712	1.354	18.902	10.661	-.944	20.080	38
39	1	4.0	0	485	-1.278	3.897	10.882	7.101	-4.683	11.784	39
40	288.8	431.1	.97.3	387	-.914	4.089	2.337	4.330	-1.175	5.504	40
41	202.5	422.8	71.9	415	-.843	3.773	-.086	3.773	-.843	4.417	41
42	288.0	434.5	107.3	378	-.804	3.178	-.081	3.178	-.804	3.785	42
43	572.7	480.6	225.4	270	-.738	3.248	-3.035	3.851	-1.228	5.088	43
44	721.8	642.4	281.8	281	-.248	1.787	-2.138	2.343	-.287	2.838	44
45	700.8	564.2	279.9	252	-.052	.080	.048	.084	-.056	1.008	45
46	111.8	388.6	4.0	488	-.071	.071	.460	.230	-.230	.460	46
47	1	4.0	0	485	-.008	.028	.818	.418	-.397	.818	47
48	258.3	429.3	82.0	393	-.088	.088	.225	.153	-.124	.277	48
49	11.2	352.5	4.0	522	-.022	.212	-.022	.212	-.022	.212	49
50	1	4.0	0	485	-.079	.348	.787	.584	-.318	.903	50
51	588.1	487.0	224.1	270	-.280	2.894	.010	2.894	-.280	3.154	51
52	501.6	463.4	194.4	280	-.218	2.784	-.351	2.775	-.260	3.003	52
53	552.8	471.8	218.2	288	-.091	.093	.074	.100	-.099	.198	53
54	120.8	412.8	41.7	480	.037	-.023	.182	.108	-.083	.201	54
55	479.3	480.1	184.4	300	.034	.015	.089	.070	-.020	.081	55
56	1	4.0	0	485	.037	-.187	1.182	.517	-.687	1.184	56
57	1	4.0	0	485	-.002	-.100	1.823	.682	-.784	1.428	57
58	184.4	484.3	737.8	073	-.243	2.788	.533	2.782	-.287	3.028	58
59	1380.8	781.8	624.2	108	-.151	2.886	-.832	2.728	-.285	3.013	59
60	448.2	455.3	188.8	313	-.130	1.048	.085	1.07	-.133	2.00	60
61	60.0	408.4	20.4	475	.081	-.039	.488	.258	-.215	.473	61

62	252.6	428.8	90.5	.394	047	- 013	- 005	.047	- 014	.061	62
63	430.8	453.0	163.2	.320	140	- 060	.084	.150	-.071	.221	63
64	268.9	429.6	83.0	.392	007	.114	.010	.114	.008	.108	64
65	.1	4.0	.0	.495	-1.802	4.317	-12.531	16.468	-3.951	20.417	65
66	.1	4.0	.0	.495	-2.817	16.460	-48.688	33.469	-19.827	53.298	66
67	.1	4.0	.0	.495	4.134	-1.848	-35.086	18.844	-16.659	35.803	67
68	185.0	417.9	67.7	.431	.021	.011	-.158	.096	-.064	.160	68
69	377.3	445.5	140.5	.342	.078	-.078	.100	.093	-.091	.184	69
70	.1	4.0	.0	.495	.001	8.319	1.247	8.385	-.045	8.411	70
71	.1	4.0	.0	.495	-1.786	20.136	15.170	22.507	-4.138	26.843	71
72	.1	4.0	.0	.495	-5.866	18.021	18.728	19.846	-8.650	28.436	72
73	.1	4.0	.0	.495	7.411	-3.811	5.043	7.952	-4.352	12.204	73
74	.1	4.0	.0	.495	.037	-.039	-2.442	1.221	-1.222	2.443	74
75	818.2	540.8	347.0	.179	-.009	.023	.008	.024	-.0	.034	75
76	343.4	489.8	213.5	.273	-.094	.173	-.013	.173	-.0	.268	76
77	.1	4.0	.0	.495	-4.778	8.405	7.550	7.580	-.5	13.483	77
78	.1	4.0	.0	.495	4.847	-.432	8.643	8.450	-2.35	8.485	78

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THETA	SIG1/SIG3	LEVEL	ELE
1	.243	.481	.003	.481	.243	.119	.814	1.976	.079	1
2	.250	.517	.016	.518	.248	.134	3.356	2.080	.088	2
3	.346	.591	.170	.480	.285	.173	39.644	1.843	.074	3
4	.361	.533	.004	.523	.361	.088	1.455	1.876	1.719	4
5	.267	.545	.014	.547	.365	.090	4.401	1.484	1.806	5
6	.482	.855	.013	.856	.481	.087	3.788	1.421	1.864	6
7	.520	.720	.007	.720	.520	.100	2.088	1.385	2.003	7
8	.333	.624	-.002	.624	.333	.086	-.480	1.574	1.811	8
9	.340	1.033	.011	1.034	.340	.347	-.890	3.043	2.06	9
10	.413	1.425	.055	1.428	.411	.509	3.079	3.477	.281	10
11	.442	1.446	.018	1.466	.422	.522	-7.386	3.495	.285	11
12	.362	.887	-.088	.901	.348	.277	-9.251	2.590	.163	12
13	.138	.278	-.003	.276	.138	.089	-1.191	2.006	.052	13
14	.181	.283	.028	.281	.183	.084	16.763	1.709	.040	14
15	.083	.170	.009	.171	.082	.040	8.651	1.859	.782	15
16	.288	.289	.024	.293	.244	.024	44.738	1.200	.467	16
17	.336	.413	.012	.415	.334	.041	8.468	1.243	.811	17
18	.369	.488	.008	.489	.365	.052	4.389	1.287	1.044	18
19	.261	.471	.015	.472	.260	.086	6.556	1.311	1.120	19
20	.518	.621	.009	.632	.514	.059	4.237	1.229	1.175	20
21	.368	.475	.004	.475	.368	.054	2.181	1.293	1.078	21
22	.409	1.618	.078	1.618	.404	.607	3.584	4.004	.337	22
23	.318	1.218	-.186	1.242	.282	.478	-8.578	4.254	.287	23
24	.273	.652	-.082	.668	.256	.207	-11.725	2.614	.135	24
25	.042	.091	.002	.091	.042	.025	1.810	2.187	-.022	25
26	.001	-.004	.003	.003	-.005	.004	68.421	-.461	-1.000	26
27	.074	.008	.008	.075	.008	.034	85.502	8.847	.871	27
28	.074	.029	-.027	.087	.018	.035	-64.721	5.298	.707	28
29	.140	.223	.000	.223	.140	.041	1.107	1.593	.830	29
30	.198	.289	-.009	.289	.197	.081	-5.127	1.517	1.020	30
31	.248	.348	-.013	.348	.248	.052	-7.158	1.420	1.033	31
32	.230	.328	-.019	.332	.226	.053	-10.847	1.488	1.058	32
33	.318	.628	-.006	.628	.317	.088	-3.198	1.215	1.111	33
34	.525	.619	.001	.619	.523	.048	.625	1.184	.959	34
35	.480	.751	.064	.733	.438	.846	-3.482	2.988	.348	35
36	.083	.303	.010	.303	.083	.128	2.204	5.710	.107	36
37	.031	-.049	-.036	.078	.003	.037	-37.860	28.128	8.968	37
38	.086	.136	.048	.159	.044	.057	28.217	3.618	.973	38
39	.032	.282	.088	.282	.002	.148	18.444	131.067	48.358	39
40	-.036	.344	-.063	.355	-.049	.202	-9.168	-7.293	-1.000	40
41	-.296	.267	-.010	.267	-.298	.262	-1.032	-.502	-1.000	41
42	-.274	.411	-.298	.522	-.385	.454	20.484	-1.384	-1.000	42
43	-.119	1.136	-.711	1.487	-.440	.849	-24.289	-3.312	-1.000	43
44	.112	1.079	-.183	1.113	-.079	.617	10.384	14.057	4.864	44
45	-.142	.425	-.108	.445	-.162	.303	10.444	-2.751	-1.000	45
46	.010	.028	.016	.037	.000	.010	29.656	82.722	30.378	46
47	.087	.154	.120	.245	-.004	.128	37.282	-63.974	-1.000	47
48	.024	.275	.275	.528	-.127	.326	28.720	-4.146	-1.000	48
49	.086	.104	-.134	.171	-.163	.187	-26.569	-1.053	-1.000	49
50	.022	.308	-.055	.320	.011	.154	-10.530	27.820	9.970	50
51	.120	1.185	-.383	1.287	.008	.644	-17.128	158.297	58.471	51
52	.247	.786	.078	.771	-.283	.512	4.245	-3.049	-1.000	52
53	.329	.386	.164	.418	-.381	.388	11.632	-1.157	-1.000	53
54	.109	.088	.113	.202	-.027	.118	90.422	-7.371	-1.000	54
55	.384	.223	.217	.518	.081	.227	53.460	8.408	2.753	55
56	.181	.189	.081	.228	.123	.052	37.208	1.855	.318	56
57	.072	.232	-.084	.283	.034	.118	-23.315	7.417	2.385	57
58	-1.088	1.108	-.428	1.188	-1.168	1.178	-10.865	-1.018	-1.000	58
59	-1.888	.867	.281	.715	-1.885	1.205	8.250	-.422	-1.000	59
60	-.471	.260	.082	.289	-.481	.375	6.356	-.560	-1.000	60
61	.035	.014	.064	.069	.040	.085	49.646	2.229	1.000	61
62	.279	.107	.053	.294	.092	.101	74.287	3.188	.817	62
63	.808	.030	.242	.875	-.039	.487	74.031	-22.510	-1.000	63
64	.447	.673	-.082	.689	.432	.128	-14.361	1.595	.221	64
65	.509	.539	-.008	.540	.509	.016	-3.253	1.082	.023	65
66	.583	.586	-.020	.595	.544	.026	-25.374	1.094	.025	66
67	.189	.292	-.012	.293	.187	.044	-7.112	1.487	.181	67
68	.184	.118	-.077	.228	.087	.084	-88.908	3.502	.930	68
69	.280	.180	.191	.288	-.228	.263	72.423	1.310	1.000	69
70	.420	.448	-.034	.469	.398	.037	-34.488	1.185	.089	70
71	.141	.775	.006	.776	.140	.018	9.870	1.049	.018	71
72	.370	.403	.007	.405	.368	.018	10.784	1.098	.037	72
73	.231	.257	.003	.264	.231	.013	7.286	1.112	.043	73
74	.008	.017	.001	.017	.008	.004	7.231	2.047	.288	74
75	-.018	.224	.032	.228	-.023	.125	7.314	-10.126	-1.000	75
76	-.184	.976	-.027	.976	-.184	.580	-1.321	-5.239	-1.000	76
77	.077	.103	.003	.104	.077	.013	8.536	1.345	.128	77
78	.198	.208	.003	.208	.195	.007	11.247	1.071	.026	78

PROGRAM SSTIPN

SSTIPN 1 LAYER OF GEOTEXTILE, P=2000 PPI, E=5X, T=1000

TOTAL NUMBER OF NODES-----	95
NUMBER OF BAR ELEMENTS-----	9
NUMBER OF DIFF BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO OF INTERFACE ELE IN PREEXIST PART-----	0
NUMBER OF INTERFACE ELE IN FOUNDATION-----	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	38
NUMBER OF NODES IN FOUNDATION-----	62
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	4
NUMBER OF LOAD CASES-----	2

CALING FACTOR ----- 1.00000

ATMOSPHERIC PRESSURE --- 1.05800

UNIT WEIGHT OF WATER --- 0.3120

INPUTATION SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO	1	APPLY LOAD CASE	1
INCREMENT NO	2	PUT ON LAYER NO	1
INCREMENT NO	3	PUT ON LAYER NO	2
INCREMENT NO	4	PUT ON LAYER NO	3
INCREMENT NO	5	PUT ON LAYER NO	4
INCREMENT NO	6	PUT ON LAYER NO	5
INCREMENT NO	7	PUT ON LAYER NO	6
INCREMENT NO	8	APPLY LOAD CASE	2

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		B.C. CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	000	000	1	1	1
2	10 000	000	1	1	1
3	20 000	000	1	1	1
4	30 000	000	1	1	1
5	40 000	000	1	1	1
6	50 000	000	1	1	1
7	60 000	000	1	1	1
8	70 000	000	1	1	1
9	80 000	000	1	1	1
10	88 000	000	1	1	1
11	74 000	000	1	1	1
12	62 000	000	1	1	1
13	50 000	000	1	1	1
14	40 000	000	1	1	1
15	10 000	4 000	0	0	0
16	20 000	4 000	0	0	0
17	30 000	4 000	0	0	0
18	40 000	4 000	0	0	0
19	50 000	4 000	0	0	0
20	60 000	4 000	0	0	0
21	70 000	4 000	0	0	0
22	80 000	4 000	0	0	0
23	88 000	4 000	0	0	0
24	74 000	4 000	0	0	0
25	62 000	4 000	0	0	0
26	50 000	4 000	1	0	1
27	40 000	7 000	1	0	1
28	30 000	7 000	0	0	0
29	20 000	7 000	0	0	0
30	10 000	7 000	0	0	0
31	00 000	7 000	0	0	0
32	42 000	7 000	0	0	0
33	50 000	7 000	0	0	0
34	60 000	7 000	0	0	0
35	70 000	7 000	0	0	0
36	80 000	7 000	0	0	0
37	88 000	7 000	0	0	0
38	74 000	7 000	0	0	0
39	62 000	7 000	1	0	1
40	50 000	10 000	1	0	1
41	40 000	10 000	0	0	0
42	30 000	10 000	0	0	0
43	20 000	10 000	0	0	0
44	10 000	10 000	0	0	0
45	00 000	10 000	0	0	0
46	42 000	10 000	0	0	0
47	50 000	10 000	0	0	0
48	60 000	10 000	0	0	0
49	70 000	10 000	0	0	0
50	80 000	10 000	0	0	0
51	88 000	10 000	0	0	0
52	74 000	10 000	1	0	1
53	62 000	11 500	0	0	0
54	50 000	11 500	0	0	0
55	40 000	11 500	0	0	0
56	30 000	11 500	0	0	0
57	20 000	11 500	0	0	0
58	10 000	11 500	0	0	0
59	00 000	11 500	0	0	0
60	42 000	11 500	0	0	0
61	50 000	11 500	1	0	1
62	42 000	13 000	0	0	0
63	50 000	13 000	0	0	0
64	60 000	13 000	0	0	0
65	70 000	13 000	0	0	0
66	80 000	13 000	0	0	0
67	88 000	13 000	0	0	0
68	74 000	13 000	0	0	0
69	62 000	13 000	1	0	1
70	50 000	14 000	0	0	0
71	40 000	14 000	0	0	0
72	30 000	14 000	0	0	0
73	20 000	14 000	0	0	0
74	10 000	14 000	0	0	0
75	00 000	14 000	0	0	0
76	42 000	14 000	0	0	0
77	50 000	14 000	1	0	1
78	60 000	15 000	0	0	0
79	70 000	15 000	0	0	0
80	80 000	15 000	0	0	0
81	88 000	15 000	0	0	0
82	74 000	15 000	0	0	0
83	62 000	15 000	0	0	0
84	50 000	15 000	1	0	1
85	40 000	16 000	0	0	0
86	30 000	16 000	0	0	0
87	20 000	16 000	0	0	0
88	10 000	16 000	0	0	0
89	00 000	16 000	0	0	0
90	42 000	16 000	1	0	1
91	50 000	17 000	0	0	0
92	60 000	17 000	0	0	0
93	70 000	17 000	0	0	0
94	80 000	17 000	0	0	0
95	88 000	17 000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	240	1.00	0

ELMT NO	CONNECTED NODES I J	MATL NO
1	43 44	1
2	44 45	1
3	45 46	1
4	46 47	1
5	47 48	1
6	48 49	1
7	49 50	1
8	50 51	1
9	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	YOUNG'S MODULUS UNITS: WY CONSTANT	EXPOONENT	RATIO	BULK MODULUS CONSTANT	EXPOONENT	STRENGTH C	PARAMETERS PHI	DPHI	KO	
1	.0880	8000.00	500	500	1500.00	800	00	35.00	.00	50
2	.0830	40.00	300	300	20.00	200	08	00	.00	50
3	.0800	1000.00	400	.700	500.00	500	.50	40.00	.00	50

FOUR-NODES SOLID ELEMENT DATA

ELET NO	CONNECTED NODES I J K L	MATL NO	ELEMENT CENTER COORDINATES X-ORD Y-ORD
1	1 2 15 14	3	5.000 2.000
2	2 3 15 15	3	15.000 2.000
3	3 4 17 15	3	25.000 2.000
4	4 5 14 15	2	33.000 2.000
5	5 6 19 16	2	38.000 2.000
6	6 7 20 19	2	46.000 2.000
7	7 8 21 20	2	52.000 2.000
8	8 9 22 21	2	58.000 2.000
9	9 10 23 22	3	62.000 2.000
10	10 11 24 23	3	70.000 2.000
11	11 12 25 24	3	78.000 2.000
12	12 13 26 25	3	86.000 2.000
13	13 14 28 27	3	5.000 5.500
14	14 15 29 28	3	15.000 5.500
15	15 16 30 29	2	25.000 5.500
16	16 17 31 30	2	33.000 5.500
17	17 18 32 31	2	39.000 5.500
18	18 19 33 32	2	45.000 5.500
19	20 21 34 33	2	52.000 5.500
20	21 22 35 34	2	58.000 5.500
21	22 23 36 35	2	62.000 5.500
22	23 24 37 36	3	70.000 5.500
23	24 25 38 37	3	78.000 5.500
24	25 26 39 38	3	86.000 5.500
25	27 28 41 40	3	5.000 8.500
26	28 29 42 41	2	15.000 8.500
27	29 30 43 42	2	25.000 8.500
28	30 31 44 43	2	33.000 8.500
29	31 32 45 44	2	39.000 8.500
30	32 33 46 45	2	45.000 8.500
31	33 34 47 46	2	52.000 8.500
32	34 35 48 47	2	58.000 8.500
33	35 36 49 48	2	62.000 8.500
34	36 37 50 49	2	70.000 8.500
35	37 38 51 50	3	78.000 8.500
36	38 39 52 51	3	86.000 8.500
37	43 44 53 53	1	34.500 10.750
38	44 45 54 54	1	38.000 10.750
39	45 46 55 54	1	45.000 10.750
40	46 47 56 55	1	52.000 10.750
41	47 48 57 56	1	58.000 10.750
42	48 49 58 57	1	62.000 10.750
43	49 50 59 58	1	70.000 10.750
44	50 51 60 59	1	78.000 10.750
45	51 52 61 60	1	86.000 10.750
46	52 53 62 62	1	40.500 12.250
47	53 54 63 62	1	48.000 12.250
48	54 55 64 63	1	52.000 12.250
49	55 56 65 64	1	58.000 12.250
50	57 58 66 65	1	62.000 12.250
51	58 59 67 66	1	70.000 12.250
52	59 60 68 67	1	78.000 12.250
53	60 61 69 68	1	86.000 12.250
54	62 63 70 70	1	49.000 13.500
55	63 64 71 71	1	52.000 13.500
56	64 65 72 72	1	58.000 13.500
57	65 66 73 73	1	62.000 13.500
58	66 67 74 74	1	70.000 13.500
59	67 68 75 75	1	78.000 13.500
60	68 69 76 76	1	86.000 13.500
61	70 71 77 77	1	48.000 14.500
62	71 72 78 78	1	52.000 14.500
63	72 73 79 79	1	58.000 14.500
64	73 74 80 80	1	62.000 14.500
65	74 75 81 81	1	70.000 14.500
66	75 76 82 82	1	78.000 14.500
67	76 77 83 83	1	86.000 14.500
68	77 78 84 84	1	53.000 15.500
69	78 79 85 85	1	58.000 15.500
70	79 80 86 86	1	62.000 15.500
71	81 82 87 87	1	70.000 15.500
72	82 83 88 88	1	78.000 15.500
73	83 84 89 89	1	86.000 15.500
74	85 86 90 90	1	57.000 16.500
75	86 87 91 91	1	62.000 16.500
76	87 88 92 92	1	70.000 16.500
77	89 90 93 93	1	78.000 16.500
78	91 92 94 94	1	86.000 16.500

CONSTRUCTION LAYER INFORMATION

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP	X-FORCE	Y-FORCE
1	0	0
2	0	0
3	0	0
4	0	0
5	0	0
6	0	0
7	0	0
8	0	0
9	0	0
10	0	0
11	0	0
12	0	0
13	0	0
14	0	0
15	0	0
16	0	0
17	0	0
18	0	0
19	0	0
20	0	0
21	0	0
22	0	0
23	0	0
24	0	0
25	0	0
26	0	0
27	0	0
28	0	0
29	0	0
30	0	0
31	0	0
32	0	0
33	0	0
34	0	0
35	0	0
36	0	0
37	0	0
38	0	0
39	0	0
40	0	0
41	0	0
42	0	0
43	0	0
44	0	0
45	0	0
46	0	0
47	0	0
48	0	0
49	0	0
50	0	0
51	0	0
52	0	0
53	0	0
54	0	0
55	0	0
56	0	0
57	0	0
58	0	0
59	0	0
60	0	0
61	0	0
62	0	0
63	0	0
64	0	0
65	0	0
66	0	0
67	0	0
68	0	0
69	0	0
70	0	0
71	0	0
72	0	0
73	0	0
74	0	0
75	0	0
76	0	0
77	0	0
78	0	0
79	0	0
80	0	0
81	0	0
82	0	0
83	0	0
84	0	0
85	0	0
86	0	0
87	0	0
88	0	0
89	0	0
90	0	0
91	0	0
92	-4.50	0
93	-4.50	0
94	-2.00	0
95	0	0

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0000	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0002	.0000	.0000	.0002	15
16	.0000	.0001	.0000	.0004	.0007	.0000	.0008	16
17	.0007	.0002	.0000	.0052	.0025	.0000	.0054	17
18	.0219	.0085	.0000	.2151	.0428	.0000	.2193	18
19	.0333	.0038	.0000	.3034	.0772	.0000	.3131	19
20	.0171	.0124	.0000	.1857	.1342	.0000	.2133	20
21	.0070	.0075	.0000	.0844	.1408	.0000	.1528	21
22	.0003	.0008	.0000	.0008	.0040	.0000	.0040	22
23	.0010	.0022	.0000	.0008	.0053	.0000	.0054	23
24	.0008	.0087	.0000	.0010	.0108	.0000	.0108	24
25	.0022	.0022	.0000	.0020	.0084	.0000	.0087	25
26	.0000	.0004	.0000	.0000	.0027	.0000	.0027	26
27	.0000	.0000	.0000	.0000	.0001	.0000	.0001	27
28	.0000	.0000	.0000	.0000	.0002	.0000	.0002	28
29	.0001	.0000	.0000	.0016	.0005	.0000	.0016	29
30	.0243	.0053	.0000	.2303	.0702	.0000	.2408	30
31	.0444	.0188	.0000	.3346	.0750	.0000	.3429	31
32	.0888	.0055	.0000	.3884	.1405	.0000	.3924	32
33	.0488	.0288	.0000	.2810	.2883	.0000	.3389	33
34	.0484	.0088	.0000	.2229	.2416	.0000	.3287	34
35	.0888	.0103	.0000	.1888	.1833	.0000	.2208	35
36	.0013	.0010	.0000	.0008	.0135	.0000	.0135	36
37	.0021	.0110	.0000	.0010	.0137	.0000	.0164	37
38	.0078	.0041	.0000	.0018	.0108	.0000	.0108	38
39	.0030	.0017	.0000	.0000	.0048	.0000	.0048	39
40	.0000	.0000	.0000	.0000	.0001	.0000	.0001	40
41	.0000	.0000	.0000	.0001	.0002	.0000	.0003	41
42	.0212	.0078	.0000	.0798	.0838	.0000	.0863	42
43	.0472	.0115	.0000	.2888	.1194	.0000	.2824	43
44	.0478	.0201	.0000	.2598	.0804	.0000	.2887	44
45	.0130	.0084	.0000	.2427	.1828	.0000	.2144	45
46	.0484	.0388	.0000	.2082	.3881	.0000	.4228	46
47	.0388	.0131	.0000	.1728	.3284	.0000	.3781	47
48	.0283	.0012	.0000	.1381	.2403	.0000	.2782	48
49	.0004	.0884	.0000	.0871	.2215	.0000	.2288	49
50	.0021	.0284	.0000	.0188	.0500	.0000	.0538	50
51	.0002	.0038	.0000	.0083	.0135	.0000	.0184	51
52	.0000	.0023	.0000	.0000	.2788	.0000	.2788	52
53	.0408	.0201	.0000	.4828	.1184	.0000	.4778	53
54	.0417	.0078	.0000	.3881	.1815	.0000	.4178	54
55	.0828	.0381	.0000	.2138	.3430	.0000	.4040	55
56	.0888	.0220	.0000	.1708	.3437	.0000	.3837	56
57	.0342	.0111	.0000	.1234	.2825	.0000	.3083	57
58	.0078	.0888	.0000	.0884	.2487	.0000	.2588	58
59	.0080	.0318	.0000	.0173	.0811	.0000	.0827	59
60	.0028	.0040	.0000	.0084	.0488	.0000	.0504	60
61	.0000	.0025	.0000	.0000	.3113	.0000	.3113	61
62	.0548	.0088	.0000	.0888	.0283	.0000	.1012	62
63	.0842	.0344	.0000	.1087	.1878	.0000	.1888	63
64	.0881	.0214	.0000	.1084	.1817	.0000	.2121	64
65	.0704	.0288	.0000	.1142	.1844	.0000	.2258	65
66	.0188	.0808	.0000	.0887	.2224	.0000	.2303	66
67	.0128	.0327	.0000	.0627	.1175	.0000	.1175	67
68	.0082	.0043	.0000	.0082	.0108	.0000	.0138	68
69	.0000	.0025	.0000	.0000	.0041	.0000	.0041	69
70	.0888	.0195	.0000	.0884	.0840	.0000	.1088	70
71	.0878	.0344	.0000	.0881	.1228	.0000	.1388	71
72	.0888	.0212	.0000	.0888	.1317	.0000	.1848	72
73	.0718	.0248	.0000	.1025	.1470	.0000	.1788	73
74	.0278	.0827	.0000	.0801	.2028	.0000	.2113	74
75	.0178	.0338	.0000	.0822	.1288	.0000	.1387	75
76	.0083	.0047	.0000	.0102	.0102	.0000	.0144	76
77	.0000	.0025	.0000	.0000	.0028	.0000	.0028	77
78	.0715	.0343	.0000	.0888	.0752	.0000	.1148	78
79	.0888	.0278	.0000	.0888	.0721	.0000	.1122	79
80	.0714	.0234	.0000	.0883	.0781	.0000	.1188	80
81	.4800	.2221	.0000	.4787	.2875	.0000	.5484	81
82	.2848	.2748	.0000	.1875	.3875	.0000	.4037	82
83	.7481	.0488	.0000	.7388	.0718	.0000	.7434	83
84	.0000	.0825	.0000	.0000	.0724	.0000	.0724	84
85	.0884	.0219	.0000	.0788	.0428	.0000	.0878	85
86	.0720	.0220	.0000	.0807	.0471	.0000	.0935	86
87	.0873	.3428	.0000	.0788	.3873	.0000	.3752	87
88	.0830	.8448	.0000	.8848	.8872	.0000	.8048	88
89	.8702	.1023	.0000	.8818	.1233	.0000	.8788	89
90	.0000	.2488	.0000	.0000	.2223	.0000	.2223	90
91	.0248	.0217	.0000	.0288	.0217	.0000	.0381	91
92	.0338	.3435	.0000	.0338	.3435	.0000	.3452	92
93	.0318	.8480	.0000	.0318	.8480	.0000	.8488	93
94	.2888	.2340	.0000	.2888	.2340	.0000	.3473	94
95	.0000	.3828	.0000	.0000	.3828	.0000	.3828	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS - INTERNAL MEMBER FORCES

ELEMENT NO	AXIAL FORCE
INCREMENTAL VALUES	
1	.0253
2	.0470
3	.0778
4	.6267
5	.8408
6	.8117
7	.0401
8	.0571
9	.0088
TOTAL VALUES	
1	.2847
2	.8022
3	.1087
4	2.1271
5	2.1870
6	2.3714
7	1.1488

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE	
1	528.3	330.4	227.8	185	001	000	002	002	-001	002	1	
2	528.3	330.4	227.8	185	001	000	002	002	-001	002	2	
3	528.3	330.4	227.8	185	024	-023	084	037	-025	072	3	
4	528.3	330.4	227.8	185	1.749	-588	2.419	2.286	-1.083	3.248	4	
5	528.3	330.4	227.8	185	494	735	431	7.432	4.327	-3.181	7.488	5
6	528.3	330.4	227.8	185	494	735	431	7.432	4.327	-3.181	7.488	6
7	528.3	330.4	227.8	185	493	-1.338	3.438	2.886	3.842	-1.741	5.583	7
8	528.3	330.4	227.8	185	494	-741	1.811	-887	1.903	-833	2.733	8
9	478.0	349.7	195.9	220	007	117	007	117	007	110	9	
10	478.2	349.3	193.5	238	-010	201	.028	202	-011	213	10	
11	478.1	372.8	193.0	241	-008	202	-072	.208	-012	220	11	
12	496.8	352.0	205.6	208	013	101	-.042	108	008	.098	12	
13	426.5	256.9	194.7	121	001	002	-.002	002	000	002	13	
14	463.4	271.8	207.0	118	009	008	-.019	013	-.010	024	14	
15	2.1	13.6	7.7	473	1.188	-1.137	3.403	2.071	-2.039	4.110	15	
16	7.3	16.5	2.6	422	2.818	-1.885	5.387	3.810	-2.957	6.886	16	
17	1.1	16.7	4.4	489	1.000	517	5.839	3.888	-2.171	5.858	17	
18	8.6	17.2	2.2	494	-1.394	3.288	4.127	4.088	-2.173	6.241	18	
19	8.6	17.1	2.2	498	-2.085	2.912	4.608	4.637	-2.780	7.427	19	
20	7.7	18.2	2.2	434	-1.341	4.187	2.581	4.445	-1.820	6.285	20	
21	8.6	17.2	2.2	494	-1.989	2.626	1.773	2.831	-1.195	4.028	21	
22	444.3	388.6	186.0	244	-.018	2.687	-.043	2.697	-.018	3.044	22	
23	406.8	308.5	165.5	229	-006	235	-094	243	-015	258	23	
24	403.9	275.1	169.4	192	023	118	-.047	123	017	108	24	
25	408.9	229.7	187.1	093	-001	000	001	000	-001	001	25	
26	.0	1.1	0	485	408	-488	1.034	588	-1.079	1.687	26	
27	4	13.4	0	494	2.079	-1.719	1.236	2.177	-1.817	3.994	27	
28	2.9	13.7	1.0	484	810	-577	-187	816	-583	1.399	28	
29	11.9	15.9	4.4	363	139	1.192	871	1.289	042	1.248	29	
30	5	15.2	2	484	-782	2.612	1.375	2.746	-897	3.643	30	
31	8	15.9	2	494	-1.170	3.243	-2.779	3.844	-1.571	5.215	31	
32	8	15.6	2	494	-1.288	3.029	-3.518	3.858	-1.887	5.548	32	
33	7	17.8	2	494	-1.491	4.816	-4.07	4.922	-1.497	6.419	33	
34	7	17.2	2	493	-245	3.973	271	3.977	-248	4.226	34	
35	250.5	228.7	97.3	287	-059	555	239	577	-082	4.859	35	
36	370.8	228.0	181.8	145	-.048	4.215	1.802	4.783	-216	4.999	36	
37	1	6.0	0	495	013	248	17.380	8.808	-8.574	17.382	37	
38	134.4	414.1	46.2	244	-700	1.899	18.843	10.892	-9.415	20.047	38	
39	84.3	408.2	28.8	485	-1.233	3.852	10.680	7.081	-8.823	11.744	39	
40	1	4.0	0	495	-815	4.327	2.899	4.701	-1.289	5.991	40	
41	1	4.0	0	495	-878	5.391	4.82	5.401	-947	6.388	41	
42	1	4.0	0	495	-877	4.468	1.552	4.581	-948	5.519	42	
43	413.3	450.5	155.7	327	-427	3.080	-853	3.110	-457	3.587	43	
44	526.1	487.2	205.7	280	-180	2.822	-450	2.839	-187	3.036	44	
45	842.8	587.2	348.9	208	044	2.521	2.897	3.188	-823	3.811	45	
46	1	4.0	0	495	024	-224	1.823	871	-893	1.824	46	
47	1	4.0	0	495	115	-037	1.729	907	-829	1.735	47	
48	195.4	421.0	88.9	418	078	-005	361	222	-149	371	48	
49	1	4.0	0	495	-314	559	1.258	887	-843	1.530	49	
50	1	4.0	0	495	-304	2.053	2.072	2.409	-981	3.370	50	
51	1213.4	809.8	572.9	148	-532	3.841	-1.304	3.937	-827	4.584	51	
52	1859.3	930.5	760.0	092	003	2.398	-1.347	2.574	-173	2.748	52	
53	725.6	542.5	295.5	228	078	076	058	081	-083	184	53	
54	1	4.0	0	495	131	-088	1.015	542	-586	1.038	54	
55	1273.3	838.0	610.3	043	042	009	024	045	005	040	55	
56	80.8	407.8	27.5	488	103	-037	384	228	-182	390	56	
57	1	4.0	0	495	-818	099	1.306	488	-1.003	1.489	57	
58	705.3	931.0	283.7	237	-402	2.814	2.051	3.215	-803	4.018	58	
59	1255.3	778.1	848.2	149	-189	2.723	2.004	3.037	-474	3.911	59	
60	982.5	562.1	444.8	104	-113	071	025	072	-114	186	60	
61	1	4.0	0	495	135	-013	1.080	911	-489	1.100	61	

82	190.8	421.0	87.2	.420	004	.019	-.038	.032	-.008	.041	62
83	530.8	487.8	207.7	.278	079	-.044	.128	108	-.072	.178	83
84	.1	4.0	.0	.495	2.156	8.422	23.222	16.084	-7.816	23.610	84
85	.1	4.0	.0	.495	-4.183	20.808	10.185	21.901	-5.158	27.059	85
86	.1	4.0	.0	.495	-3.672	17.585	-51.290	34.724	-20.401	55.525	86
87	.1	4.0	.0	.495	4.562	-.482	-38.437	21.424	-17.343	38.787	87
88	.1	4.0	.0	.495	-.040	.027	-.320	.157	-.170	.327	88
89	383.8	443.8	135.0	.348	078	-.057	.076	088	-.067	.155	89
90	.1	4.0	.0	.495	2.404	5.967	-18.380	12.557	-4.186	18.743	90
91	.1	4.0	.0	.495	-4.255	20.581	-1.570	20.816	-4.279	24.895	91
92	.1	4.0	.0	.495	7.342	18.166	21.022	21.938	-11.116	33.054	92
93	.1	4.0	.0	.495	8.187	-2.888	6.290	8.012	-3.794	12.806	93
94	.1	4.0	.0	.495	0.065	-.028	-4.319	2.178	-2.141	4.320	94
95	807.5	508.0	240.3	.284	001	.024	.169	088	-.073	.171	95
96	503.8	483.8	195.4	.289	-.101	.188	-.208	.221	-.134	.355	96
97	.1	4.0	.0	.495	-5.897	8.738	8.888	8.466	-7.423	15.888	97
98	.1	4.0	.0	.495	5.167	-.513	8.676	7.937	-3.283	11.220	98

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-2	TAU-MAX	THETA	SIG1/SIG3	LEVEL	ELE
1	.245	481	005	481	.245	118	1.228	1.986	.078	1
2	.254	528	023	531	.252	138	4.703	2.104	.091	2
3	.357	.374	.127	.482	.238	.127	43.076	2.088	.085	3
4	.376	547	005	547	.376	085	1.840	1.484	1.707	4
5	.382	547	015	.548	.388	092	4.879	1.482	1.810	5
6	.488	664	013	665	.488	098	3.934	1.419	1.984	6
7	.524	.725	.008	.728	.524	.101	1.851	1.384	2.010	7
8	.346	537	-.002	537	.346	085	-.577	1.580	1.904	8
9	.358	.981	011	.982	.358	.312	-.973	2.739	.181	9
10	.416	1.438	.084	1.441	.413	.514	2.996	3.482	.283	10
11	.420	1.448	.140	1.465	.421	.522	7.768	3.483	.285	11
12	.386	901	-.084	.914	.352	.281	-8.953	2.598	.165	12
13	.140	.278	-.003	.278	.140	089	-1.332	1.991	.052	13
14	.171	.280	.038	.285	.165	.058	22.043	1.709	.081	14
15	.105	.174	.018	.177	.102	.038	12.384	1.741	.753	15
16	.272	.277	.037	.312	.237	.038	43.238	1.315	.750	16
17	.321	408	.014	410	.319	.048	8.812	1.285	.810	17
18	.273	481	.008	481	.272	.065	-.552	1.283	1.091	18
19	.382	.478	.008	.478	.381	.067	4.583	1.317	1.187	19
20	.501	.818	.008	.818	.500	.057	3.124	1.228	1.146	20
21	.331	436	.004	436	.331	.053	2.005	1.319	1.084	21
22	.381	1.628	.082	1.633	.376	.628	3.730	4.342	.359	22
23	.313	1.220	-.158	1.248	.287	.480	-9.800	4.338	.302	23
24	.275	.863	-.080	.879	.289	.210	-11.165	2.819	.138	24
25	.042	.081	.002	.081	.042	.024	2.188	2.188	.021	25
26	.008	.002	.005	.008	.004	.008	84.329	-2.089	-1.000	26
27	.100	.010	.028	.107	.003	.082	75.012	38.624	1.046	27
28	.083	.038	-.031	.084	.018	.033	-68.719	4.833	.889	28
29	.177	.200	.005	.201	.176	.012	12.027	1.140	.247	29
30	.208	.323	-.008	.323	.209	.057	-2.804	1.546	1.140	30
31	.251	.382	-.008	.382	.250	.081	-4.509	1.410	1.025	31
32	.203	.303	-.018	.305	.200	.052	-8.828	1.524	1.080	32
33	.483	.592	-.005	.592	.483	.055	-1.884	1.227	1.088	33
34	.540	.838	.002	.838	.540	.048	9.14	1.177	.859	34
35	.481	1.757	.048	1.740	.478	.431	2.883	3.642	.377	35
36	.128	.307	-.011	.307	.127	.090	-3.428	2.417	.088	36
37	.014	.041	-.064	.083	-.028	.068	-37.848	-2.871	-1.000	37
38	.033	.105	.040	.122	.018	.053	23.988	7.808	2.530	38
39	.138	.328	.178	.433	.034	.198	30.448	12.548	4.293	39
40	.027	.221	-.016	.222	.025	.088	-4.681	8.804	2.901	40
41	.040	.368	-.002	.368	.040	.184	-.380	8.283	3.088	41
42	.035	.311	-.003	.324	.022	.151	-12.357	16.087	5.237	42

43	.181	1.181	-.430	1.341	.022	.888	-20.348	61.082	22.328	43
44	.280	1.128	.110	1.422	.278	.433	7.348	4.157	1.158	44
45	-.084	.403	.102	.425	-.075	.280	12.038	-8.836	-1.000	45
46	-.018	.002	.013	.008	-.024	.017	28.525	-.349	-1.000	46
47	.088	.181	.108	.222	-.004	.113	38.213	-87.502	-1.000	47
48	.380	.371	.330	.711	.051	.330	45.848	14.088	4.858	48
49	.070	.227	-.182	.328	-.032	.140	-32.071	-10.289	-1.000	49
50	.029	.272	-.040	.278	-.023	.128	-9.019	12.192	4.181	50
51	-.881	1.089	-.584	1.244	-.837	1.041	-17.384	-1.486	-1.000	51
52	-.849	.537	.237	.987	-.880	.924	7.423	-1.099	-1.000	52
53	.381	.323	.148	.384	.382	.378	1.407	7.828	-1.000	53
54	.083	.071	.078	.156	-.001	.078	47.178	-138.844	-1.000	54
55	.510	.189	.159	.573	.107	.233	88.483	8.374	1.826	55
56	.383	.270	.205	.538	.114	.212	52.880	4.717	1.382	56
57	.076	.278	-.087	.308	-.044	.132	-20.518	7.010	2.234	57
58	-.815	.912	-.270	.958	-.881	.810	-9.742	-1.449	-1.000	58
59	-.251	.771	.018	.771	-1.252	1.011	4.485	-.815	-1.000	59
60	.822	.308	.081	.315	-.828	.872	4.589	-.381	-1.000	60
61	.012	.022	.062	.022	-.015	.005	15.368	1.788	.288	61
62	.186	.131	.049	.155	.083	.081	52.159	2.100	.408	62
63	.632	-.082	.300	.884	-.205	.628	87.841	-3.198	-1.000	63
64	.445	.541	-.077	.584	.402	.091	-28.040	1.453	.188	64
65	.874	.711	.004	.712	.874	.019	8.191	1.058	.021	65
66	.584	.597	-.020	.607	.584	.027	-28.147	1.086	.038	66
67	.214	.281	-.018	.285	.210	.037	-12.777	1.354	.132	67
68	.081	.018	.034	.080	-.001	.041	61.107	-83.778	-1.000	68
69	.323	.073	.134	.371	-.118	.243	72.748	-3.218	-1.000	69
70	.418	.445	-.049	.462	.381	.050	-37.384	1.284	.098	70
71	.858	.895	-.001	.895	.858	.018	-.974	1.058	.021	71
72	.438	.478	.005	.477	.437	.020	12.087	1.098	.038	72
73	.229	.238	.003	.238	.229	.005	15.871	1.042	.018	73
74	.010	.018	.000	.018	.010	.004	2.307	1.848	.314	74
75	.087	.213	.407	.581	-.262	.412	40.599	-2.142	-1.000	75
76	.148	1.000	.403	1.128	-.274	.701	-17.558	-4.118	-1.000	76
77	.084	.081	.004	.081	.083	.014	8.182	1.529	.197	77
78	.205	.217	.004	.218	.204	.007	15.870	1.072	.027	78

SSTIPN: 1 LAYER OF GEOTEXTILE, P=2000 PPI, E=5%, EXTENDED, T=1000

TOTAL NUMBER OF NODES----- 85
NUMBER OF BAR ELEMENTS----- 11
NUMBER OF DIFF BAR MATERIALS----- 1
NUMBER OF BEAM ELEMENTS----- 0
NUMBER OF DIFF BEAM MATERIALS----- 0
NUMBER OF NODAL LINKS----- 0
NUMBER OF INTERFACE ELEMENTS----- 0
NO OF INTERFACE ELE IN PREEXIST PART 0
NUMBER OF INTERFACE ELE IN FOUNDATION 0
NUMBER OF INTERFACE MATERIALS----- 0
TOTAL NUMBER OF SOIL ELEMENTS----- 78
NUMBER OF DIFF SOIL MATERIALS----- 3
NUMBER OF ELEMENTS IN FOUNDATION----- 38
NUMBER OF NODES IN FOUNDATION----- 52
NUMBER OF PREEEXISTING ELEMENTS----- 0
NUMBER OF PREEEXISTING NODES----- 0
NUMBER OF CONSTRUCTION LAYERS----- 6
NUMBER OF LOAD CASES----- 2

CALING FACTOR ----- 1 00000

ATMOSPHERIC PRESSURE --- 1 05800

UNIT WEIGHT OF WATER --- 03120

COMPUTATION SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO 1 APPLY LOAD CASE 1
INCREMENT NO 2 PUT ON LAYER NO 1
INCREMENT NO 3 PUT ON LAYER NO 2
INCREMENT NO 4 PUT ON LAYER NO 3
INCREMENT NO 5 PUT ON LAYER NO 4
INCREMENT NO 6 PUT ON LAYER NO 5
INCREMENT NO 7 PUT ON LAYER NO 6
INCREMENT NO 8 APPLY LOAD CASE 2

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		B. C. CODE		
	X-ORD	Y-ORD	X	Y	ZZ
1	000	000	1	1	1
2	10 000	000	1	1	1
3	20 000	000	1	1	1
4	30 000	000	1	1	1
5	38 000	000	1	1	1
6	42 000	000	1	1	1
7	50 000	000	1	1	1
8	54 000	000	1	1	1
9	58 000	000	1	1	1
10	68 000	000	1	1	1
11	74 000	000	1	1	1
12	82 000	000	1	1	1
13	90 000	000	1	1	1
14	000	4 000	0	0	0
15	10 000	4 000	0	0	0
16	20 000	4 000	0	0	0
17	30 000	4 000	0	0	0
18	38 000	4 000	0	0	0
19	42 000	4 000	0	0	0
20	50 000	4 000	0	0	0
21	54 000	4 000	0	0	0
22	58 000	4 000	0	0	0
23	68 000	4 000	0	0	0
24	74 000	4 000	0	0	0
25	82 000	4 000	0	0	0
26	90 000	4 000	1	0	1
27	000	7 000	1	0	1
28	10 000	7 000	0	0	0
29	20 000	7 000	0	0	0
30	30 000	7 000	0	0	0
31	38 000	7 000	0	0	0
32	42 000	7 000	0	0	0
33	50 000	7 000	0	0	0
34	54 000	7 000	0	0	0
35	58 000	7 000	0	0	0
36	68 000	7 000	0	0	0
37	74 000	7 000	0	0	0
38	82 000	7 000	0	0	0
39	90 000	7 000	1	0	1
40	000	10 000	1	0	1
41	10 000	10 000	0	0	0
42	20 000	10 000	0	0	0
43	30 000	10 000	0	0	0
44	38 000	10 000	0	0	0
45	42 000	10 000	0	0	0
46	50 000	10 000	0	0	0
47	54 000	10 000	0	0	0
48	58 000	10 000	0	0	0
49	68 000	10 000	0	0	0
50	74 000	10 000	0	0	0
51	82 000	10 000	0	0	0
52	90 000	10 000	1	0	1
53	000	11 500	0	0	0
54	10 000	11 500	0	0	0
55	20 000	11 500	0	0	0
56	30 000	11 500	0	0	0
57	38 000	11 500	0	0	0
58	42 000	11 500	0	0	0
59	50 000	11 500	0	0	0
60	54 000	11 500	0	0	0
61	58 000	11 500	1	0	1
62	42 000	13 000	0	0	0
63	50 000	13 000	0	0	0
64	54 000	13 000	0	0	0
65	58 000	13 000	0	0	0
66	68 000	13 000	0	0	0
67	74 000	13 000	0	0	0
68	82 000	13 000	0	0	0
69	90 000	13 000	1	0	1
70	48 000	14 000	0	0	0
71	50 000	14 000	0	0	0
72	54 000	14 000	0	0	0
73	58 000	14 000	0	0	0
74	68 000	14 000	0	0	0
75	74 000	14 000	0	0	0
76	82 000	14 000	0	0	0
77	90 000	14 000	1	0	1
78	50 000	15 000	0	0	0
79	54 000	15 000	0	0	0
80	58 000	15 000	0	0	0
81	68 000	15 000	0	0	0
82	74 000	15 000	0	0	0
83	82 000	15 000	0	0	0
84	90 000	15 000	1	0	1
85	54 000	16 000	0	0	0
86	58 000	16 000	0	0	0
87	68 000	16 000	0	0	0
88	74 000	16 000	0	0	0
89	82 000	16 000	0	0	0
90	90 000	16 000	1	0	1
91	58 000	17 000	0	0	0
92	68 000	17 000	0	0	0
93	74 000	17 000	0	0	0
94	82 000	17 000	0	0	0
95	90 000	17 000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	240	1.00	6.

ELMY NO	CONNECTED NODES I J	MATL NO
1	41 42	1
2	42 43	1
3	43 44	1
4	44 45	1
5	45 46	1
6	46 47	1
7	47 48	1
8	48 49	1
9	49 50	1
10	50 51	1
11	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S CONSTANT	MODULUS EXPONENT	RATIO	BULK CONSTANT	MODULUS EXPONENT	STRENGTH C	PARAMETERS PHI	DPHI	K0
1	0800	8000 00	500	500	1800 00	500	00	35.00	00	50
2	0830	40 00	300	500	20 00	200	00	00	00	50
3	0800	1000 00	400	700	500 00	500	50	40 00	00	50

FOUR NODES SOLID ELEMENT DATA

ELEM NO.	CONNECTED NODES I J K L	MATL NO.	ELEMENT CENTER X-ORD	Y-ORD
1	1 2 15 14	3	5.000	2.000
2	2 3 16 15	3	5.000	2.000
3	3 4 17 16	3	25.000	2.000
4	4 5 18 17	2	33.000	2.000
5	5 6 19 18	2	38.000	2.000
6	6 7 20 19	2	48.000	2.000
7	7 8 21 20	2	52.000	2.000
8	8 9 22 21	2	56.000	2.000
9	9 10 23 22	3	62.000	2.000
10	10 11 24 23	3	70.000	2.000
11	11 12 25 24	3	78.000	2.000
12	12 13 26 25	3	88.000	2.000
13	13 14 27 26	3	5.000	5.500
14	14 15 28 27	3	15.000	5.500
15	15 16 29 28	2	25.000	5.500
16	16 17 30 29	2	35.000	5.500
17	17 18 31 30	2	45.000	5.500
18	18 19 32 31	2	55.000	5.500
19	19 20 33 32	2	65.000	5.500
20	20 21 34 33	2	75.000	5.500
21	21 22 35 34	2	85.000	5.500
22	22 23 36 35	2	95.000	5.500
23	23 24 37 36	3	105.000	5.500
24	24 25 38 37	3	115.000	5.500
25	25 26 39 38	3	125.000	5.500
26	26 27 40 39	3	135.000	5.500
27	27 28 41 40	3	145.000	5.500
28	28 29 42 41	2	15.000	8.500
29	29 30 43 42	2	25.000	8.500
30	30 31 44 43	2	35.000	8.500
31	31 32 45 44	2	45.000	8.500
32	32 33 46 45	2	55.000	8.500
33	33 34 47 46	2	65.000	8.500
34	34 35 48 47	2	75.000	8.500
35	35 36 49 48	2	85.000	8.500
36	36 37 50 49	2	95.000	8.500
37	37 38 51 50	3	105.000	8.500
38	38 39 52 51	3	115.000	8.500
39	39 40 53 52	1	125.000	10.750
40	40 41 54 53	1	135.000	10.750
41	41 42 55 54	1	145.000	10.750
42	42 43 56 55	1	15.000	10.750
43	43 44 57 56	1	25.000	10.750
44	44 45 58 57	1	35.000	10.750
45	45 46 59 58	1	45.000	10.750
46	46 47 60 59	1	55.000	10.750
47	47 48 61 60	1	65.000	10.750
48	48 49 62 61	1	75.000	10.750
49	49 50 63 62	1	85.000	10.750
50	50 51 64 63	1	95.000	10.750
51	51 52 65 64	1	105.000	10.750
52	52 53 66 65	1	115.000	10.750
53	53 54 67 66	1	125.000	10.750
54	54 55 68 67	1	135.000	10.750
55	55 56 69 68	1	145.000	10.750
56	56 57 70 69	1	15.000	13.500
57	57 58 71 70	1	25.000	13.500
58	58 59 72 71	1	35.000	13.500
59	59 60 73 72	1	45.000	13.500
60	60 61 74 73	1	55.000	13.500
61	61 62 75 74	1	65.000	13.500
62	62 63 76 75	1	75.000	13.500
63	63 64 77 76	1	85.000	13.500
64	64 65 78 77	1	95.000	13.500
65	65 66 79 78	1	105.000	13.500
66	66 67 80 79	1	115.000	13.500
67	67 68 81 80	1	125.000	13.500
68	68 69 82 81	1	135.000	13.500
69	69 70 83 82	1	145.000	13.500
70	70 71 84 83	1	15.000	16.500
71	71 72 85 84	1	25.000	16.500
72	72 73 86 85	1	35.000	16.500
73	73 74 87 86	1	45.000	16.500
74	74 75 88 87	1	55.000	16.500
75	75 76 89 88	1	65.000	16.500
76	76 77 90 89	1	75.000	16.500
77	77 78 91 90	1	85.000	16.500
78	78 79 92 91	1	95.000	16.500
79	79 80 93 92	1	105.000	16.500
80	80 81 94 93	1	115.000	16.500
81	81 82 95 94	1	125.000	16.500

SSSTIPN: 1 LAYER OF GEOTEXTILE, P=2000 PPI, E=5%, EXTENDED, T=1000

* LOAD CASE ----- 2 *

LARGEST ELE NO. IN THIS INCREMENT 78
LARGEST N P NO IN THIS INCREMENT 95

BAND WIDTH----- 40
TOTAL NUMBER OF EQUATIONS----- 222
NUMBER OF EQUATIONS IN BLOCK----- 64
NUMBER OF BLOCKS----- 3
NUMBER OF "N.P." FORCE CARDS----- 3
NUMBER OF PRESSURE CARDS----- 0

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP	X-FORCE	Y-FORCE
1	0	0
2	0	0
3	0	0
4	0	0
5	0	0
6	0	0
7	0	0
8	0	0
9	0	0
10	0	0
11	0	0
12	0	0
13	0	0
14	0	0
15	0	0
16	0	0
17	0	0
18	0	0
19	0	0
20	0	0
21	0	0
22	0	0
23	0	0
24	0	0
25	0	0
26	0	0
27	0	0
28	0	0
29	0	0
30	0	0
31	0	0
32	0	0
33	0	0
34	0	0
35	0	0
36	0	0
37	0	0
38	0	0
39	0	0
40	0	0
41	0	0
42	0	0
43	0	0
44	0	0
45	0	0
46	0	0
47	0	0
48	0	0
49	0	0
50	0	0
51	0	0
52	0	0
53	0	0
54	0	0
55	0	0
56	0	0
57	0	0
58	0	0
59	0	0
60	0	0
61	0	0
62	0	0

63 0 0
 64 0 0
 65 0 0
 66 0 0
 67 0 0
 68 0 0
 69 0 0
 70 0 0
 71 0 0
 72 0 0
 73 0 0
 74 0 0
 75 0 0
 76 0 0
 77 0 0
 78 0 0
 79 0 0
 80 0 0
 81 0 0
 82 0 0
 83 0 0
 84 0 0
 85 0 0
 86 0 0
 87 0 0
 88 0 0
 89 0 0
 90 0 0
 91 0 0
 92 0 -4.50
 93 0 -4.50
 94 0 -2.00
 95 0 0

LOAD CASE : 2 ITERATION : 2

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	0000	0000	0000	0000	0000	0000	0000	1
2	0000	0000	0000	0000	0000	0000	0000	2
3	0000	0000	0000	0000	0000	0000	0000	3
4	0000	0000	0000	0000	0000	0000	0000	4
5	0000	0000	0000	0000	0000	0000	0000	5
6	0000	0000	0000	0000	0000	0000	0000	6
7	0000	0000	0000	0000	0000	0000	0000	7
8	0000	0000	0000	0000	0000	0000	0000	8
9	0000	0000	0000	0000	0000	0000	0000	9
10	0000	0000	0000	0000	0000	0000	0000	10
11	0000	0000	0000	0000	0000	0000	0000	11
12	0000	0000	0000	0000	0000	0000	0000	12
13	0000	0000	0000	0000	0000	0000	0000	13
14	0000	0003	0000	0000	0013	0000	0013	14
15	0002	0001	0000	0008	0007	0000	0010	15
16	0001	0000	0000	0003	0005	0000	0006	16
17	0004	0001	0000	0048	0022	0000	0051	17
18	0152	0089	0000	1782	0198	0000	1793	18
19	0312	0020	0000	2467	0739	0000	2575	19
20	0350	0055	0000	1461	1162	0000	1667	20
21	0248	0188	0000	0882	1350	0000	1474	21
22	0008	0009	0000	0001	0052	0000	0052	22
23	0008	0018	0000	0001	0049	0000	0049	23
24	0009	0054	0000	0017	0104	0000	0105	24
25	0021	0022	0000	0024	0057	0000	0082	25
26	0000	0002	0000	0000	0030	0000	0030	26
27	0000	0005	0000	0000	0024	0000	0024	27
28	0005	0003	0000	0026	0014	0000	0030	28
29	0000	0000	0000	0003	0006	0000	0007	29
30	0130	0038	0000	1283	0813	0000	1383	30
31	0258	0183	0000	2137	0402	0000	2174	31
32	0491	0105	0000	2741	1256	0000	2914	32
33	0830	0048	0000	2883	2338	0000	3248	33
34	0889	0174	0000	2252	2199	0000	2474	34
35	0829	0428	0000	1855	1711	0000	2280	35
36	0008	0048	0000	0020	0131	0000	0133	36
37	0022	0104	0000	0031	0190	0000	0192	37
38	0028	0041	0000	0026	0109	0000	0112	38
39	0000	0012	0000	0000	0049	0000	0049	39
40	0000	0006	0000	0000	0032	0000	0032	40
41	0025	0078	0000	0113	0028	0000	0115	41
42	0160	0027	0000	0800	0142	0000	0617	42
43	0300	0090	0000	1081	0923	0000	1422	43
44	0339	0185	0000	1189	0294	0000	1206	44
45	0388	0172	0000	1259	1681	0000	2084	45
46	0422	0028	0000	1180	2001	0000	2225	46
47	0375	0709	0000	0993	2980	0000	3141	47
48	0225	0812	0000	0886	2723	0000	2808	48
49	0023	0848	0000	0788	2097	0000	2105	49
50	0020	0250	0000	0005	0441	0000	0441	50
51	0002	0039	0000	0013	0126	0000	0127	51
52	0000	0018	0000	0000	0084	0000	0084	52
53	0380	0180	0000	1012	0723	0000	1244	53
54	0361	0176	0000	0931	0928	0000	1322	54
55	0392	0028	0000	0907	2259	0000	2434	55
56	0397	0225	0000	0685	2706	0000	2847	56
57	0114	0827	0000	1018	2991	0000	3180	57
58	0099	0864	0000	0288	2297	0000	2428	58
59	0048	0284	0000	0449	0911	0000	1015	59
60	0021	0042	0000	0032	0121	0000	0125	60

61	0000	0019	0000	0000	- 0065	0000	0085	61
62	- 0327	0178	0000	- 0841	- 0186	0000	0583	62
63	0382	- 0013	0000	- 0804	- 1223	0000	1384	63
64	- 0413	- 0232	0000	- 0388	- 1828	0000	1784	64
65	- 0882	- 0852	0000	- 0851	- 2217	0000	2375	65
66	- 0231	- 0893	0000	- 0529	- 1721	0000	1799	66
67	- 0104	- 0271	0000	- 0281	- 1197	0000	1233	67
68	- 0080	- 0048	0000	- 0087	- 0111	0000	0129	68
69	0000	0020	0000	0000	- 0043	0000	0043	69
70	0317	0083	0000	- 0438	- 0355	0000	0581	70
71	0318	- 0004	0000	- 0473	- 0788	0000	0888	71
72	- 0321	- 0230	0000	- 0511	- 1131	0000	1241	72
73	- 0408	- 0828	0000	- 0628	- 1739	0000	1848	73
74	- 0818	- 2858	0000	- 0768	- 3467	0000	3550	74
75	- 0132	- 0281	0000	- 0272	- 1308	0000	1337	75
76	- 0072	- 0048	0000	- 0089	- 0104	0000	0137	76
77	0000	0019	0000	0000	- 0030	0000	0030	77
78	0270	0009	0000	0388	0388	0000	0511	78
79	0214	0231	0000	0318	0887	0000	0737	79
80	0204	- 0819	0000	- 0328	- 1277	0000	1318	80
81	0241	- 2873	0000	- 0378	- 3229	0000	3251	81
82	0847	- 2853	0000	0838	- 3407	0000	3508	82
83	8845	- 8897	0000	8828	- 8842	0000	8857	83
84	0000	0814	0000	0000	0880	0000	0880	84
85	- 0108	- 0284	0000	- 0147	- 0408	0000	0432	85
86	0017	- 0814	0000	- 0038	- 0988	0000	0987	86
87	0831	- 2893	0000	0828	- 3088	0000	3084	87
88	0042	- 8033	0000	0011	- 8338	0000	8338	88
89	5380	- 1223	0000	8038	- 1488	0000	5242	89
90	0000	2353	0000	0000	2188	0000	2188	90
91	0278	- 0212	0000	0278	- 0812	0000	0857	91
92	0294	- 2889	0000	0294	- 2889	0000	2914	92
93	0312	- 8048	0000	0312	- 8048	0000	8055	93
94	2718	- 2477	0000	2718	- 2477	0000	2878	94
95	0000	3728	0000	0000	3728	0000	3728	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO AXIAL FORCE

INCREMENTAL VALUES

1	3257
2	3258
3	1540
4	2027
5	0877
6	2842
7	8988
8	8048
9	1308
10	0888
11	0062

TOTAL VALUES

1	-1 1895
2	-1 1834
3	- 3834
4	- 3813
5	2383

6	1.1202
7	1.8483
8	1.4988
9	8428
10	0224
11	0378

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	840.8	342.8	233.0	180	.004	.008	.000	.008	.004	.004	1
2	800.3	314.8	216.3	17	.002	.002	.020	.008	.012	.020	2
3	534.7	334.4	231.8	1	.021	.021	.048	.032	.032	.084	3
4	8	17.2	2	484	.571	.876	6.082	3.888	-2.423	8.082	4
5	8	17.2	2	484	.571	.876	6.082	3.888	-2.423	8.082	5
6	8	17.2	2	484	.571	.876	6.082	3.888	-2.423	8.082	6
7	8	17.2	2	484	.571	.876	6.082	3.888	-2.423	8.082	7
8	8	17.2	2	484	.571	.876	6.082	3.888	-2.423	8.082	8
9	475.0	343.8	195.3	216	.001	.128	.002	.128	.001	.127	9
10	472.8	382.7	191.2	238	.010	.181	.012	.182	.010	.202	10
11	478.4	373.6	191.5	244	.005	.201	.081	.208	.012	.221	11
12	613.4	366.1	212.1	210	.018	.108	.047	.114	.010	.104	12
13	530.1	315.0	235.3	127	.017	.008	.003	.017	.008	.011	13
14	374.2	222.8	186.0	127	.017	.010	.016	.006	.032	.037	14
15	1.7	13.5	8	479	.854	.817	1.744	1.860	-1.222	2.282	15
16	7.9	16.9	2.8	417	2.174	-1.157	2.584	2.811	-1.593	4.204	16
17	1.3	16.9	4	484	1.074	.520	3.210	2.428	.831	3.257	17
18	8	17.2	2	484	-1.754	2.450	3.034	3.231	-1.384	4.588	18
19	8	17.2	2	484	-1.386	3.207	4.571	4.188	-2.385	6.581	19
20	8	17.2	2	484	-1.487	4.181	3.288	4.824	-1.939	8.583	20
21	8	17.2	2	484	-1.043	2.903	1.737	3.088	-1.230	4.316	21
22	442.2	310.3	177.2	244	.017	.280	.016	.281	.017	.284	22
23	404.2	310.3	185.5	233	.002	.230	.107	.242	.014	.288	23
24	480.8	313.0	193.5	191	.032	.114	.087	.127	.023	.104	24
25	337.8	215.8	145.1	184	.070	.011	.085	.082	.033	.128	25
26	4	13.4	1	488	.229	-.271	1.102	.884	-.828	1.210	26
27	0	1	1	488	-.932	.052	.052	.874	-.932	1.808	27
28	1.6	13.5	5	481	.802	-.505	-1.288	1.070	-.773	1.842	28
29	1.6	14.7	5	481	.579	.857	-1.073	1.272	.184	1.108	29
30	8	15.2	2	484	.205	1.882	-3.208	2.808	-1.086	3.884	30
31	8	15.8	2	484	.534	2.578	-4.389	3.898	-1.537	5.388	31
32	8	15.5	2	484	-1.131	2.987	-4.844	4.032	-2.175	6.207	32
33	8	17.7	2	484	-1.389	4.942	-2.852	5.228	-1.828	8.584	33
34	8	17.1	3	482	-.120	3.884	-.538	3.717	-.049	3.880	34
35	289.4	288.0	117.0	278	.008	.447	-.133	.458	.001	.451	35
36	342.5	218.5	187.8	188	.008	.008	.001	.088	.009	.080	36
37	348.8	441.7	129.2	354	.008	-.083	.088	.184	.073	.238	37
38	178.8	418.7	81.5	427	.008	-.014	1.403	.913	.890	1.803	38
39	272.3	421.4	98.4	386	-.068	.108	.288	.182	-.133	.316	39
40	160.2	414.8	48.7	441	-.282	1.270	.012	1.270	-.282	1.522	40
41	1	4.0	0	485	-.208	3.488	1.866	3.668	-.388	4.021	41
42	302.1	438.3	110.0	374	-.888	3.882	1.842	1.888	-.813	4.802	42
43	837.4	888.7	249.9	278	.817	3.241	-2.588	3.828	1.002	4.828	43
44	1088.8	778.2	432.7	221	.309	1.723	-2.054	2.280	.247	2.527	44
45	1129.4	718.5	488.6	180	.028	.088	.011	.088	.028	.088	45
46	4	4.0	0	488	.048	.002	.056	.018	.058	.072	46
47	173.1	418.8	80.8	428	.080	.032	.433	.330	-.212	.441	47
48	194.1	421.4	88.4	419	.078	.017	.358	.325	-.233	.558	48
49	1	4.0	0	485	-.229	.822	2.839	1.810	-1.217	3.027	49
50	724.3	808.7	288.0	288	-.288	2.784	1.132	1.518	-.839	2.383	50
51	1788.8	888.7	820.2	077	-.182	2.888	-.382	2.702	-.198	2.897	51
52	1040.8	833.1	488.8	139	-.081	.088	.040	.088	-.064	.134	52
53	208.8	822.4	414.8	414	.070	.048	.071	.040	.088	.138	53
54	872.5	882.4	288.3	283	.123	-.024	-.048	.131	-.021	.181	54
55	803.3	482.7	185.2	280	.370	-.116	.318	.380	-.184	.548	55
56	1	4.0	0	485	-.124	9.730	1.801	9.821	-.218	10.038	56
57	1	4.0	0	485	-.443	12.804	.216	12.808	-.480	12.888	57
58	1189.6	731.4	518.8	148	-.201	2.708	-.722	2.780	-.248	2.888	58
59	782.2	508.5	320.5	183	-.087	.072	.033	.072	-.098	.171	59
60	78.0	407.8	28.9	.487	.084	.008	.049	.083	-.004	.087	60

62	308.8	438.8	111.8	372	-.033	.012	-.154	.070	-.080	.180	62
63	428.1	482.8	182.0	321	.125	-.042	-.028	-.128	-.044	.170	63
64	447.9	488.5	170.8	315	.107	.082	.138	.153	.008	.147	64
65	1	4.0	0	485	-1.032	14.008	-8.033	15.014	-2.037	17.081	65
66	1	4.0	0	485	-3.818	18.888	-11.880	28.742	-18.870	46.812	66
67	1	4.0	0	488	4.211	-1.878	-11.871	18.288	-16.789	36.074	67
68	188.8	418.1	88.3	430	-.127	.121	-.478	.284	-.271	.338	68
69	343.2	440.8	128.5	357	-.144	.037	-.184	.089	-.178	.248	69
70	488.7	488.1	178.4	308	.022	.036	.114	.118	-.012	.130	70
71	1	4.0	0	485	-.788	12.834	4.382	12.883	-1.118	13.889	71
72	1	4.0	0	485	-8.408	18.808	1.778	17.208	-7.948	28.341	72
73	1	4.0	0	488	7.418	-2.115	5.788	8.228	-2.825	11.154	73
74	1	4.0	0	488	-.315	-.023	-1.188	8.434	-.770	1.208	74
75	881.1	888.2	430.8	138	-.020	.018	-.013	.018	-.021	.028	75
76	1079.1	888.8	472.8	142	-.018	.032	.012	.082	-.018	.111	76
77	1	4.0	0	485	-4.838	8.338	7.988	7.818	-6.118	13.737	77
78	1	4.0	0	485	9.080	-.588	7.208	6.814	-2.343	9.157	78

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TMU-MAX	THEYA	SIG1/SIG3	LEVEL	ELE
1	270	823	-.001	829	270	129	1.131	1.947	.083	1
2	228	488	.004	478	218	128	10.010	2.178	.084	2
3	332	388	.006	478	318	117	20.370	1.881	.077	3
4	382	588	.004	548	373	087	1.457	1.484	1.731	4
5	382	588	.013	583	381	081	3.575	1.477	1.812	5
6	484	888	.011	857	483	087	3.328	1.418	1.537	6
7	517	718	.008	718	517	089	1.843	1.384	1.588	7
8	338	528	-.002	528	338	095	1.508	1.586	1.502	8
9	325	1.013	-.007	1.013	325	344	1.811	3.114	.207	9
10	398	1.371	.021	1.372	395	488	1.259	3.472	.274	10
11	488	1.442	.158	1.487	423	522	1.801	3.484	.284	11
12	394	888	-.083	873	377	.288	-9.788	2.578	.170	12
13	228	315	.006	315	225	.045	3.545	1.397	.030	13
14	081	218	.084	241	038	101	19.519	0.273	.083	14
15	074	182	.007	183	073	040	5.053	2.088	.794	15
16	278	283	.028	313	258	027	38.781	1.212	.546	16
17	335	418	.006	418	334	042	6.188	1.287	.857	17
18	371	473	.005	473	370	052	3.608	1.279	1.034	18
19	481	168	.010	168	483	085	5.183	1.104	1.034	19
20	513	827	.008	828	513	058	3.979	1.225	1.154	20
21	388	475	.004	475	388	083	1.918	1.281	1.070	21
22	381	1.545	.020	1.545	380	583	1.314	4.289	.344	22
23	334	1.218	-.120	1.258	339	478	-11.033	4.137	.287	23
24	334	712	-.111	742	308	218	-15.262	2.428	.134	24
25	311	084	.159	393	008	194	62.522	36.882	.179	25
26	002	.002	.000	.002	.001	.000	55.957	1.871	.009	26
27	.023	.000	.010	.034	.003	.017	72.548	-8.883	.000	27
28	088	032	-.035	086	009	039	-57.881	9.301	.781	28
29	188	245	-.003	243	185	.040	-2.120	1.408	.782	29
30	202	284	-.009	297	201	.049	-5.423	1.488	.981	30
31	268	345	-.012	345	247	050	-7.107	1.405	1.000	31
32	219	316	-.009	320	216	052	-11.184	1.483	1.041	32
33	518	824	-.020	825	515	055	-4.223	1.218	1.089	33
34	514	808	-.024	808	514	048	-2.585	1.185	.855	34
35	523	1.82	.151	1.81	510	601	-7.271	3.388	.302	35
36	081	381	.060	381	081	135	.009	4.351	.111	36
37	815	087	-.149	844	.058	393	-78.872	14.837	5.088	37
38	175	081	.084	227	039	084	58.186	5.802	1.785	38
39	184	332	.182	419	.078	171</				

43	- 182	1 181	- 868	1 819	- 820	1 120	26 256	-2 813	-1 000	43	
44	204	1 014	.305	1 116	105	505	18 552	10 805	3 570	44	
45	- 044	483	.035	.485	- 046	256	4 250	-10 033	-1 000	45	
46	010	028	.015	.037	000	018	31 723	780 280	288 677	46	
47	181	082	.152	.278	- 035	157	52 304	-7 832	-1 000	47	
48	448	485	.485	.532	000	488	43 881	-56280	340	-1 000	48
49	- 027	101	- 127	180	- 108	143	31 534	-1 701	-1 000	49	
50	103	.405	- 064	.427	081	173	-14 582	5 284	-1 582	50	
51	- 578	887	- 519	1 153	- 732	843	-15 715	-1 575	-1 000	51	
52	- 880	886	.086	.491	- 486	485	3 090	-1 005	-1 000	52	
53	- 402	378	.124	.369	- 422	410	5 083	- 848	-1 000	53	
54	245	030	.085	.281	- 006	143	88 282	-48 440	-1 000	54	
55	1 018	130	- 165	1 048	100	474	-78 851	10 484	3 518	55	
56	2 675	095	.658	2 255	- 108	1 180	73 066	-21 337	-1 000	56	
57	603	670	- 132	772	501	138	-37 782	1 843	202	57	
58	454	562	- 081	580	427	081	-24 359	1 381	142	58	
59	- 848	882	- 191	913	- 870	881	-8 188	-1 050	-1 000	59	
60	- 534	280	.082	.281	- 544	417	6 378	- 534	-1 000	60	
61	016	033	.056	.081	- 032	057	-40 705	-2 852	-1 000	61	
62	- 068	.031	- 108	.084	- 153	108	-41 272	- 420	-1 000	62	
63	800	020	.012	800	090	355	88 057	8 818	2 842	63	
64	887	708	.159	880	615	183	58 589	1 594	221	64	
65	828	557	- 003	558	528	015	-6 228	1 252	021	65	
66	822	585	- 017	582	515	024	-22 511	1 082	034	66	
67	186	302	- 014	304	.184	.055	-7 568	1 587	211	67	
68	137	118	- 248	288	- 288	279	-31 475	- 827	-1 000	68	
69	821	243	.171	.177	- 887	283	-21 076	- 237	-1 000	69	
70	448	685	.187	758	358	188	28 480	2 112	413	70	
71	328	575	.013	578	525	027	14 818	1 102	038	71	
72	388	432	.004	433	388	018	6 034	1 082	034	72	
73	232	242	.002	243	232	005	13 077	1 048	017	73	
74	- 008	004	- 002	004	- 008	005	10 037	- 788	-1 000	74	
75	- 158	170	- 064	178	- 168	173	-8 148	-1 084	-1 000	75	
76	- 016	1 036	.059	1 039	- 022	531	3 188	-47 722	-1 007	76	
77	072	098	.003	089	072	013	8 303	1 371	138	77	
78	187	210	.003	211	187	007	12 347	1 070	028	78	

SSTIPN: 1 LAYER OF GEOTEXTILE, P=2000 PPI, 2%SK, T=0

TOTAL NUMBER OF NODES-----	86
NUMBER OF BAR ELEMENTS-----	8
NUMBER OF DIFF BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFAC ELEMENTS-----	0
NO OF INTERFACE ELE IN PREEXIST PART	0
NUMBER OF INTERFACE ELE IN FOUNDATION	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	38
NUMBER OF NODES IN FOUNDATION-----	82
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	1
NUMBER OF LOAD CASES-----	1

CALING FACTOR ----- 1.00000

ATMOSPHERIC PRESSUPE --- 1.06800

UNIT WEIGHT OF WATER --- 0.3120

COMPUTATION SEQUENCE FOR A TOTAL OF 7 INCREMENTS

INCREMENT NO 1	PUT ON LAYER NO 1
INCREMENT NO 2	PUT ON LAYER NO 2
INCREMENT NO 3	PUT ON LAYER NO 3
INCREMENT NO 4	PUT ON LAYER NO 4
INCREMENT NO 5	PUT ON LAYER NO 5
INCREMENT NO 6	PUT ON LAYER NO 6
INCREMENT NO 7	APPLY LOAD CASE 1

MODAL POINT INPUT DATA

NODE NUMBER	MODAL POINT COORDINATES		B.C. CODE		
	X-ORD	Y-ORD	X	Y	Z
1	0.000	0.000	1	1	1
2	10.000	0.000	1	1	1
3	20.000	0.000	1	1	1
4	30.000	0.000	1	1	1
5	38.000	0.000	1	1	1
6	42.000	0.000	1	1	1
7	50.000	0.000	1	1	1
8	54.000	0.000	1	1	1
9	58.000	0.000	1	1	1
10	68.000	0.000	1	1	1
11	74.000	0.000	1	1	1
12	82.000	0.000	1	1	1
13	90.000	0.000	1	1	1
14	0.000	4.000	0	0	1
15	10.000	4.000	0	0	0
16	20.000	4.000	0	0	0
17	30.000	4.000	0	0	0
18	38.000	4.000	0	0	0
19	42.000	4.000	0	0	0
20	50.000	4.000	0	0	0
21	54.000	4.000	0	0	0
22	58.000	4.000	0	0	0
23	68.000	4.000	0	0	0
24	74.000	4.000	0	0	0
25	82.000	4.000	0	0	0
26	90.000	4.000	1	0	1
27	0.000	7.000	1	0	1
28	10.000	7.000	0	0	0
29	20.000	7.000	0	0	0
30	30.000	7.000	0	0	0
31	38.000	7.000	0	0	0
32	42.000	7.000	0	0	0
33	50.000	7.000	0	0	0
34	54.000	7.000	0	0	0
35	58.000	7.000	0	0	0
36	68.000	7.000	0	0	0
37	74.000	7.000	0	0	0
38	82.000	7.000	0	0	0
39	90.000	7.000	1	0	1
40	0.000	10.000	1	0	1
41	10.000	10.000	0	0	0
42	20.000	10.000	0	0	0
43	30.000	10.000	0	0	0
44	38.000	10.000	0	0	0
45	42.000	10.000	0	0	0
46	50.000	10.000	0	0	0
47	54.000	10.000	0	0	0
48	58.000	10.000	0	0	0
49	68.000	10.000	0	0	0
50	74.000	10.000	0	0	0
51	82.000	10.000	0	0	0
52	90.000	10.000	1	0	1
53	0.000	11.000	0	0	0
54	10.000	11.000	0	0	0
55	20.000	11.000	0	0	0
56	30.000	11.000	0	0	0
57	38.000	11.000	0	0	0
58	42.000	11.000	0	0	0
59	50.000	11.000	0	0	0
60	54.000	11.000	0	0	0
61	58.000	11.000	1	0	1
62	42.000	13.000	0	0	0
63	50.000	13.000	0	0	0
64	54.000	13.000	0	0	0
65	58.000	13.000	0	0	0
66	68.000	13.000	0	0	0
67	74.000	13.000	0	0	0
68	82.000	13.000	0	0	0
69	90.000	13.000	1	0	1
70	0.000	14.000	0	0	0
71	10.000	14.000	0	0	0
72	20.000	14.000	0	0	0
73	30.000	14.000	0	0	0
74	38.000	14.000	0	0	0
75	42.000	14.000	0	0	0
76	50.000	14.000	0	0	0
77	54.000	14.000	1	0	1
78	60.000	15.000	0	0	0
79	64.000	15.000	0	0	0
80	68.000	15.000	0	0	0
81	74.000	15.000	0	0	0
82	74.000	15.000	0	0	0
83	82.000	15.000	0	0	0
84	90.000	15.000	1	0	1
85	0.000	16.000	0	0	0
86	10.000	16.000	0	0	0
87	20.000	16.000	0	0	0
88	30.000	16.000	0	0	0
89	38.000	16.000	0	0	0
90	42.000	16.000	1	0	1
91	50.000	17.000	0	0	0
92	54.000	17.000	0	0	0
93	58.000	17.000	0	0	0
94	68.000	17.000	0	0	0
95	74.000	17.000	1	0	1

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	480	1.00	0

ELM NO	CONNECTED NODES I J	MATL NO
1	43 44	1
2	44 45	1
3	45 46	1
4	46 47	1
5	47 48	1
6	48 49	1
7	49 50	1
8	50 51	1
9	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT	WY	CONSTANT	YOUNG'S MODULUS EXPONENT	RATIO	BULK MODULUS CONSTANT	EXPONENT	STRENGTH C	PHI	DPHI	KD		
1	0820	8000	00	500	500	1500	00	800	00	35	00	00	50
2	0530	40	00	300	300	20	00	200	05	00	00	00	50
3	0800	1000	00	400	700	500	00	500	50	40	00	00	50

Node Nodes Solid Element Data

ELET NO	CONNECTED NODES I J K L	MATL NO	ELEMENT CENTER COORDINATES Y-ORD	Y-ORD
1	1 2 15 14	3	5.000	2.000
2	2 3 16 15	3	15.000	2.000
3	3 4 17 16	3	25.000	2.000
4	4 5 18 17	2	35.000	2.000
5	5 6 19 18	2	39.000	2.000
6	6 7 20 19	2	48.000	2.000
7	7 8 21 20	2	52.000	2.000
8	8 9 22 21	2	58.000	2.000
9	9 10 23 22	3	62.000	2.000
10	10 11 24 23	3	70.000	2.000
11	11 12 25 24	3	78.000	2.000
12	12 13 26 25	3	88.000	2.000
13	13 14 27 26	3	5.000	5.500
14	14 15 28 27	3	15.000	5.500
15	15 16 29 28	3	25.000	5.500
16	16 17 30 29	2	35.000	5.500
17	17 18 31 30	2	39.000	5.500
18	18 19 32 31	2	48.000	5.500
19	19 20 33 32	2	48.000	5.500
20	20 21 34 33	2	52.000	5.500
21	21 22 35 34	2	58.000	5.500
22	22 23 36 35	2	62.000	5.500
23	23 24 37 36	3	70.000	5.500
24	24 25 38 37	3	78.000	5.500
25	25 26 39 38	3	88.000	5.500
26	26 27 40 39	3	5.000	8.500
27	27 28 41 40	2	15.000	8.500
28	28 29 42 41	2	25.000	8.500
29	29 30 43 42	2	35.000	8.500
30	30 31 44 43	2	39.000	8.500
31	31 32 45 44	2	48.000	8.500
32	32 33 46 45	2	48.000	8.500
33	33 34 47 46	2	52.000	8.500
34	34 35 48 47	2	58.000	8.500
35	35 36 49 48	2	62.000	8.500
36	36 37 50 49	2	70.000	8.500
37	37 38 51 50	3	78.000	8.500
38	38 39 52 51	3	88.000	8.500
39	39 40 53 52	1	34.500	10.750
40	40 41 54 53	1	39.000	10.750
41	41 42 55 54	1	48.000	10.750
42	42 43 56 55	1	52.000	10.750
43	43 44 57 56	1	58.000	10.750
44	44 45 58 57	1	62.000	10.750
45	45 46 59 58	1	70.000	10.750
46	46 47 60 59	1	78.000	10.750
47	47 48 61 60	1	88.000	10.750
48	48 49 62 61	1	40.000	12.250
49	49 50 63 62	1	48.000	12.250
50	50 51 64 63	1	52.000	12.250
51	51 52 65 64	1	58.000	12.250
52	52 53 66 65	1	58.000	12.250
53	53 54 67 66	1	62.000	12.250
54	54 55 68 67	1	70.000	12.250
55	55 56 69 68	1	78.000	12.250
56	56 57 70 69	1	88.000	12.250
57	57 58 71 70	1	47.000	13.500
58	58 59 72 71	1	52.000	13.500
59	59 60 73 72	1	58.000	13.500
60	60 61 74 73	1	62.000	13.500
61	61 62 75 74	1	70.000	13.500
62	62 63 76 75	1	78.000	13.500
63	63 64 77 76	1	88.000	13.500
64	64 65 78 77	1	48.000	14.500
65	65 66 79 78	1	48.000	14.500
66	66 67 80 79	1	58.000	14.500
67	67 68 81 80	1	62.000	14.500
68	68 69 82 81	1	70.000	14.500
69	69 70 83 82	1	78.000	14.500
70	70 71 84 83	1	88.000	14.500
71	71 72 85 84	1	33.000	15.500
72	72 73 86 85	1	38.000	15.500
73	73 74 87 86	1	48.000	15.500
74	74 75 88 87	1	58.000	15.500
75	75 76 89 88	1	62.000	15.500
76	76 77 90 89	1	70.000	15.500
77	77 78 91 90	1	78.000	15.500
78	78 79 92 91	1	88.000	15.500

SSTIPN: 1 LAYER OF GEOTEXTILE, P=2400 PPI, E=6%, T=0

```
*****  
*****  
* LOAD CASE ----- 1 *  
*****  
*****
```

LARGEST ELE. NO IN THIS INCREMENT 78
LARGEST N P NO IN THIS INCREMENT 85

BAND WIDTH----- 40
TOTAL NUMBER OF EQUATIONS----- 222
NUMBER OF EQUATIONS IN BLOCK----- 85
NUMBER OF BLOCKS----- 3
NUMBER OF "N P" FORCE CARDS----- 3
NUMBER OF PRESSURE CARDS----- 0

MODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP	X-FORCE	Y-FORCE
1	0.	0
2	0	0
3	0	0
4	0	0
5	0	0
6	0	0
7	0	0
8	0	0
9	0	0
10	0	0
11	0	0
12	0	0
13	0	0
14	0	0
15	0	0
16	0	0
17	0	0
18	0	0
19	0	0
20	0	0
21	0	0
22	0	0
23	0	0
24	0	0
25	0	0
26	0	0
27	0	0
28	0	0
29	0	0
30	0	0
31	0	0
32	0	0
33	0	0
34	0	0
35	0	0
36	0	0
37	0	0
38	0	0
39	0	0
40	0	0
41	0	0
42	0	0
43	0	0
44	0	0
45	0	0
46	0	0
47	0	0
48	0	0
49	0	0
50	0	0
51	0	0
52	0	0
53	0	0
54	0	0
55	0	0
56	0	0
57	0	0
58	0	0
59	0	0
60	0	0
61	0	0
62	0	0
<hr/>		
63	0	0
64	0	0
65	0	0
66	0	0
67	0	0
68	0	0
69	0	0
70	0	0
71	0	0
72	0	0
73	0	0
74	0	0
75	0	0
76	0	0
77	0	0
78	0	0
79	0	0
80	0	0
81	0	0
82	0	0
83	0	0
84	0	0
85	0	0
86	0	0
87	0	0
88	0	0
89	0	0
90	0	0
91	0	0
92	0	-4.50
93	0	-4.50
94	0	-2.00
95	0	0

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	.0000	.0000	.0000	.0000	.0000	.0000	.0000	1
2	.0000	.0000	.0000	.0000	.0000	.0000	.0000	2
3	.0000	.0000	.0000	.0000	.0000	.0000	.0000	3
4	.0000	.0000	.0000	.0000	.0000	.0000	.0000	4
5	.0000	.0000	.0000	.0000	.0000	.0000	.0000	5
6	.0000	.0000	.0000	.0000	.0000	.0000	.0000	6
7	.0000	.0000	.0000	.0000	.0000	.0000	.0000	7
8	.0000	.0000	.0000	.0000	.0000	.0000	.0000	8
9	.0000	.0000	.0000	.0000	.0000	.0000	.0000	9
10	.0000	.0000	.0000	.0000	.0000	.0000	.0000	10
11	.0000	.0000	.0000	.0000	.0000	.0000	.0000	11
12	.0000	.0000	.0000	.0000	.0000	.0000	.0000	12
13	.0000	.0000	.0000	.0000	.0000	.0000	.0000	13
14	.0000	.0000	.0000	.0000	.0000	.0000	.0000	14
15	.0000	.0000	.0000	.0001	.0000	.0000	.0001	15
16	.0000	.0000	.0000	.0001	.0005	.0000	.0005	16
17	-.0024	.0021	.0020	-.0235	.0020	.0000	.0045	17
18	-.0177	.0031	.0000	-.1827	.0231	.0000	.1860	18
19	-.0343	-.0006	.0000	-.2587	-.0772	.0000	.2700	19
20	-.0350	.0088	.0000	-.1547	.1167	.0000	.1938	20
21	-.0238	-.0175	.0000	-.0641	-.1404	.0000	.1543	21
22	-.0008	-.0012	.0000	-.0003	-.0045	.0000	.0055	22
23	-.0007	.0014	.0000	-.0001	-.0047	.0000	.0047	23
24	.0008	-.0051	.0000	.0010	-.0103	.0000	.0103	24
25	.0020	-.0024	.0000	.0018	-.0057	.0000	.0080	25
26	.0000	-.0001	.0000	.0000	-.0031	.0000	.0031	26
27	.0000	.0000	.0000	.0000	.0000	.0000	.0000	27
28	.0000	.0000	.0000	.0001	-.0001	.0000	.0001	28
29	.0001	.0000	.0000	.0007	.0003	.0000	.0008	29
30	-.0148	.0027	.0000	-.1241	.0474	.0000	.1328	30
31	-.0285	.0173	.0000	-.2127	.0581	.0000	.2205	31
32	-.0420	.0072	.0000	-.3245	-.1303	.0000	.3128	32
33	-.0778	-.0080	.0000	-.2558	-.2258	.0000	.3418	33
34	-.0938	-.0184	.0000	-.2321	-.2300	.0000	.3267	34
35	-.0838	-.0451	.0000	-.1848	-.1782	.0000	.2428	35
36	.0000	-.0035	.0000	.0022	.0119	.0000	.0121	36
37	.0018	.0088	.0000	.0018	.0188	.0000	.0187	37
38	.0028	.0047	.0000	.0018	.0112	.0000	.0114	38
39	.0000	.0006	.0000	.0000	-.0053	.0000	.0053	39
40	.0000	.0000	.0000	.0000	.0000	.0000	.0000	40
41	.0000	.0000	.0000	.0001	-.0001	.0000	.0001	41
42	-.0178	.0050	.0000	-.0872	.0200	.0000	.0806	42
43	-.0335	.0059	.0000	-.1308	.0733	.0000	.1487	43
44	-.0330	.0220	.0000	-.1288	.0581	.0000	.1405	44
45	-.0340	.0077	.0000	-.1244	-.1772	.0000	.2165	45
46	-.0304	-.0084	.0000	-.1057	-.3028	.0000	.3207	46
47	-.0250	-.0209	.0000	-.0881	-.3071	.0000	.3188	47
48	-.0192	-.0879	.0000	-.0888	-.2857	.0000	.2941	48
49	-.0023	-.0707	.0000	-.0283	-.1388	.0000	.2007	49
50	.0017	-.0272	.0000	-.0051	-.0467	.0000	.0470	50
51	.0010	-.0054	.0000	-.0015	-.0139	.0000	.0140	51
52	.0000	.0015	.0000	.0000	-.0087	.0000	.0087	52
53	-.0318	.0222	.0000	-.1788	.0883	.0000	.1567	53
54	-.0350	.0079	.0000	-.1078	-.1087	.0000	.1538	54
55	-.0400	-.0088	.0000	-.0892	-.2398	.0000	.2586	55
56	-.0388	-.0382	.0000	-.0841	-.2826	.0000	.3073	56
57	-.0188	-.0707	.0000	-.1184	-.2140	.0000	.3348	57
58	-.0081	-.0723	.0000	-.0818	-.2277	.0000	.2335	58
59	-.0038	-.0284	.0000	-.0191	-.0857	.0000	.0876	59
60	-.0019	-.0057	.0000	-.0043	-.0513	.0000	.0515	60
61	.0000	.0018	.0000	.0000	-.0457	.0000	.0457	61
62	-.0482	.0082	.0000	-.0783	-.0280	.0000	.0838	62
63	-.0859	.0078	.0000	-.0887	-.1348	.0000	.1612	63
64	-.0811	.0387	.0000	-.0852	-.1842	.0000	.2073	64
65	-.0844	-.0784	.0000	-.0893	-.2198	.0000	.2412	65
66	-.0171	-.0737	.0000	-.0807	-.1612	.0000	.1880	66
67	-.0080	-.0280	.0000	-.0280	-.1225	.0000	.1251	67
68	-.0048	-.0080	.0000	-.0070	-.0127	.0000	.0145	68
69	.0000	.0018	.0000	.0000	-.0058	.0000	.0058	69
70	-.0504	.0014	.0000	-.0880	-.0456	.0000	.0818	70
71	-.0608	.0072	.0000	-.0717	-.0881	.0000	.1136	71
72	-.0520	-.0383	.0000	-.0759	-.1344	.0000	.1544	72
73	-.0585	-.0758	.0000	-.0832	-.1744	.0000	.1922	73
74	-.0885	-.2334	.0000	-.0878	-.2895	.0000	.3150	74
75	-.0118	-.0287	.0000	-.0274	-.1334	.0000	.1382	75
76	-.0073	-.0061	.0000	-.0082	-.0118	.0000	.0151	76
77	.0000	.0018	.0000	.0000	-.0045	.0000	.0045	77
78	.0488	.0072	.0000	.0573	.0488	.0000	.0781	78
79	.0441	.0385	.0000	.0589	.0883	.0000	.1050	79
80	-.0428	-.0752	.0000	-.0577	-.1284	.0000	.1408	80
81	-.0472	-.2344	.0000	-.0631	-.2739	.0000	.2811	81
82	.1852	-.2805	.0000	.2050	-.3768	.0000	.4290	82
83	.1832	-.0438	.0000	.2432	-.0310	.0000	.2449	83
84	.0000	.0888	.0000	.0000	.0880	.0000	.0880	84
85	-.0345	-.0389	.0000	-.0229	-.0546	.0000	.0871	85
86	-.0342	-.0749	.0000	-.0400	-.0851	.0000	.1032	86
87	-.0303	.1485	.0000	-.0367	-.1886	.0000	.1884	87
88	-.0237	-.5429	.0000	-.0280	-.5989	.0000	.5985	88
89	.5870	-.1060	.0000	.5552	-.1078	.0000	.5866	89
90	.0000	.2391	.0000	.0000	.2370	.0000	.2370	90
91	.0028	-.0748	.0000	.0025	-.0748	.0000	.0748	91
92	-.0012	-.3489	.0000	-.0012	-.3489	.0000	.3489	92
93	-.0017	-.5445	.0000	-.0017	-.5445	.0000	.5445	93
94	.2832	-.2339	.0000	.2832	-.2339	.0000	.2832	94
95	.0000	.1790	.0000	.0000	.1790	.0000	.1790	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS - INTERNAL MEMBER FORCES

ELEMENT NO	AXIAL FORCE
INCREMENTAL VALUES	
1	.0240
2	.0154
3	.2185
4	.6395
5	.8848
6	.0223
7	.2403
8	.0425
9	.0605
TOTAL VALUES	
1	.1415
2	.3509
3	1.1232
4	1.3912
5	2.3120
6	2.5730
7	1.3117

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

FILE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-2	GAMMAX	ELE
1	523.8	328.3	226.8	155	000	000	001	001	-001	001	1
2	520.8	330.4	224.2	151	000	008	005	007	-001	008	2
3	549.8	343.1	238.8	152	022	-019	045	032	-029	081	3
4	8	17.1	2	494	1.493	-389	2.128	1.972	-888	2.740	4
5	7	17.3	2	494	825	801	8.415	3.821	-2.595	8.415	5
6	7	18.0	2	494	860	2.424	5.415	4.000	-2.224	16.227	6
7	7	18.4	2	494	-1.123	3.214	3.031	3.990	-1.809	5.299	7
8	6	17.0	2	494	-797	1.825	-881	1.897	-889	2.788	8
9	483.4	351.8	198.8	218	-001	128	000	128	-001	128	9
10	478.4	370.0	183.8	238	-007	187	023	187	-008	185	10
11	479.1	373.2	183.0	241	-006	199	-066	204	-011	215	11
12	497.9	382.8	208.1	206	012	110	-040	114	008	108	12
13	431.4	253.4	192.8	120	000	001	-002	001	-001	002	13
14	481.3	284.7	201.7	119	004	003	011	007	-006	013	14
15	1.8	13.5	8	480	839	-780	1.751	1.080	-1.181	2.241	15
16	11.2	17.3	4.1	382	2.232	-1.240	2.183	2.541	-1.549	4.090	16
17	1.8	17.0	8	484	1.223	401	3.389	2.548	-922	3.468	17
18	8	17.2	2	494	-824	2.898	2.970	3.241	-1.387	4.807	18
19	8	17.1	2	494	-1.439	3.308	4.860	4.328	-2.480	6.785	19
20	7	18.2	2	494	-1.837	4.370	3.210	4.772	-2.039	6.811	20
21	8	17.1	2	494	-1.048	2.988	1.880	3.182	-1.209	4.371	21
22	472.2	373.3	189.3	247	-003	280	029	239	-004	265	22
23	407.1	305.8	185.8	239	-007	277	-082	239	-014	252	23
24	408.1	278.0	171.2	192	023	128	-051	135	017	117	24
25	409.0	229.7	187.2	092	-001	000	001	000	-001	001	25
26	0	13.4	1	485	290	-339	843	502	-580	1.082	26
27	4	13.4	1	485	983	-770	546	1.025	-812	1.837	27
28	2.1	13.8	7	474	724	-382	-1.236	998	-671	1.889	28
29	1.4	14.4	5	484	581	818	-552	983	384	609	29
30	5	15.1	2	484	281	2.071	-3.803	3.124	-1.344	4.476	30
31	6	15.8	2	484	-514	2.575	-4.789	3.879	-1.819	5.698	31
32	6	15.5	2	494	-1.080	3.079	-4.882	4.206	-2.208	6.414	32
33	7	17.3	2	494	-1.313	4.908	-2.881	5.184	-1.589	6.773	33
34	8	17.2	2	492	-133	3.583	-311	3.590	-139	3.729	34
35	231.0	213.7	83.0	281	-024	513	-085	518	-027	543	35
36	342.8	216.8	148.0	158	002	118	-009	118	002	116	36
37	1	4.0	0	495	-014	797	3.413	2.145	-1.382	3.508	37
38	1	4.0	0	495	-605	607	5.039	2.592	-2.590	5.183	38
39	40.4	403.2	13.8	482	-162	850	1.416	1.080	-572	1.832	39
40	1	4.0	0	495	-281	2.074	1.341	2.253	-440	2.693	40
41	1	4.0	0	495	051	3.972	2.441	4.321	-297	4.819	41
42	184.4	418.8	53.8	435	-854	3.870	1.774	4.038	-822	4.880	42
43	829.7	547.9	247.0	275	-324	2.936	-310	2.943	-332	3.275	43
44	1314.9	884.4	558.8	177	-111	2.718	184	2.721	-114	2.835	44
45	781.9	807.5	315.1	241	-035	2.590	037	2.580	-035	2.825	45
46	1	4.0	0	495	-058	-042	1.444	673	-775	1.444	46
47	11.9	398.8	4.0	495	058	-039	1.555	788	-789	1.558	47
48	11.9	398.8	4.0	485	019	035	2.216	1.135	-1.081	2.218	48
49	11.9	393.8	4.0	495	-274	268	3.226	1.633	-1.839	3.272	49
50	1	4.0	0	495	-421	337	1.888	844	-887	1.851	50
51	1088.0	883.7	475.8	152	-280	2.771	124	2.773	-282	3.034	51
52	1808.3	1028.5	885.1	085	-175	2.701	-398	2.714	-189	2.903	52
53	1384.1	741.2	853.3	059	-086	087	032	069	-068	137	53
54	228.7	425.8	81.4	404	106	-058	041	109	-057	166	54
55	805.8	464.0	196.2	289	111	-020	069	119	-022	148	55
56	522.8	486.8	203.9	282	118	-030	148	149	-081	210	56
57	1	4.0	0	495	-216	8.002	3.154	8.295	509	8.803	57
58	1	4.0	0	455	-587	10.877	1.874	10.755	-225	11.400	58
59	2088.1	1048.0	1048.0	000	-194	2.694	-874	2.732	-232	2.966	59
60	1187.0	822.4	344.4	083	-099	081	041	063	-102	185	60
61	284.9	433.0	103.2	381	063	-008	058	073	-018	090	61

62	194.7	421.5	88.6	419	020	006	194	110	-084	194	62
63	543.5	489.8	213.5	273	065	-028	-082	081	-044	125	63
64	304.5	438.6	110.9	373	114	034	155	181	-013	175	64
65	1	4.0	0	485	-2.075	15.884	-12.895	17.778	-4.164	21.947	65
66	1	4.0	0	485	-3.428	17.877	-51.215	34.821	-20.573	55.394	66
67	1	4.0	0	485	4.588	-2.315	-38.515	20.701	-18.428	39.129	67
68	67.6	406.3	23.0	472	-044	039	-178	056	-101	186	68
69	365.2	443.8	135.5	347	002	007	067	036	-028	067	69
70	1	4.0	0	495	010	5.884	1.423	5.781	-078	5.859	70
71	1	4.0	0	495	-1.708	19.609	13.021	21.440	-3.539	24.979	71
72	1	4.0	0	485	-6.876	18.850	16.646	19.469	-9.595	29.065	72
73	1	4.0	0	495	8.122	-4.508	8.437	8.895	-5.279	14.174	73
74	1	4.0	0	495	-007	-011	-2.723	1.353	-1.370	2.723	74
75	853.2	558.3	383.1	175	000	015	133	075	-080	134	75
76	573.2	429.1	418.1	183	-039	104	-115	124	-059	183	76
77	1	4.0	0	495	-5.284	6.526	8.912	8.019	-6.777	14.796	77
78	1	4.0	0	495	5.128	-554	8.707	7.484	-2.512	10.398	78

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THEYA	SIG1/SIG3	LEVEL	ELE
1	242	478	002	478	242	118	432	1.979	079	1
2	246	515	012	516	247	134	2.577	2.085	088	2
3	349	392	106	481	260	110	39.424	1.448	072	3
4	362	534	004	535	362	088	1.421	1.476	1.724	4
5	379	558	013	559	378	081	4.198	1.480	1.815	5
6	668	661	012	662	468	097	3.481	1.415	1.941	6
7	522	720	007	721	521	100	1.889	1.382	1.992	7
8	339	530	-002	530	339	095	-508	1.983	1.908	8
9	329	1.029	-003	1.029	329	350	-238	3.129	210	9
10	404	1.358	045	1.362	402	480	2.706	3.358	287	10
11	439	432	128	444	422	513	-7.232	3.429	280	11
12	375	954	-022	955	364	300	-7.946	2.849	174	12
13	138	273	-004	273	138	059	-11.598	2.018	052	13
14	152	261	022	265	143	059	11.036	1.795	044	14
15	078	188	007	188	078	040	5.013	2.024	803	15
16	285	279	024	306	258	024	48.456	1.188	484	16
17	341	422	010	423	340	042	7.281	1.245	833	17
18	372	475	006	475	372	052	3.442	1.279	1.037	18
19	388	476	010	477	387	055	5.322	1.300	1.101	19
20	520	635	008	636	519	058	3.844	1.225	1.168	20
21	384	481	003	482	384	054	1.827	1.279	1.074	21
22	407	1.525	056	1.528	404	052	2.836	3.779	312	22
23	304	1.201	-137	1.221	284	488	-8.481	4.305	296	23
24	292	723	-088	740	275	233	-11.053	2.699	149	24
25	042	090	001	090	042	024	1.600	2.157	021	25
26	000	000	000	000	000	000	83.371	6.842	1.000	26
27	000	000	000	000	000	000	588.767	488.178	599	27
28	070	035	-023	090	015	037	-56.834	5.991	746	28
29	154	237	-000	237	154	041	-315	1.537	829	29
30	205	303	-010	305	204	050	-5.867	1.493	1.006	30
31	281	344	-013	350	249	051	-7.596	1.406	1.010	31
32	241	338	-020	342	237	052	-11.454	1.443	1.049	32
33	516	623	-008	624	515	055	-4.907	1.213	1.094	33
34	485	590	001	590	485	047	-838	1.190	942	34
35	436	661	043	662	433	815	-2.010	3.435	332	35
36	117	387	-024	382	113	139	-7.085	3.460	1.109	36
37	041	087	-081	130	-001	085	-34.811	-111.500	-1.000	37
38	012	057	029	071	-002	036	28.943	-32.857	-1.000	38
39	076	308	175	400	-018	209	28.310	-22.745	-1.000	39
40	018	314	008	314	018	148	1.593	17.748	6.225	40
41	034	326	020	327	033	147	3.828	9.918	3.315	41
42	291	353	084	364	-302	333	7.327	-1.204	-1.000	42

43	-172	1.120	-812	1.384	-416	890	-21.718	-3.278	-1.000	43
44	115	1.112	140	1.137	096	521	7.784	11.815	4.020	44
45	019	447	060	455	011	222	7.850	43.231	15.898	45
46	-103	-084	074	-001	-156	078	38.858	004	-1.000	46
47	065	188	183	320	-086	193	35.736	-4.809	-1.000	47
48	144	422	181	485	070	212	24.843	7.033	2.243	48
49	021	101	051	101	-041	071	2.483	-2.483	-1.000	49
50	068	403	-087	424	047	188	-13.877	8.964	2.880	50
51	-736	899	-439	1.009	-846	928	-14.118	-1.193	-1.000	51
52	657	937	-004	937	-857	787	-131	-1.427	-1.000	52
53	530	383	076	386	536	483	4.736	7.226	1.000	53
54	400	103	072	417	088	165	77.069	4.831	1.424	54
55	814	151	171	886	109	373	76.380	7.817	2.534	55
56	896	085	332	1.012	-051	532	70.657	-19.749	-1.000	56
57	474	606	-116	678	405	135	-30.342	1.866	247	57
58	343	474	-084	500	317	082	-22.021	1.578	215	58
59	-1.159	1.017	289	1.080	-1.192	1.121	-6.937	-861	-1.000	59
60	-822	257	117	289	836	553	6.090	-322	-1.000	60
61	102	066	174	259	092	175	47.991	-2.512	-1.000	61
62	233	085	236	408	-088	247	53.716	-4.828	-1.000	62
63	518	-034	-187	588	-076	317	-75.190	-7.350	-1.000	63
64	770	635	108	830	575	128	80.988	1.444	185	64
65	552	544	-005	544	552	018	-9.130	1.053	022	65
66	577	810	-020	820	567	027	-25.180	1.084	038	66
67	197	291	-012	292	198	049	-7.415	1.500	186	67
68	015	006	-018	014	-023	019	-24.241	-630	-1.000	68
69	110	041	135	215	064	140	52.121	-3.371	1.000	69
70	332	349	040	381	299	041	-38.974	1.278	103	70
71	722	756	005	757	721	018	8.491	1.049	018	71
72	401	437	007	438	400	019	10.119	1.096	036	72
73	242	257	005	256	241	009	15.752	1.073	027	73
74	008	018	001	016	006	004	6.489	2.135	422	74
75	044	176	485	599	379	469	41.123	-1.578	-1.000	75
76	167	1033	481	1.202	336	769	-19.336	-3.578	-1.000	76
77	082	088	004	089	081	014	7.527	1.448	166	77
78	202	214	003	215	201	007	14.581	1.071	026	78

SSTIPN: 1 LAYER OF GEOTEXTILE, P=2400 PPI, E=5%, T=1000

TOTAL NUMBER OF NODES-----	88
NUMBER OF BAR ELEMENTS-----	9
NUMBER OF DIFF BAR MATERIALS-----	1
NUMBER OF BEAM ELEMENTS-----	0
NUMBER OF DIFF BEAM MATERIALS-----	0
NUMBER OF NODAL LINKS-----	0
NUMBER OF INTERFACE ELEMENTS-----	0
NO OF INTERFACE ELE IN PREEXIST PART-----	0
NUMBER OF INTERFACE ELE IN FOUNDATION-----	0
NUMBER OF INTERFACE MATERIALS-----	0
TOTAL NUMBER OF SOIL ELEMENTS-----	78
NUMBER OF DIFF SOIL MATERIALS-----	3
NUMBER OF ELEMENTS IN FOUNDATION-----	88
NUMBER OF NODES IN FOUNDATION-----	82
NUMBER OF PREEXISTING ELEMENTS-----	0
NUMBER OF PREEXISTING NODES-----	0
NUMBER OF CONSTRUCTION LAYERS-----	6
NUMBER OF LOAD CASES-----	2

CALING FACTOR -----	1.00000
ATMOSPHERIC PRESSURE ---	1.05800
UNIT WEIGHT OF WATER ---	0.03120

COMPUTATION SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO 1	APPLY LOAD CASE	1
INCREMENT NO 2	PUT ON LAYER NO	1
INCREMENT NO 3	PUT ON LAYER NO	2
INCREMENT NO 4	PUT ON LAYER NO	3
INCREMENT NO 5	PUT ON LAYER NO	4
INCREMENT NO 6	PUT ON LAYER NO	5
INCREMENT NO 7	PUT ON LAYER NO	6
INCREMENT NO 8	APPLY LOAD CASE	2

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		B.C CODE		
	X-GRD	Y-GRD	X	Y	ZZ
1	000	000	1	1	1
2	10 000	000	1	1	1
3	20 000	000	1	1	1
4	30 000	000	1	1	1
5	38 000	000	1	1	1
6	42 000	000	1	1	1
7	50 000	000	1	1	1
8	54 000	000	1	1	1
9	58 000	000	1	1	1
10	68 000	000	1	1	1
11	74 000	000	1	1	1
12	82 000	000	1	1	1
13	90 000	000	1	1	1
14	000	4 000	0	0	0
15	10 000	4 000	0	0	0
16	20 000	4 000	0	0	0
17	30 000	4 000	0	0	0
18	38 000	4 000	0	0	0
19	42 000	4 000	0	0	0
20	50 000	4 000	0	0	0
21	54 000	4 000	0	0	0
22	58 000	4 000	0	0	0
23	68 000	4 000	0	0	0
24	74 000	4 000	0	0	0
25	82 000	4 000	0	0	0
26	90 000	4 000	0	0	0
27	000	7 000	1	0	1
28	10 000	7 000	0	0	0
29	20 000	7 000	0	0	0
30	30 000	7 000	0	0	0
31	38 000	7 000	0	0	0
32	42 000	7 000	0	0	0
33	50 000	7 000	0	0	0
34	54 000	7 000	0	0	0
35	58 000	7 000	0	0	0
36	68 000	7 000	0	0	0
37	74 000	7 000	0	0	0
38	82 000	7 000	0	0	0
39	90 000	7 000	1	0	1
40	000	10 000	1	0	1
41	10 000	10 000	0	0	0
42	20 000	10 000	0	0	0
43	30 000	10 000	0	0	0
44	38 000	10 000	0	0	0
45	42 000	10 000	0	0	0
46	50 000	10 000	0	0	0
47	54 000	10 000	0	0	0
48	58 000	10 000	0	0	0
49	68 000	10 000	0	0	0
50	74 000	10 000	0	0	0
51	82 000	10 000	0	0	0
52	90 000	10 000	1	0	1
53	000	11 500	0	0	0
54	10 000	11 500	0	0	0
55	20 000	11 500	0	0	0
56	30 000	11 500	0	0	0
57	38 000	11 500	0	0	0
58	42 000	11 500	0	0	0
59	50 000	11 500	0	0	0
60	54 000	11 500	0	0	0
61	58 000	11 500	1	0	1

62	42.000	12 000	0	0	0
63	50 000	12 000	0	0	0
64	54 000	12 000	0	0	0
65	58 000	12 000	0	0	0
66	68 000	12 000	0	0	0
67	74 000	12 000	0	0	0
68	82 000	12 000	0	0	0
69	90 000	12 000	1	0	1
70	48 000	14 000	0	0	0
71	50 000	14 000	0	0	0
72	54 000	14 000	0	0	0
73	58 000	14 000	0	0	0
74	68 000	14 000	0	0	0
75	74 000	14 000	0	0	0
76	82 000	14 000	0	0	0
77	90 000	14 000	1	0	1
78	50 000	15 000	0	0	0
79	54 000	15 000	0	0	0
80	58 000	15 000	0	0	0
81	68 000	15 000	0	0	0
82	74 000	15 000	0	0	0
83	82 000	15 000	0	0	0
84	90 000	15 000	1	0	1
85	54 000	16 000	0	0	0
86	58 000	16 000	0	0	0
87	68 000	16 000	0	0	0
88	74 000	16 000	0	0	0
89	82 000	16 000	0	0	0
90	90 000	16 000	1	0	1
91	58 000	17 000	0	0	0
92	68 000	17 000	0	0	0
93	74 000	17 000	0	0	0
94	82 000	17 000	0	0	0
95	90 000	17 000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
1	480	1.00	0

ELMT NO	CONNECTED NODES I	J	MATL NO.
1	43	44	1
2	44	45	1
3	45	46	1
4	46	47	1
5	47	48	1
6	44	50	1
7	48	50	1
8	50	51	1
9	51	52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S MODULUS CONSTANT	EXPOONENT	RATIO	BULK MODULUS CONSTANT	EXPOONENT	STRENGTH C	PARAMETERS PHI	OPHI	K0
1	0880	8000 00	300	500	1500 00	.800	00	35 00	00	50
2	0530	40 00	300	800	20 00	200	00	00 00	00	50
3	0800	1000 00	400	700	500 00	500	50	40 00	00	50

FOUR NODES SOLID ELEMENT DATA

ELET NO	CONNECTED NODES I	J	K	L	MATL NO	ELEMENT CENTER COORDINATES X-ORD	Y-ORD
1	1	2	15	14	3	5.000	2.000
2	2	3	16	15	3	15.000	2.000
3	3	4	17	16	3	25.000	2.000
4	4	5	18	17	3	33.000	2.000
5	5	8	19	18	2	39.000	2.000
6	6	7	20	19	2	48.000	2.000
7	7	8	21	20	2	52.000	2.000
8	8	9	22	21	2	58.000	2.000
9	9	10	23	22	3	67.000	2.000
10	10	11	24	23	3	70.000	2.000
11	11	12	25	24	3	78.000	2.000
12	12	13	26	25	3	88.000	2.000
13	14	15	28	27	3	5.000	5.500
14	15	16	29	28	3	15.000	5.500
15	16	17	30	29	2	25.000	5.500
16	17	18	31	30	2	33.000	5.500
17	18	19	32	31	2	39.000	5.500
18	19	20	33	32	2	48.000	5.500
19	20	21	34	33	2	52.000	5.500
20	21	22	36	34	2	58.000	5.500
21	22	23	38	36	2	62.000	5.500
22	23	24	37	38	3	70.000	5.500
23	24	25	38	37	3	78.000	5.500
24	25	26	39	38	3	88.000	5.500
25	27	28	41	40	3	5.000	8.500
26	28	29	42	41	2	15.000	8.500
27	29	30	43	42	2	25.000	8.500
28	30	31	44	43	2	33.000	8.500
29	31	32	45	44	2	39.000	8.500
30	32	33	46	45	2	48.000	8.500
31	33	34	47	46	2	52.000	8.500
32	34	35	48	47	2	58.000	8.500
33	35	36	49	48	2	62.000	8.500
34	36	37	50	49	2	70.000	8.500
35	37	38	51	50	3	78.000	8.500
36	38	39	52	51	3	88.000	8.500
37	43	44	53	52	1	34.500	10.750
38	44	45	54	53	1	39.000	10.750
39	45	46	55	54	1	46.000	10.750
40	46	47	56	55	1	52.000	10.750
41	47	48	57	56	1	58.000	10.750
42	48	49	58	57	1	62.000	10.750
43	49	50	59	58	1	70.000	10.750
44	50	51	60	59	1	78.000	10.750
45	51	52	61	60	1	88.000	10.750
46	53	54	62	62	1	40.500	12.250
47	54	55	63	62	1	46.000	12.250
48	55	56	64	63	1	52.000	12.250
49	56	57	65	64	1	58.000	12.250
50	57	58	66	65	1	62.000	12.250
51	58	59	67	66	1	70.000	12.250
52	59	60	68	67	1	78.000	12.250
53	60	61	69	68	1	88.000	12.250
54	62	63	71	70	1	47.000	13.500
55	63	64	72	71	1	52.000	13.500
56	64	65	72	72	1	58.000	13.500
57	65	66	74	73	1	62.000	13.500
58	66	67	75	74	1	70.000	13.500
59	67	68	76	75	1	78.000	13.500
60	68	69	77	76	1	88.000	13.500
61	70	71	78	78	1	48.000	14.500
62	71	72	79	78	1	52.000	14.500
63	72	73	80	79	1	58.000	14.500
64	73	74	81	80	1	62.000	14.500
65	74	75	82	81	1	70.000	14.500
66	75	76	83	82	1	78.000	14.500
67	76	77	84	83	1	88.000	14.500
68	78	79	85	85	1	53.000	15.500
69	79	80	86	85	1	58.000	15.500
70	80	81	87	86	1	62.000	15.500
71	81	82	88	87	1	70.000	15.500
72	82	83	89	88	1	78.000	15.500
73	83	84	90	89	1	88.000	15.500
74	85	86	91	91	1	57.000	16.500
75	86	87	92	91	1	62.000	16.500
76	87	88	92	92	1	70.000	16.500
77	88	89	94	93	1	78.000	16.500
78	89	90	95	94	1	88.000	16.500

CONSTRUCTION LAYER INFORMATION

MODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP X-FORCE Y-FORCE

NP	X-FORCE	Y-FORCE
1	0.	0
2	0	0
3	0	0
4	0	0
5	0	0
6	0	0
7	0	0
8	0	0
9	0	0
10	0	0
11	0	0
12	0	0
13	0	0
14	0	0
15	0	0
16	0	0
17	0	0
18	0	0
19	0	0
20	0	0
21	0	0
22	0	0
23	0	0
24	0	0
25	0	0
26	0	0
27	0	0
28	0	0
29	0	0
30	0	0
31	0	0
32	0	0
33	0	0
34	0	0
35	0	0
36	0	0
37	0	0
38	0	0
39	0	0
40	0	0
41	0	0
42	0	0
43	0	0
44	0	0
45	0	0
46	0	0
47	0	0
48	0	0
49	0	0
50	0	0
51	0	0
52	0	0
53	0	0
54	0	0
55	0	0
56	0	0
57	0	0
58	0	0
59	0	0
60	0	0
61	0	0
62	0	0
63	0	0
64	0	0
65	0	0
66	0	0
67	0	0
68	0	0
69	0	0
70	0	0
71	0	0
72	0	0
73	0	0
74	0	0
75	0	0
76	0	0
77	0	0
78	0	0
79	0	0
80	0	0
81	0	0
82	0	0
83	0	0
84	0	0
85	0	0
86	0	0
87	0	0
88	0	0
89	0	0
90	0	0
91	0	0
92	0	0
93	-4	50
94	-1	50
95	-2	00
96	0	0

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROSTAT	TOTAL	NP
1	0000	0000	0000	0000	0000	0000	0000	1
2	0000	0000	0000	0000	0000	0000	0000	2
3	0000	0000	0000	0000	0000	0000	0000	3
4	0000	0000	0000	0000	0000	0000	0000	4
5	0000	0000	0000	0000	0000	0000	0000	5
6	0000	0000	0000	0000	0000	0000	0000	6
7	0000	0000	0000	0000	0000	0000	0000	7
8	0000	0000	0000	0000	0000	0000	0000	8
9	0000	0000	0000	0000	0000	0000	0000	9
10	0000	0000	0000	0000	0000	0000	0000	10
11	0000	0000	0000	0000	0000	0000	0000	11
12	0000	0000	0000	0000	0000	0000	0000	12
13	0000	0000	0000	0000	0000	0000	0000	13
14	0000	0000	0000	0000	0000	0000	0000	14
15	0000	0000	0000	0001	0000	0000	0001	15
16	0000	0000	0000	0002	0000	0000	0002	16
17	0003	0001	0000	0045	0023	0000	0051	17
18	0152	0058	0000	1893	0323	0000	1922	18
19	0390	0054	0000	2712	0747	0000	2812	19
20	0857	0081	0000	1850	1183	0000	2037	20
21	0287	0223	0000	0885	1483	0000	1620	21
22	0011	0015	0000	0005	0058	0000	0066	22
23	0007	0014	0000	0002	0048	0000	0048	23
24	0007	0013	0000	0003	0103	0000	0103	24
25	0020	0023	0000	0018	0076	0000	0093	25
26	0000	0001	0000	0000	0030	0000	0030	26
27	0000	0000	0000	0000	0001	0000	0001	27
28	0000	0000	0000	0000	0001	0000	0001	28
29	0001	0000	0000	0010	0003	0000	0010	29
30	0116	0023	0000	1860	0576	0000	1757	30
31	0214	0184	0000	2501	0579	0000	2567	31
32	0441	0175	0000	2848	1296	0000	3129	32
33	0883	0100	0000	2815	2277	0000	3487	33
34	1027	0200	0000	2378	2406	0000	3383	34
35	0882	0512	0000	1863	1439	0000	2479	35
36	0002	0034	0000	0018	0123	0000	0124	36
37	0017	0102	0000	0011	0188	0000	0188	37
38	0026	0046	0000	0014	0111	0000	0112	38
39	0000	0012	0000	0000	0051	0000	0051	39
40	0000	0000	0000	0000	0001	0000	0001	40
41	0000	0000	0000	0001	0001	0000	0002	41
42	0116	0038	0000	0862	0238	0000	0704	42
43	0240	0051	0000	1818	0853	0000	1878	43
44	0541	0211	0000	1558	0422	0000	1618	44
45	0238	0246	0000	1480	1788	0000	2309	45
46	0222	0085	0000	1225	3047	0000	3285	46
47	0207	0321	0000	1065	3195	0000	3368	47
48	0189	0787	0000	0880	2955	0000	3093	48
49	0038	0459	0000	0404	2045	0000	2044	49
50	0007	0274	0000	0124	0448	0000	0464	50
51	0003	0050	0000	0051	0150	0000	0158	51
52	0000	0018	0000	0000	0096	0000	0096	52
53	0842	0214	0000	2419	0887	0000	2577	53
54	1118	0292	0000	1878	0942	0000	2099	54
55	0989	0258	0000	1808	2581	0000	3041	55
56	0787	0547	0000	1328	3110	0000	3382	56
57	0251	0818	0000	1234	3202	0000	3432	57
58	0081	0714	0000	0837	2235	0000	2299	58
59	0048	0291	0000	0293	0883	0000	0887	59
60	0027	0058	0000	0051	0513	0000	0518	60
61	0000	0022	0000	0000	0453	0000	0453	61
62	1024	0286	0000	1351	0131	0000	1357	62
63	1027	0277	0000	1375	1525	0000	2053	63
64	1043	0815	0000	1394	2056	0000	2484	64
65	1036	0941	0000	1392	2342	0000	2725	65
66	0177	0727	0000	0516	1583	0000	1885	66
67	0103	0287	0000	0285	1231	0000	1259	67
68	0081	0080	0000	0082	0128	0000	0153	68
69	0000	0022	0000	0000	0055	0000	0055	69
70	0984	0004	0000	1155	0474	0000	1248	70
71	0889	0275	0000	1178	1072	0000	1550	71
72	0984	0813	0000	1187	1575	0000	1878	72
73	0988	0940	0000	1260	1908	0000	2287	73
74	1112	2364	0000	1387	3068	0000	3342	74
75	0133	0304	0000	0310	1334	0000	1373	75
76	0087	0060	0000	0109	0120	0000	0182	76
77	0000	0020	0000	0000	0043	0000	0043	77
78	0887	0279	0000	0993	0880	0000	1203	78
79	0884	0814	0000	1013	1119	0000	1508	79
80	0870	0833	0000	1018	1454	0000	1775	80
81	0904	2396	0000	1082	2788	0000	2981	81
82	1888	2826	0000	1775	3536	0000	3958	82
83	2285	0472	0000	2842	0725	0000	4980	83
84	0000	0887	0000	0000	0872	0000	0872	84
85	0789	0815	0000	0844	0791	0000	1157	85
86	0821	0521	0000	0861	1132	0000	1422	86
87	0787	3575	0000	0431	3745	0000	3644	87
88	0890	5440	0000	0731	8001	0000	8045	88
89	8303	1087	0000	5291	1114	0000	5407	89
90	0000	2367	0000	0000	2336	0000	2336	90
91	0448	0330	0000	0448	0330	0000	1032	91
92	0482	3579	0000	0482	3579	0000	3812	92
93	0482	5459	0000	0482	5459	0000	5460	93
94	2342	2409	0000	2342	2409	0000	3360	94
95	0000	3721	0000	0000	3721	0000	3721	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO	AXIAL FORCE
INCREMENTAL VALUES	
1	0120
2	0227
3	0149
4	1758
5	2207
6	4884
7	2757
8	0208
9	0190
TOTAL VALUES	
1	4884
2	7819
3	14108
4	18235
5	21136
6	28570
7	18836

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELC	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	524.8	325.2	227.1	185	000	000	001	001	-001	002	1
2	521.2	331.8	224.3	182	000	000	007	008	-001	010	2
3	542.0	338.2	235.2	182	022	-021	046	031	-030	062	3
4	.6	17.1	2	484	1.540	-444	2.184	2.018	-0.920	2.936	4
5	.7	17.3	2	484	.882	.517	6.885	3.828	-2.729	6.857	5
6	.7	18.0	2	484	.663	2.428	5.732	4.138	-2.394	6.311	6
7	.7	18.4	2	482	-1.194	3.321	3.289	3.850	-1.724	5.574	7
8	.6	17.0	2	484	.882	1.901	-882	1.970	-832	2.901	8
9	487.2	355.0	198.9	218	-001	171	002	131	-001	133	9
10	480.4	370.8	194.0	238	-007	188	027	188	-008	187	10
11	478.2	371.3	192.8	240	-008	188	-082	203	-010	213	11
12	498.1	382.0	205.2	208	011	107	-038	111	007	103	12
13	432.7	254.8	183.5	120	000	001	-002	008	-009	002	13
14	486.2	287.6	203.4	118	006	003	014	008	-009	017	14
15	1.9	13.8	8	478	.847	-828	2.400	1.482	-1.823	2.285	15
16	0.8	18.8	3.1	407	2.241	-1.333	3.444	2.838	-2.027	4.863	16
17	1.0	18.7	3	490	.971	.506	3.703	2.805	-1.128	3.732	17
18	.8	17.2	2	484	-809	2.722	2.728	3.188	-1.278	4.482	18
19	.8	17.1	2	484	-1.480	3.378	4.908	4.401	-2.512	6.314	19
20	.7	18.2	2	484	-1.756	4.540	3.102	4.902	-2.117	7.019	20
21	.8	17.2	2	484	-1.052	3.095	1.851	3.253	-1.210	4.463	21
22	484.8	370.2	186.0	248	-002	267	017	264	-004	272	22
23	400.3	299.9	182.9	228	-008	234	-075	240	-014	253	23
24	408.6	277.0	188.9	193	-020	127	-048	132	-015	118	24
25	409.0	228.7	187.2	093	-001	000	001	000	-001	001	25
26	0	1	0	485	.337	-401	388	578	-841	1.217	26
27	4	13.4	1	485	1.303	-1.029	373	1.218	-1.044	2.382	27
28	2.3	13.8	.8	472	.850	-1.387	-1.189	928	-845	1.573	28
29	7.8	15.4	2.8	410	208	1.083	-881	1.144	1.88	988	29
30	.5	15.2	2	484	.293	2.106	-2.250	2.818	-1.105	4.023	30
31	.8	18.8	2	484	-487	2.598	-4.159	3.843	-1.541	5.184	31
32	.6	15.5	2	484	-1.124	3.181	-4.488	4.148	-2.030	6.228	32
33	.7	17.9	2	484	-1.348	5.078	-2.248	5.270	-1.539	6.809	33
34	.9	17.2	3	481	-171	3.438	-030	3.857	-171	3.807	34
35	228.4	219.4	82.9	288	-047	497	088	501	-052	553	35
36	343.1	216.7	148.1	188	-023	139	037	141	-025	186	36
37	1	4.0	0	485	-022	823	8.217	4.531	-3.779	8.260	37
38	1	4.0	0	485	-487	467	0.158	5.052	-5.111	10.203	38
39	1	4.0	0	485	-288	1.040	6.883	3.879	-3.107	6.987	39
40	1	4.0	0	485	-504	2.838	5.103	4.217	-1.883	6.100	40
41	1	4.0	0	485	-288	4.209	3.787	4.901	-890	5.391	41
42	200.8	422.7	70.8	478	-875	3.833	1.816	4.067	-849	4.515	42
43	588.9	504.8	230.8	271	-338	2.959	-283	2.888	-344	3.310	43
44	721.8	642.1	281.8	281	-118	2.742	173	2.745	-121	2.868	44
45	681.0	588.0	258.1	271	-044	2.585	038	2.588	-044	2.840	45
46	1	4.0	0	485	-164	084	-227	258	-010	248	46
47	11.2	398.8	4.0	485	-087	124	812	483	-456	938	47
48	11.9	398.6	4.0	485	-321	342	2.097	1.110	-1.090	2.199	48
49	11.9	398.6	4.0	485	-720	704	4.378	2.294	-2.310	4.804	49
50	1	4.0	0	485	-884	504	2.618	1.388	-1.516	2.903	50
51	1231.7	733.0	548.2	127	-259	2.788	186	2.770	-281	3.031	51
52	888.4	1018.8	884.3	088	-178	2.703	-373	2.715	-191	2.908	52
53	894.3	845.1	380.7	175	-078	075	044	078	-082	160	53
54	151.2	418.2	52.6	427	024	001	086	048	-023	069	54
55	528.7	487.2	205.6	280	024	012	048	048	-008	051	55
56	517.2	485.3	201.4	284	-054	-002	142	107	-050	153	56
57	1	4.0	0	485	-483	8.321	5.182	0.025	-1.187	10.193	57
58	1	4.0	0	485	-803	10.989	4.023	11.303	-1.137	12.441	58
59	2080.3	1040.2	1040.2	000	-209	2.687	-520	2.713	-235	2.948	59
60	828.7	518.4	353.2	154	-117	071	081	078	-121	187	60
61	842.9	521.0	387.2	148	021	-009	174	084	-022	179	61

62	114.9	411.8	39.5	482	026	002	183	112	-043	195	62
63	538.9	488.8	210.0	276	052	-027	-108	079	-055	133	63
64	310.7	438.4	113.4	370	102	045	198	158	-010	188	64
65	1	4.0	0	485	-2.410	14.511	-11.812	18.288	-4.288	20	65
66	1	4.0	0	485	-3.308	17.588	-47.347	33.003	-18.742	51	66
67	1	4.0	0	485	4.274	-285	-36.151	20.208	-18.230	36	67
68	114.8	411.8	39.5	482	018	008	033	030	-008	035	68
69	353.3	442.2	130.8	352	007	-007	044	025	-025	050	69
70	1	4.0	0	405	005	5.881	1.533	5.888	-083	6	70
71	1	4.0	0	485	-1.809	19.756	13.263	21.818	-3.469	25	71
72	1	4.0	0	485	7.193	18.952	12.018	19.845	-10.166	30	72
73	1	4.0	0	485	7.850	-4.241	6.368	8.791	-5.172	13.983	73
74	1	4.0	0	485	000	-013	-2.743	1.389	-1.373	2.743	74
75	884.1	567.1	379.2	188	-002	018	120	088	-053	121	75
76	884.2	568.6	383.8	185	-048	115	-124	138	-069	205	76
77	1	4.0	0	485	-5.510	6.701	9.135	8.220	-7.029	15	77
78	1	4.0	0	485	4.778	-215	8.819	7.348	-2.788	10.135	78

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THETA	SIG1/SIG3	LEVEL	ELE
1	.242	.480	.003	.480	.242	.118	.757	1.874	.078	1
2	.290	.521	.016	.522	.249	.136	3.420	2.093	.080	2
3	.344	.582	.027	.471	.284	.108	40.030	1.854	.071	3
4	.161	.322	.004	.533	.380	.086	1.446	1.477	1.720	4
5	.374	.553	.014	.584	.373	.080	4.245	1.185	1.810	5
6	.488	.659	.012	.880	.466	.097	3.881	1.417	1.943	6
7	.530	.723	.007	.729	.523	.100	2.140	1.378	2.000	7
8	.340	.532	.002	.532	.340	.088	1.507	1.562	1.914	8
9	.338	1.057	.003	1.057	.335	.361	3.264	3.160	2.16	9
10	.409	1.370	.053	1.373	.408	.483	3.183	3.381	2.88	10
11	.433	1.421	.120	1.438	.419	.508	-6.828	3.428	2.78	11
12	.388	.922	-.078	.943	.389	.284	-7.744	2.855	1.72	12
13	.137	.275	-.003	.275	.137	.088	-1.402	2.002	.052	13
14	.182	.289	.028	.289	.181	.057	14.588	1.781	.082	14
15	.084	.172	.010	.182	.088	.039	7.404	1.914	.778	15
16	.289	.274	.020	.288	.246	.021	34.445	1.172	.423	16
17	.318	.401	.008	.402	.314	.044	5.151	1.279	.676	17
18	.379	.482	.006	.482	.378	.052	3.236	1.274	1.038	18
19	.371	.480	.010	.481	.370	.055	5.361	1.299	1.07	19
20	.528	.648	.008	.648	.528	.059	3.878	1.224	1.182	20
21	.400	.508	.003	.508	.400	.054	1.844	1.270	1.079	21
22	.420	.540	.070	.544	.416	.084	3.581	3.711	3.10	22
23	.287	1.184	-.122	1.208	.280	.462	-7.881	4.287	2.85	23
24	.274	.708	-.083	.722	.259	.231	-10.521	2.790	1.51	24
25	.042	.080	.001	.080	.042	.024	1.810	2.141	.021	25
26	.004	.001	.003	.008	.002	.004	84.087	-2.888	1.000	26
27	.078	.001	.013	.079	.001	.020	80.288	-34.181	1.000	27
28	.087	.081	-.038	.082	.017	.038	-47.221	5.525	.754	28
29	.172	.183	-.023	.208	.187	.028	-32.103	1.322	.508	29
30	.208	.314	-.008	.314	.208	.053	-8.858	1.510	1.081	30
31	.281	.354	-.014	.358	.288	.048	-8.433	1.378	.973	31
32	.282	.353	-.018	.358	.248	.054	-10.039	1.432	1.074	32
33	.538	.840	-.012	.841	.535	.053	-6.568	1.189	1.083	33
34	.488	.585	.018	.588	.488	.048	9.861	1.188	.821	34
35	.419	.586	.021	.588	.419	.024	1.971	3.979	3.42	35
36	.081	.371	-.024	.373	.088	.142	-4.818	4.202	1.15	36
37	.081	.085	-.064	.140	.005	.088	-38.538	29.332	10.531	37
38	.006	.082	.033	.089	-.011	.040	27.779	-6.074	-1.000	38
39	.087	.280	.088	.324	.024	.150	19.882	13.780	4.723	39
40	.080	.338	.018	.340	.048	.145	3.572	8.940	2.208	40
41	.027	.321	.018	.322	.028	.148	3.071	12.211	4.187	41
42	.419	.582	.141	.587	-.443	.418	3.911	-8.773	-1.000	42

43	-.148	1.171	-.587	1.374	-.352	.883	-20.083	-3.900	-1.000	43
44	.202	1.058	.134	1.079	.182	.448	8.705	5.334	1.834	44
45	-.024	.453	.057	.480	-.020	.250	8.538	-11.484	-1.000	45
46	-.080	-.032	.084	.031	-.143	.087	38.949	-218	-1.000	46
47	.088	-.231	.154	.329	-.012	.170	32.428	-27.582	-1.000	47
48	-.020	.320	.148	.378	-.078	.228	20.598	-4.983	-1.000	48
49	-.125	.098	.040	.108	-.132	.119	9.838	-4.807	-1.000	49
50	.080	.398	-.078	.414	.048	.185	-12.185	9.429	3.132	50
51	-.780	.822	-.088	1.322	-.880	.891	-13.824	-1.180	-1.000	51
52	-.731	.948	.047	.950	-.732	.841	1.801	-1.288	-1.000	52
53	-.448	.553	.108	.587	-.414	.414	7.603	-7.787	-1.000	53
54	.171	.089	.088	.238	.024	.106	58.325	9.709	3.237	54
55	.364	.144	.134	.419	.079	.170	84.127	5.321	1.808	55
56	.850	.049	.318	.713	-.074	.383	82.939	-9.587	-1.000	56
57	.487	.608	-.118	.674	.401	.138	-29.280	1.678	.252	57
58	.342	.478	-.061	.501	.319	.081	-20.927	1.572	.212	58
59	-.188	1.030	-.233	1.054	-.192	1.123	-8.978	-8.884	-1.000	59
60	.730	.240	.117	.254	-.744	.488	8.778	-3.41	-1.000	60
61	.027	.083	-.008	.084	.028	.028	18.700	3.230	.829	61
62	.283	.075	.177	.370	-.032	.201	59.014	-11.825	-1.000	62
63	.418	.052	-.207	.486	-.131	.313	-89.280	-3.801	-1.000	63
64	.738	.886	.114	.822	.582	.120	83.821	1.412	1.853	64
65	.840	.577	.030	.577	.840	.018	3.38	1.088	.025	65
66	.877	.811	-.018	.819	.888	.025	-24.188	1.089	.033	66
67	.200	.275	-.019	.276	.187	.040	-10.408	1.411	.183	67
68	.127	.103	.032	.148	.080	.035	58.279	1.861	.320	68
69	.085	.022	.105	.188	.084	.122	58.215	1.488	1.000	69
70	.338	.357	-.040	.388	.308	.041	-37.137	1.270	.100	70
71	.724	.754	.006	.759	.723	.018	9.011	1.050	.018	71
72	.387	.433	.007	.435	.386	.019	10.812	1.088	.037	72
73	.233	.258	.054	.288	-.232	.013	9.108	1.112	.042	73
74	.008	.017	.001	.017	.008	.004	8.430	2.055	.292	74
75	.030	.184	.483	.587	-.353	.480	40.179	-1.805	-1.000	75
76	.177	.027	.450	1.178	-.327	.752	-18.375	-3.800	-1.000	76
77	.080	.080	.004	.087	.089	.014	7.619	1.468	1.174	77
78	.201	.214	.004	.215	.200	.007	14.188	1.074	.028	78

SSTIPN 1 LAYER OF GEOTEXTILE, P=4000 PPI, E=5%, EXTENDED, T=1000

TOTAL NUMBER OF NODES	85
NUMBER OF SAC ELEMENTS	11
NUMBER OF DIFF BAR MATERIALS	1
NUMBER OF BEAM ELEMENTS	0
NUMBER OF DIFF BEAM MATERIALS	0
NUMBER OF NODAL LINKS	0
NUMBER OF INTERFACE ELEMENTS	0
NO OF INTERFACE ELE IN PREEXIST PART	0
NUMBER OF INTERFACE ELE IN FOUNDATION	0
NUMBER OF INTERFACE MATERIALS	0
TOTAL NUMBER OF SOIL ELEMENTS	78
NUMBER OF DIFF SOIL MATERIALS	3
NUMBER OF ELEMENTS IN FOUNDATION	36
NUMBER OF NODES IN FOUNDATION	52
NUMBER OF PREEXISTING ELEMENTS	0
NUMBER OF PREEXISTING NODES	0
NUMBER OF CONSTRUCTION LAYERS	6
NUMBER OF LOAD CASES	2

CALLING FACTOR	1.00000
ATMOSPHERIC PRESSURE	1.05300
UNIT WEIGHT OF WATER	0.3120

CONSTRUCTION SEQUENCE FOR A TOTAL OF 8 INCREMENTS

INCREMENT NO	1	APPLY LOAD CASE	1
INCREMENT NO	2	PUT ON LAYER NO	1
INCREMENT NO	3	PUT ON LAYER NO	2
INCREMENT NO	4	PUT ON LAYER NO	3
INCREMENT NO	5	PUT ON LAYER NO	4
INCREMENT NO	6	PUT ON LAYER NO	5
INCREMENT NO	7	PUT ON LAYER NO	6
INCREMENT NO	8	APPLY LOAD CASE	2

NODAL POINT INPUT DATA

NODE NUMBER	NODAL POINT COORDINATES		B C CGSE		
	X-ORD	Y-ORD	X	Y	Z
1	000	000	1	1	1
2	10 000	000	1	1	1
3	20 000	000	1	1	1
4	30 000	000	1	1	1
5	38 000	000	1	1	1
6	42 000	000	1	1	1
7	50 000	000	1	1	1
8	54 000	000	1	1	1
9	58 000	000	1	1	1
10	68 000	000	1	1	1
11	74 000	000	1	1	1
12	82 000	000	1	1	1
13	90 000	000	1	1	1
14	000	4 000	0	0	0
15	10 000	4 000	0	0	0
16	20 000	4 000	0	0	0
17	30 000	4 000	0	0	0
18	38 000	4 000	0	0	0
19	42 000	4 000	0	0	0
20	50 000	4 000	0	0	0
21	54 000	4 000	0	0	0
22	58 000	4 000	0	0	0
23	68 000	4 000	0	0	0
24	74 000	4 000	0	0	0
25	82 000	4 000	0	0	0
26	90 000	4 000	0	0	0
27	000	7 000	1	0	1
28	10 000	7 000	0	0	0
29	20 000	7 000	0	0	0
30	30 000	7 000	0	0	0
31	38 000	7 000	0	0	0
32	42 000	7 000	0	0	0
33	50 000	7 000	0	0	0
34	54 000	7 000	0	0	0
35	58 000	7 000	0	0	0
36	68 000	7 000	0	0	0
37	74 000	7 000	0	0	0
38	82 000	7 000	0	0	0
39	90 000	7 000	1	0	1
40	000	10 000	1	0	1
41	10 000	10 000	0	0	0
42	20 000	10 000	0	0	0
43	30 000	10 000	0	0	0
44	38 000	10 000	0	0	0
45	42 000	10 000	0	0	0
46	50 000	10 000	0	0	0
47	54 000	10 000	0	0	0
48	58 000	10 000	0	0	0
49	68 000	10 000	0	0	0
50	74 000	10 000	0	0	0
51	82 000	10 000	0	0	0
52	90 000	10 000	1	0	1
53	000	11 500	0	0	0
54	10 000	11 500	0	0	0
55	20 000	11 500	0	0	0
56	30 000	11 500	0	0	0
57	38 000	11 500	0	0	0
58	42 000	11 500	0	0	0
59	50 000	11 500	0	0	0
60	54 000	11 500	0	0	0
61	58 000	11 500	1	0	1

62	42 000	13 000	0	0	0
63	50 000	13 000	0	0	0
64	54 000	13 000	0	0	0
65	58 000	13 000	0	0	0
66	68 000	13 000	0	0	0
67	74 000	13 000	0	0	0
68	82 000	13 000	1	0	1
69	90 000	13 000	0	0	0
70	000	14 000	0	0	0
71	10 000	14 000	0	0	0
72	20 000	14 000	0	0	0
73	30 000	14 000	0	0	0
74	38 000	14 000	0	0	0
75	42 000	14 000	0	0	0
76	50 000	14 000	0	0	0
77	54 000	14 000	1	0	1
78	58 000	15 000	0	0	0
79	68 000	15 000	0	0	0
80	74 000	15 000	0	0	0
81	82 000	15 000	0	0	0
82	90 000	15 000	0	0	0
83	000	16 000	1	0	1
84	10 000	16 000	0	0	0
85	20 000	16 000	0	0	0
86	30 000	16 000	0	0	0
87	38 000	16 000	0	0	0
88	42 000	16 000	0	0	0
89	50 000	16 000	1	0	1
90	54 000	17 000	0	0	0
91	58 000	17 000	0	0	0
92	68 000	17 000	0	0	0
93	74 000	17 000	1	0	1

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS-----

MATERIAL NUMBER	E	AREA	WEIGHT/LENGTH
	480	1.00	0.1

ELMT NO	CONNECTED NODES I J	MATL NO.
1	21 42	1
2	42 43	1
3	43 44	1
4	44 45	1
5	45 46	1
6	46 47	1
7	47 48	1
8	48 49	1
9	49 50	1
10	50 51	1
11	51 52	1

SOIL MATERIAL PROPERTY DATA

MATL	UNIT WT	YOUNG'S CONSTANT	MODULUS EXPONENT	RATIO	BULK MODULUS CONSTANT	MODULUS EXPONENT	STRENGTH C	PARAMETERS PHI	DPHI	KD
1	0.80	8000 00	500	500	1500 00	800	00	35.00	00	50
2	.0530	40 00	300	800	20 00	200	05	00	00	50
3	0.800	1000 00	400	700	500.00	500	.50	40.00	00	50

FOUR NODES SOLID ELEMENT DATA

ELMT NO	CONNECTED NODES I J K L	MATL NO.	ELEMENT CENTER COORDINATES X-ORD Y-ORD
1	1 2 15 14	3	5 000 2 000
2	2 3 16 15	3	15 000 2 000
3	3 4 17 18	3	25 000 2 000
4	4 5 18 17	2	33 000 2 000
5	5 6 19 18	2	39 000 2 000
6	6 7 20 18	2	45 000 2 000
7	7 8 21 20	2	52 000 2 000
8	8 9 22 21	2	58 000 2 000
9	9 10 23 22	3	62 000 2 000
10	10 11 24 23	3	70 000 2 000
11	11 12 25 24	3	78 000 2 000
12	12 13 26 25	3	86 000 2 000
13	13 14 27 26	3	5 000 5 500
14	15 16 28 27	3	18 000 5 500
15	16 17 30 28	2	25 000 5 500
16	17 18 31 30	2	33 000 5 500
17	18 19 32 31	2	39 000 5 500
18	19 20 33 32	2	46 000 5 500
19	20 21 34 33	2	52 000 5 500
20	21 22 35 34	2	58 000 5 500
21	22 23 36 35	2	62 000 5 500
22	23 24 37 36	3	70 000 5 500
23	24 25 38 37	3	78 000 5 500
24	26 28 39 38	3	86 000 5 500
25	27 28 41 40	3	5 000 8 500
26	28 29 42 41	2	15 000 8 500
27	29 30 43 42	2	25 000 8 500
28	30 31 44 43	2	33 000 8 500
29	31 32 45 44	2	39 000 8 500
30	32 33 46 45	2	45 000 8 500
31	33 34 47 46	2	52 000 8 500
32	34 35 48 47	2	58 000 8 500
33	35 36 49 48	2	62 000 8 500
34	36 37 50 48	2	70 000 8 500
35	37 38 51 50	3	78 000 8 500
36	38 39 52 51	3	86 000 8 500
37	43 44 53 53	1	34 500 10 750
38	44 45 54 53	1	39 000 10 750
39	45 46 55 54	1	45 000 10 750
40	46 47 56 55	1	52 000 10 750
41	47 48 57 56	1	58 000 10 750
42	48 49 58 57	1	62 000 10 750
43	49 50 59 58	1	70 000 10 750
44	50 51 60 59	1	78 000 10 750
45	51 52 61 60	1	86 000 10 750
46	53 54 62 62	1	40 500 12 250
47	54 55 63 62	1	46 000 12 250
48	55 56 64 63	1	52 000 12 250
49	56 57 65 64	1	58 000 12 250
50	57 58 66 65	1	62 000 12 250
51	58 59 67 66	1	70 000 12 250
52	59 60 68 67	1	78 000 12 250
53	60 61 69 68	1	86 000 12 250
54	62 63 71 70	1	47 000 13 500
55	63 64 72 71	1	52 000 13 500
56	64 65 73 72	1	58 000 13 500
57	65 66 74 73	1	62 000 13 500
58	66 67 75 74	1	70 000 13 500
59	67 68 76 75	1	78 000 13 500
60	68 69 77 76	1	86 000 13 500
61	70 71 78 78	1	49 000 14 500
62	71 72 79 78	1	52 000 14 500
63	72 73 80 79	1	58 000 14 500
64	73 74 81 80	1	62 000 14 500
65	74 75 82 81	1	70 000 14 500
66	75 76 83 82	1	78 000 14 500
67	76 77 84 83	1	86 000 14 500
68	77 78 85 84	1	53 000 15 500
69	78 79 86 85	1	58 000 15 500
70	80 81 87 86	1	62 000 15 500
71	81 82 88 87	1	70 000 15 500
72	82 83 89 88	1	78 000 15 500
73	83 84 90 89	1	86 000 15 500
74	85 86 91 91	1	57 000 16 500
75	86 87 92 91	1	62 000 16 500
76	87 88 93 92	1	70 000 16 500
77	88 89 94 93	1	78 000 16 500
78	89 90 95 94	1	86 000 16 500

SSTIPH 1 LAYER OF GEOTEXTILE, P=4000 PPI, E=5%, EXTENDED, T=1000

2 LOAD CASE ----- 2

LARGEST ELE NO IN THIS INCREMENT 78
LARGEST N P NO IN THIS INCREMENT 35

BAND WIDTH----- 40
TOTAL NUMBER OF EQUATIONS----- 222
NUMBER OF EQUATIONS IN BLOCK----- 84
NUMBER OF BLOCKS----- 3

NUMBER OF N.P. FORCE CARDS----- 3
NUMBER OF PRESSURE CARDS----- 0

NODAL POINT FORCES (WEIGHTS OF ADDED ELEMENTS)

NP X-FORCE Y-FORCE

NP	X-FORCE	Y-FORCE
1	0	0
2	0	0
3	0	0
4	0	0
5	0	0
6	0	0
7	0	0
8	0	0
9	0	0
10	0	0
11	0	0
12	0	0
13	0	0
14	0	0
15	0	0
16	0	0
17	0	0
18	0	0
19	0	0
20	0	0
21	0	0
22	0	0
23	0	0
24	0	0
25	0	0
26	0	0
27	0	0
28	0	0
29	0	0
30	0	0
31	0	0
32	0	0
33	0	0
34	0	0
35	0	0
36	0	0
37	0	0
38	0	0
39	0	0
40	0	0
41	0	0
42	0	0
43	0	0
44	0	0
45	0	0
46	0	0
47	0	0
48	0	0
49	0	0
50	0	0
51	0	0
52	0	0
53	0	0
54	0	0
55	0	0
56	0	0
57	0	0
58	0	0
59	0	0
60	0	0
61	0	0
62	0	0

63	0	0
64	0	0
65	0	0
66	0	0
67	0	0
68	0	0
69	0	0
70	0	0
71	0	0
72	0	0
73	0	0
74	0	0
75	0	0
76	0	0
77	0	0
78	0	0
79	0	0
80	0	0
81	0	0
82	0	0
83	0	0
84	0	0
85	0	0
86	0	0
87	0	0
88	0	0
89	0	0
90	0	0
91	0	0
92	0	-4.50
93	0	-4.50
94	0	-2.00
95	0	0

LOAD CASE : 2 ITERATION : 2

NP	DELTA-X	DELTA-Y	DELTA-ZZ	X-DISP	Y-DISP	ZZ-ROTAT	TOTAL	NP
1	0000	0000	0000	0000	0000	0000	0000	1
2	0000	0000	0000	0000	0000	0000	0000	2
3	0000	0000	0000	0000	0000	0000	0000	3
4	0000	0000	0000	0000	0000	0000	0000	4
5	0000	0000	0000	0000	0000	0000	0000	5
6	0000	0000	0000	0000	0000	0000	0000	6
7	0000	0000	0000	0000	0000	0000	0000	7
8	0000	0000	0000	0000	0000	0000	0000	8
9	0000	0000	0000	0000	0000	0000	0000	9
10	0000	0000	0000	0000	0000	0000	0000	10
11	0000	0000	0000	0000	0000	0000	0000	11
12	0000	0000	0000	0000	0000	0000	0000	12
13	0000	0000	0000	0000	0000	0000	0000	13
14	0000	0001	0000	0000	0013	0000	0013	14
15	0001	0001	0000	0007	0007	0000	0010	15
16	0000	0000	0000	0003	0005	0000	0008	16
17	0000	0000	0000	0041	0020	0000	0048	17
18	0003	0017	0000	1553	0088	0000	1555	18
19	0074	0053	0000	2108	0084	0000	2218	19
20	0185	0027	0000	1289	1043	0000	1635	20
21	0174	0060	0000	0508	1281	0000	1360	21
22	0004	0002	0000	0005	0041	0000	0041	22
23	0009	0027	0000	0001	0057	0000	0057	23
24	0013	0057	0000	0020	0108	0000	0110	24
25	0024	0020	0000	0027	0035	0000	0041	25
26	0000	0008	0000	0000	0025	0000	0025	26
27	0000	0003	0000	0000	0023	0000	0023	27
28	0003	0001	0000	0025	0013	0000	0029	28
29	0000	0000	0000	0008	0006	0000	0008	29
30	0008	0004	0000	1048	0463	0000	1145	30
31	0036	0045	0000	1759	0183	0000	1768	31
32	0196	0155	0000	2221	1188	0000	2519	32
33	0582	0118	0000	2043	1936	0000	2857	33
34	0832	0051	0000	1888	2020	0000	2765	34
35	0958	0277	0000	1482	1517	0000	2121	35
36	0019	0072	0000	0011	0150	0000	0151	36
37	0030	0108	0000	0037	0198	0000	0199	37
38	0030	0037	0000	0030	0105	0000	0109	38
39	0000	0022	0000	0000	0039	0000	0039	39
40	0000	0003	0000	0000	0030	0000	0030	40
41	0013	0003	0000	0108	0028	0000	0111	41
42	0050	0008	0000	0029	0080	0000	0350	42
43	0088	0007	0000	0584	0775	0000	0958	43
44	0101	0273	0000	0818	0649	0000	0820	44
45	0124	0255	0000	0878	1584	0000	1722	45
46	0148	0210	0000	0838	2654	0000	2729	46
47	0153	0133	0000	0581	2895	0000	2753	47
48	0158	0218	0000	0488	2239	0000	2287	48
49	0025	1138	0000	0108	2328	0000	2329	49
50	0082	0261	0000	0020	0453	0000	0454	50
51	0004	0025	0000	0018	0114	0000	0115	51
52	0000	0030	0000	0000	0072	0000	0072	52
53	0121	0075	0000	0487	0478	0000	0668	53
54	0121	0242	0000	0412	0858	0000	0980	54
55	0139	0212	0000	0430	1914	0000	1961	55
56	0128	0131	0000	0408	2317	0000	2352	56
57	0088	0211	0000	0875	2371	0000	2485	57
58	0079	1152	0000	0301	2590	0000	2608	58
59	0053	0278	0000	0484	0924	0000	1024	59
60	0029	0029	0000	0040	0108	0000	0115	60

61	0000	0033	0000	0000	- 0052	0000	0052	61
62	- 0118	0241	0000	- 0281	- 0338	0000	0438	62
63	- 0118	0219	0000	- 0307	- 0326	0000	0975	63
64	- 0147	0119	0000	- 0357	- 1152	0000	1208	64
65	- 0239	0220	0000	- 0468	- 1429	0000	1504	65
66	- 0281	- 1192	0000	- 0519	- 1948	0000	2014	66
67	- 0121	- 0278	0000	- 0246	- 1203	0000	1228	67
68	- 0064	0034	0000	- 0079	- 0089	0000	0127	68
69	0000	0033	0000	0000	- 0030	0000	0030	69
70	- 0114	0229	0000	- 0202	- 0275	0000	0341	70
71	- 0101	0222	0000	- 0216	- 0508	0000	0540	71
72	- 0095	0118	0000	- 0240	- 0722	0000	0761	72
73	- 0149	0135	0000	- 0324	- 1022	0000	1072	73
74	- 0173	0253	0000	- 0373	- 1812	0000	1850	74
75	- 0181	0253	0000	- 0271	- 1307	0000	1335	75
76	- 0080	0035	0000	- 0107	- 0091	0000	0141	76
77	0000	0032	0000	0000	- 0016	0000	0016	77
78	- 0077	0221	0000	- 0128	- 0189	0000	0211	78
79	- 0039	0118	0000	- 0115	- 0352	0000	0370	79
80	- 0029	0181	0000	- 0133	- 0855	0000	0868	80
81	- 0038	- 1282	0000	- 0180	- 1822	0000	1830	81
82	0279	- 1890	0000	0189	- 2498	0000	2505	82
83	5207	- 0788	0000	5188	- 0811	0000	5251	83
84	0000	0885	0000	0000	0832	0000	0832	84
85	0010	0104	0000	- 0023	- 0054	0000	0088	85
86	0097	0183	0000	0057	- 0333	0000	0237	86
87	008	- 1327	0000	0037	- 1868	0000	1466	87
88	0048	0870	0000	- 0009	- 3727	0000	3727	88
89	4235	1554	0000	3955	- 1790	0000	4341	89
90	0000	2133	0000	0000	1937	0000	1937	90
91	0203	- 0158	0000	0203	- 0158	0000	0258	91
92	0325	1339	0000	0325	- 1339	0000	1374	92
93	0389	- 3878	0000	0389	- 3878	0000	3894	93
94	2424	2771	0000	2424	- 2771	0000	3862	94
95	0000	2359	0000	0000	2359	0000	3359	95

STRUCTURAL ELEMENTS - LINEAR ELASTIC

BAR ELEMENTS--INTERNAL MEMBER FORCES

ELEMENT NO. AXIAL FORCE

INCREMENTAL VALUES

1	- 1779
2	- 1072
3	- 1812
4	- 1388
5	- 0802
6	- 0359
7	1 0846
8	1800
9	3344
10	0250
11	

TOTAL VALUES

1	-1 1070
2	-1 0808
3	- 4370
4	- 4688
5	2272

6	9160
7	1 1233
8	2.1718
9	7863
10	- .2285
11	1078

FOUR NODE SOLID ELEMENTS - MODULI AND STRAINS (STRAINS IN PERCENT)

ELE	ELAS MOD	BULK MOD	SHEAR MOD	POIS	EPS-X	EPS-Y	GAM-XY	EPS-1	EPS-3	GAMMAX	ELE
1	541.7	343.8	233.4	181	004	007	001	007	004	004	1
2	484.0	313.2	215.4	158	002	002	018	007	011	018	2
3	536.5	335.0	232.7	153	019	019	042	028	029	057	3
4	7	17.3	2	484	1.280	1.35	1.936	1.755	0.631	2.388	4
5	7	17.4	2	484	1.681	1.758	2.424	2.228	0.907	3.233	5
6	7	18.0	2	484	1.530	1.72	2.424	2.413	1.111	3.153	6
7	7	18.4	2	483	1.338	1.881	2.481	2.248	1.308	4.554	7
8	6	17.1	2	494	1.840	1.828	2.886	1.713	0.726	2.439	8
9	474.8	343.0	155.3	215	002	123	004	123	002	121	9
10	474.0	343.0	152.7	235	012	207	004	207	012	219	10
11	477.5	374.4	192.0	244	005	203	083	213	014	227	11
12	512.3	385.8	211.5	211	017	100	083	107	009	088	12
13	538.1	319.9	238.8	127	018	006	002	018	006	011	13
14	371.7	222.1	184.5	130	017	009	033	004	030	034	14
15	1.6	13.5	5	480	545	735	1.417	880	1.050	1.910	15
16	9.3	17.1	3.3	403	1.852	897	2.198	2.238	1.282	3.520	16
17	1.2	16.8	4	488	847	683	2.330	1.923	4.13	2.337	17
18	6	17.2	2	494	841	2.411	2.223	2.773	1.003	3.776	18
19	6	17.1	2	484	1.133	2.854	3.910	3.852	1.932	5.584	19
20	7	18.2	2	494	1.147	3.725	2.824	4.055	1.478	5.534	20
21	6	17.2	2	494	1.331	2.815	1.816	2.790	1.106	3.897	21
22	481.0	380.2	188.5	244	026	301	013	301	026	328	22
23	410.8	311.8	188.5	233	000	230	123	245	015	280	23
24	487.2	311.4	181.8	192	036	107	083	119	024	085	24
25	328.7	214.2	140.7	172	087	010	081	088	031	118	25
26	0	1	0	495	100	181	689	331	413	744	26
27	0	1	0	198	838	879	811	754	794	1.548	27
28	1.3	13.5	4	484	838	287	1.870	1.218	874	2.080	28
29	1.5	14.6	5	483	433	883	1.974	1.870	355	2.028	29
30	1.5	15.2	2	494	1.35	1.786	3.744	2.907	1.281	4.184	30
31	8	15.9	7	484	2.220	4.472	3.529	1.599	5.128	31	31
32	8	16.6	2	484	824	2.327	5.100	3.785	2.085	5.882	32
33	7	17.7	2	494	1.180	4.829	2.285	5.042	1.372	6.414	33
34	9	17.0	3	481	085	4.057	818	4.107	1.45	4.252	34
35	308.2	271.3	121.1	277	029	445	1.81	480	014	448	35
36	340.0	216.1	148.4	181	007	089	012	070	007	083	36
37	422.8	461.9	199.7	323	084	031	081	101	058	138	37
38	11.9	345.6	4	495	003	000	1.148	67	1	3.4	38
39	318.3	437.2	115.8	38	008	083	235	144	089	243	39
40	146.7	415.8	51.0	438	120	883	047	584	120	984	40
41	1	4.0	0	485	2.28	2.776	2.97	2.785	2.20	2.585	41
42	180.5	419.4	83.4	425	442	3.487	1.887	3.723	0.78	4.402	42
43	877.9	527.1	285.2	278	543	2.204	2.804	3.670	1.010	4.680	43
44	832.5	550.3	248.1	275	342	1.721	2.084	2.251	2.18	2.489	44
45	928.5	647.1	382.5	197	034	086	019	087	035	102	45
46	1	4.0	0	495	070	009	834	388	450	837	46
47	181.1	412.8	83.6	424	028	013	196	108	093	200	47
48	258.4	429.7	93.2	382	028	042	274	172	102	274	48
49	11.9	388.6	4.0	495	178	110	1.907	930	898	1.929	49
50	1	4.0	0	495	101	228	2.853	1.582	1.283	2.858	50
51	815.6	842.5	327.5	245	223	2.783	013	2.783	2.23	3.008	51
52	2080.4	1078.3	1008.8	033	182	2.82	388	2.875	1.75	2.880	52
53	770.8	59.3	318.5	074	074	070	085	075	078	154	53
54	208.0	423.2	43.4	413	027	013	033	033	018	082	54
55	588.7	477.6	220.5	288	072	001	027	074	004	078	55
56	834.1	582.4	248.7	275	221	112	127	233	124	357	56
57	82.6	408.0	28.2	485	058	205	245	274	011	285	57
58	1	4.0	0	485	210	2.845	827	3.012	2.78	3.288	58
59	2274.8	1137.3	1137.3	000	183	2.889	728	2.705	2.29	2.934	59
60	848.3	588.8	421.0	124	118	086	087	070	119	188	60
61	1	4.0	0	485	007	018	104	048	058	108	61

62	383.8	443.6	134.9	348	008	007	083	047	047	084	62
63	823.9	488.8	204.5	281	101	061	081	111	061	173	63
64	488.0	539.3	378.5	152	038	080	058	104	028	078	64
65	1	4.0	0	495	255	9.507	2.684	9.626	4.33	10.119	65
66	1	4.0	0	495	-3.188	13.048	-28.897	21.839	-12.085	34.024	66
67	1	4.0	0	495	3.177	4.452	-27.539	15.251	-12.526	27.777	67
68	248.1	427.9	87.7	384	072	100	189	148	117	283	68
69	383.2	448.3	143.0	340	086	039	151	013	148	162	69
70	373.0	444.8	138.8	344	018	140	139	171	013	189	70
71	1	4.0	0	485	174	9.149	2.338	9.293	318	9.811	71
72	1	4.0	0	485	-5.577	13.819	5.488	14.287	-5.88	20.253	72
73	1	4.0	0	485	5.718	4.48	2.843	8.030	758	6.788	73
74	1	4.0	0	485	218	045	388	086	347	433	74
75	1082.0	805.7	481.7	192	073	018	240	100	157	258	75
76	1587.4	827.1	773.7	032	000	085	150	114	048	181	76
77	1	4.0	0	495	-3.938	6.112	5.858	5.904	-4.730	11.834	77
78	1	4.0	0	485	4.198	0.048	3.217	4.740	5.87	5.327	78

FOUR NODE SOLID ELEMENTS - STRESSES

ELE	SIG-X	SIG-Y	TAU-XY	SIG-1	SIG-3	TAU-MAX	THEYA	SIG1/SIG3	LEVEL	ELE
1	288	528	001	528	288	129	284	1.961	083	1
2	228	488	040	473	219	127	274	2.158	088	2
3	332	387	101	484	255	105	37440	1.822	088	3
4	384	539	004	539	384	087	1.275	1.479	1.744	4
5	377	558	011	558	377	081	3.388	1.484	1.52	5
6	448	627	008	627	445	086	2.828	1.421	1.92	6
7	437	633	004	633	437	034	1.627	1.385	1.85	7
8	333	522	002	522	333	085	520	1.589	1.893	8
9	340	1.001	004	1.001	340	331	330	2.942	1.96	9
10	408	1.455	010	1.455	408	524	520	3.589	2.90	10
11	452	1.487	011	1.488	421	533	589	3.533	2.91	11
12	382	909	011	932	370	531	11.829	2.522	1.82	12
13	221	312	005	312	221	048	3.339	1.412	0.31	13
14	060	223	058	242	042	100	17.820	5.818	0.87	14
15	089	180	005	180	089	041	3.736	2.180	812	15
16	255	284	015	289	250	025	19.198	1.198	4.95	16
17	319	408	006	406	319	044	4.117	1.275	875	17
18	355	458	005	458	355	051	2.646	1.286	1.015	18
19	345	443	008	450	344	053	4.442	1.309	1.082	19
20	418	540	007	540	418	056	3.407	1.228	1.113	20
21	340	445	003	445	340	053	1.821	1.310	1.055	21
22	333	1.671	023	1.672	332	870	981	5.033	4.01	22
23	344	1.235	020	1.271	338	488	-12.585	4.284	3.02	23
24	342	857	012	898	301	189	-18.774	2.321	1.23	24
25	300	090	012	380	101	185	62.341	26.430	1.88	25
26	000	000	000	000	000	000	56.112	832	-1.000	26
27	021	004	012	028	009	018	-87.947	-2.774	-1.000	27
28	083	047	040	086	015	041	-50.825	6.569	811	28
29	147	228	007	228	147	041	-5.211	1.557	817	29
30	184	278	011	278	182	048	-8.736	1.530	987	30
31	234	328	015	328	231	048	-8.950	1.419	989	31
32	198	292	021	297	193	052	-11.887	1.534	1.032	32
33	529	880	015	832	527	052	-8.201	1.199	1.048	33
34	578	676	006	676	578	049	-3.398	1.171	985	34
35	815	741	018	774	581	039	-9.687	3.053	2.02	35
36	082	278	018	280	080	110	6.754	4.853	0.93	36
37	511	108	018	578	073	252	-74.834	7.873	-2.557	37
38	038	070	018	184	050	102	39.818	-2.887	-1.000	38
39	087	237	120	343	086	134	30.787	3		

43	112	1 324	-1 259	2 115	- 579	1 397	-32 151	-3 113	-1 000	43
44	434	936	456	1 208	185	521	30 520	7 331	2 353	44
45	110	416	063	424	- 117	270	6 787	-3 611	-1 000	45
46	010	023	003	024	009	007	14 064	2 546	575	46
47	125	105	117	.232	- 003	117	47 437	-78 985	-1 000	47
48	331	461	361	763	029	367	39 898	26 421	9 450	48
49	255	172	- 053	- 148	- 281	068	-25 353	518	-1 000	49
50	112	402	- 061	414	100	157	-11 497	4 144	1 189	50
51	831	1 277	- 858	1 806	- 960	1 283	-20 979	-1 673	-1 000	51
52	036	775	424	889	-1 131	1 000	12 550	- 789	-1 000	52
53	396	339	161	372	- 430	401	11 815	- 867	-1 000	53
54	157	033	078	195	- 005	100	64 314	-40 229	-1 000	54
55	587	176	- 048	593	170	212	-83 402	3 489	925	55
56	577	278	311	588	- 328	958	80 542	-4 846	-1 000	56
57	071	1 211	- 042	1 223	1 089	082	-15 672	1 194	057	57
58	087	208	- 057	232	072	080	-22 861	3 212	822	58
59	-1 676	887	- 271	916	-1 704	1 310	-5 980	- 337	-1 000	59
60	850	259	213	288	- 889	594	10 488	- 336	-1 000	60
61	111	056	071	160	- 008	076	55 583	20 525	7 258	61
62	066	011	- 078	122	- 044	083	-54 762	-2 743	-1 000	62
63	730	- 116	- 132	750	- 136	443	81 357	-5 528	-1 000	63
64	559	929	186	1 007	482	283	22 585	2 091	405	64
65	436	510	- 017	514	432	041	-12 574	1 129	070	65
66	403	433	- 012	437	399	019	-19 258	1 086	036	66
67	215	326	- 006	327	215	056	-2 967	1 522	194	67
68	003	271	- 156	343	- 069	206	-24 678	-4 973	-1 000	68
69	811	518	- 186	372	757	182	38 011	492	-1 000	69
70	829	921	175	1 002	547	227	25 050	1 832	309	70
71	427	476	- 009	477	426	026	-10 269	1 121	045	71
72	342	374	002	374	342	016	3 829	1 096	038	72
73	230	242	001	242	230	008	5 311	1 054	020	73
74	002	007	002	007	- 002	008	11 890	-2 858	-1 000	74
75	758	119	-1 158	918	-1 588	1 238	-34 823	- 589	-1 000	75
76	053	1 073	1 184	1 834	- 708	1 271	33 178	-2 591	-1 000	76
77	100	125	002	125	099	013	5 284	1 244	085	77
78	184	184	001	184	184	007	5 334	1 075	028	78